

Calculations for the Design of Standard 2000mm high 'L' Wall Units
with Standard Design Parameters

Note that the L wall units must form part of an overall retaining structure designed by a competent engineer, which includes a base slab of at least 200mm thickness to ensure sufficient anchorage for the dowel bars fixing the L unit base. This slab when considered together with the L wall units must be sufficient to resist the sliding and overturning on the whole retaining structure resulting from its function. This is likely to result in the need for a slab to extend significantly beyond either the toe or the heel (or both) of the L unit and have sufficient bending reinforcement in the top or bottom (or both) as dictated by the design and the site circumstances.

Contract Name: Standard Product Design

Contract Number: -

Client: -

Reference: -

In accordance with: BS EN 1992-1:2004
BS EN 1997-1:2004

Assumed parameters: Maximum soil/backfill density: 18.0 kN/m³
Minimum angle of internal friction: 32.5°
Maximum angle of slope: 30.0°
Maximum live load surcharge: 10.0 kN/m²

Design cases considered: Buried foot (toe in), zero slope + maximum surcharge
Exposed foot (toe out), zero slope + maximum surcharge
Buried foot (toe in), maximum slope + zero surcharge
Exposed foot (toe out), maximum slope + zero surcharge

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C.M. Baker

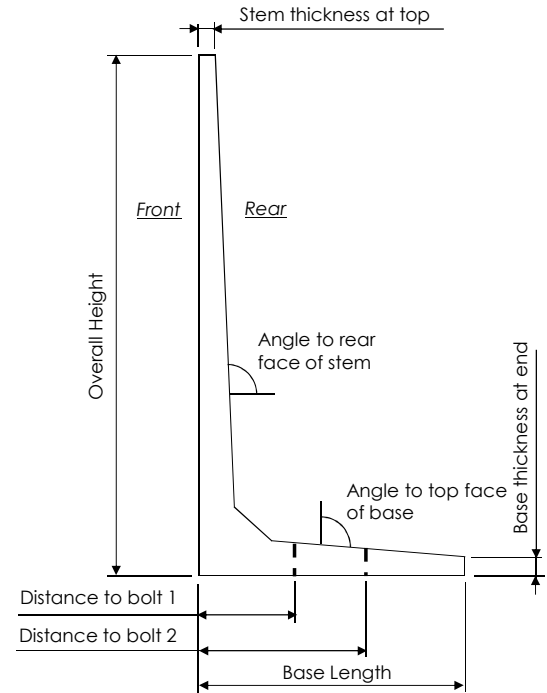
Rev	Description	By	Date

1.0 Retaining Wall Design

Overall height (h)	2000 mm
Base length (L _{base})	1000 mm
Stem thickness at top (t _{stem_top})	110 mm
Base thickness at end (t _{base_min})	120 mm
Unit width (w _u)	1000 mm
Distance to bolt 1 (from face)	500 mm
Distance to bolt 2 (from face)	N/A
Number of bolts / row (No.)	2 No.
Angle to rear face of stem (a1)	91.0°
Angle to top face of base (a2)	91.5°
Concrete Density (γ _c)	25.0 kN/m³
Average stem thickness (t _{stem})	126 mm
Average base thickness (t _{base})	133 mm
Stem height (h _{stem})	1867 mm
Toe length (L _{toe})	874 mm

Cover block (c)	45 mm
Cover deviation (ΔC _{dev})	5 mm

Height of retained soil (h _{ret})	2000 mm
Moist density (γ _{mr})	18.0 kN/m³
Char' eff' shear angle (φ' _{r,k})	32.5°
Char' wall friction angle (δ _{r,k})	21.7°
Angle of soil surface (β)	30.0°
Variable surcharge (Surcharge _G)	10.0 kN/m²



Depth to action of shear / moment (z)	500 mm
Additional shear force acting at z (V _z)	0.0 kN
Additional concentrated moment acting at z (M _z)	0.0 kNm

The following design is carried out in accordance with BS EN 1992-1:2004 & BS EN 1997-1:2004 at the ultimate limit state. Global Design (2.0) done on a per metre run basis and then pro-rated to the unit width, where not equal to a metre, in Design Values (2.6).

