

Jrbdesigns Ltd.
1 Saville Street Cudworth

Site: 19 Cropton Road
Job: Extension
job number MCP002

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SuperBeam 4.50g 441893

19 Cropton Road.SBW

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Beam: Side wall support steels first floor

Span: 3.1 m.

	Load name	Loading w1	Start x1	Loading w2	End x2	R1comp	R2comp
U	L floor	1.8	0		L	2.79	2.79
U	D floor	.9	0		L	1.39	1.39
R	D wall inner	5	2.6		3.1	0.20	2.30
R	D wall outer	7	2.6		3.1	0.28	3.22
Total load: 14.37 kN						4.67	9.70
Dead:						1.88	6.91
Live:						2.79	2.79

Load types: U:UDL R:Part UDL D: Dead; L: Live (positions in m. from R1)

Maximum B.M. = 4.04 kNm at 1.73 m. from R1

Maximum S.F. = -9.70 kN at R2

Live load deflection = $2.16 \times 1e8/EI$ at 1.55 m. from R1 (E in N/mm², I in cm⁴)

Total deflection = $4.14 \times 1e8/EI$ at 1.60 m. from R1

Steel calculation to BS449 Part 2 using S275 (Grade 43) steel

SECTION SIZE : 2No 178 x 102 x 19 UB Grade 43

D=177.8 mm B=101.2 mm t=4.8 mm T=7.9 mm $I_x=1,360 \text{ cm}^4$ $r_y=2.37 \text{ cm}$ $Z_x=153 \text{ cm}^3$

$L_e = 1.2L = 3.72\text{m}$. $L_e/r_y = 3.72 \times 100 / 2.37 = 157$ D/T = 22.5

Permissible bending stress, $p_{bc} = 93.2 \text{ N/mm}^2$ (Table 3a)

Actual bending stress, $f_{bc} = 4.037 \times 1000 / 2 \times 153.0 = 13.2 \text{ N/mm}^2$ OK

Maximum shear in web, $f_s = 9.701 \times 1000 / (2 \times 4.8 \times 177.8) = 5.7 \text{ N/mm}^2$ OK

Check unstiffened web capacities with loads of $4.669/2 = 2.334 \text{ kN}$ and $9.701/2 = 4.851 \text{ kN}$

Bearing: $p_b = 210 \text{ N/mm}^2$ (Table 9); C1 = 27.1 kN; C2 = 1.01 kN/mm

Buckling: $p_c = 141 \text{ N/mm}^2$ (Table 17a); C1 = 60.2 kN; C2 = 0.677 kN/mm

R1: Unstiffened web bearing capacity, $P_w = 27.1 \text{ kN}$: no minimum stiff bearing length required

R2: Unstiffened web bearing capacity, $P_w = 27.1 \text{ kN}$: no minimum stiff bearing length required

Live load deflection = $2.16 \times 1e8 / (2 \times 205,000 \times 1,360) = 0.4 \text{ mm}$ (L/7987) OK

Total deflection = $4.14 \times 1e8 / (2 \times 205,000 \times 1,360) = 0.7 \text{ mm}$ (L/4180)

Combined bending and shear check (14.c): $(f_{bc}/p_{bc})^2 + (f_s/p_s)^2 = 0.020$ at 1.74 m. (≤ 1.25 OK)

Bearing details

R2 (14.14kN factored): 300 x 100mm padstone

Local design strength of masonry (factored) = 0.700 N/mm^2 (User-ented value)

Factored stress under padstone = $14.14 \times 1000 / 300 \times 100 = 0.47 \text{ N/mm}^2$

Beams to be bolted together with M16 bolts at 600 c/crs.