Partial Factors on Actions	Equilibrium (Table A.1)	Comb' 1 (Table A.3)	Comb' 2 (Table A.3)
Permanent unfavourable action (γ _G)	1.10	1.35	1.00
Permanent favourable action (γ _{Gf})	0.90	1.00	1.00
Variable unfavourable action (γ _Q)	1.50	1.50	1.30
Variable favourable action (γ _{Qf})	0.00	0.00	0.00
V _z partial factor (γ _{Vz})	1.50	1.50	1.30
M _z partial factor (γ _{Mz})	1.50	1.50	1.30

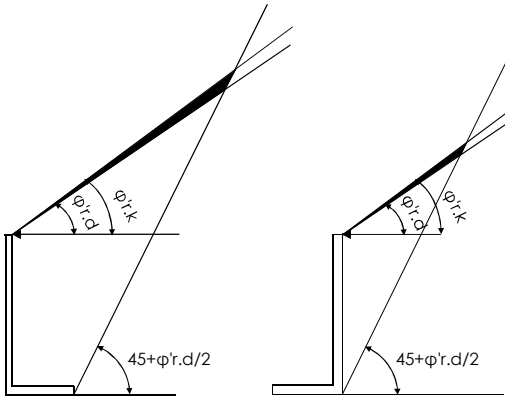
Partial Factors for Soil Parameters	Equilibrium (Table A.NA.2)	Comb' 1 (Table A.4)	Comb' 2 (Table A.4)
Angle of shearing resistance (γ _φ)	1.10	1.00	1.25
Effective cohesion (γ _c)	1.10	1.00	1.25
Weight density (γ _y)	1.00	1.00	1.00

Retained Soil Properties based on Coulomb	Equilibrium	Combination 1	Combination 2
Design eff' shear resistance angle (φ' _{r,d});	30.0°	32.5°	27.0°
Design wall friction angle (δ _{r,d});	19.9°	21.7°	17.7°
Level backfill active pressure coefficient (ka);	0.304	0.277	0.341
Sloped backfill active pressure coefficient (ka);	0.819	0.539	0.852

Soil Heights & Areas		Equilibrium		Combination 1		Combination 2	
		Rear (h _{eff1})	Front (h _{eff2})	Rear (h _{eff1})	Front (h _{eff2})	Rear (h _{eff1})	Front (h _{eff2})
Effective height of sloped soil (h _{eff})		2505 mm	2000 mm	2505 mm	2000 mm	2445 mm	2000 mm
Dimension properties (see diagram over page)	Property A	3.04305m	1.732m	2.885m	1.606m	3.249m	1.896m
	Property B	1.75691m	1.000m	1.666m	0.927m	1.876m	1.095m
	Property C	1.75691m	1.000m	1.838m	1.023m	1.656m	0.966m
	Property D	0.00000m	0.000m	-0.145m	-0.081m	0.196m	0.114m
	Property E	0.00000m	0.000m	-0.265m	-0.147m	0.320m	0.187m
	Property F	0.00000m	0.000m	-0.093m	-0.052m	0.100m	0.058m
Area of Soil (A)		0.000 m²	0.000 m²	-0.261 m²	-0.081 m²	0.336 m²	0.114 m²

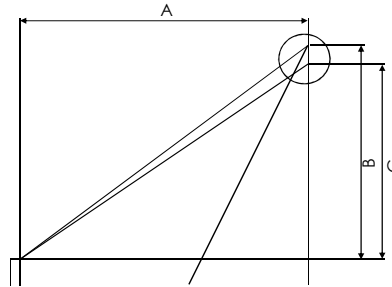
Additional Line Load

Method given in Reynolds (Table 21) to allow for the effect of the additional slope beyond the theoretical angle of repose

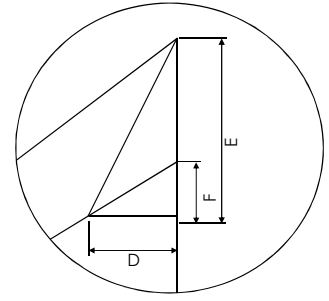


Sketch 1: Line load to rear of retaining wall

Sketch 2: Line load to front of retaining wall



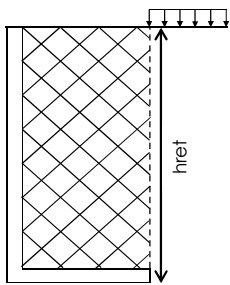
Primary Critical Dimensions



Secondary Critical Dimensions

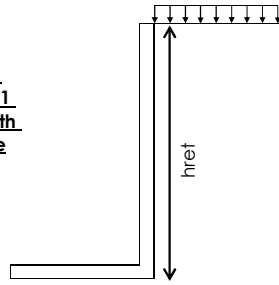
2.0 Retaining Wall Global Design

The global design determines a maximum overturning and sliding force on the retaining wall for each of the three combinations (EQU, Comb'1, Comb'2) in the two loading conditions (presented below) applied to the front and the back of the wall. All designs are done on a per/metre run basis.

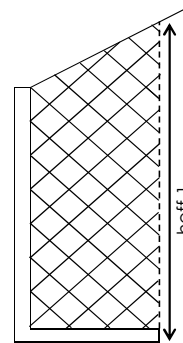


Backfill to rear of wall (A)

**Loading
condition 1**
**Level fill with
surcharge**

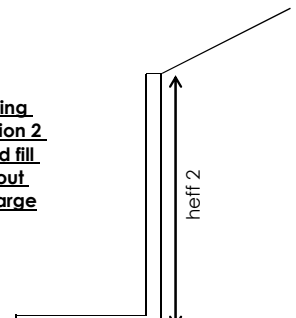


Backfill to front of wall (B)



Backfill to rear of wall (C)

**Loading
condition 2**
**Sloped fill
without
surcharge**



Backfill to front of wall (D)

2.1 Global Forces at EQU

Loading condition 1 - Backfill to rear of wall with level fill and surcharge (A)

Surcharge Load;

$$F_{sur,h} = K_a \cdot \cos(\delta_{r,d} - (90 - \alpha)) \cdot \gamma_Q \cdot \text{Surcharge}_Q \cdot h_{ret};$$

$$0.304 \cdot 0.934 \cdot 1.5 \cdot 10 \cdot 2 =$$

$$8.5 \text{ kN}$$

Lever Arm

$$1.000 \text{ m}$$

Moment

$$8.5 \text{ kNm}$$

Active Pressure;

$$F_{ret,h} = \gamma_G \cdot K_a \cdot \cos(\delta_{r,d} - (90 - \alpha)) \cdot \gamma_{mr} \cdot h_{ret}^2 / 2;$$

$$1.1 \cdot 0.304 \cdot 0.934 \cdot 18 \cdot 2^2 / 2 =$$

$$11.2 \text{ kN}$$

$$0.667 \text{ m}$$

$$7.5 \text{ kNm}$$

Additional shear load acting at z;

$$F_z = V_z \cdot \gamma_{Vz}$$

$$0 \cdot 1.5 =$$

$$0.0 \text{ kN}$$

$$1.500 \text{ m}$$

$$0.0 \text{ kNm}$$

Horizontal Force (H):

$$19.8 \text{ kN}$$

M_H :

$$16.0 \text{ kNm}$$

Weight of wall stem;

$$F_{stem} = \gamma_{Gf} \cdot h_{stem} \cdot t_{stem} \cdot \gamma_{stem};$$

$$0.9 \cdot 1.867 \cdot 0.126 \cdot 25 =$$

$$5.3 \text{ kN}$$

$$0.063 \text{ m}$$

$$0.3 \text{ kNm}$$

Weight of base;

$$F_{base} = \gamma_{Gf} \cdot L_{base} \cdot t_{base} \cdot \gamma_{base};$$

$$0.9 \cdot 1 \cdot 0.133 \cdot 25 =$$

$$3.0 \text{ kN}$$

$$0.500 \text{ m}$$

$$1.5 \text{ kNm}$$

Retained Soil;

$$F_{ret} = \gamma_{Gf} \cdot (h_{ret} \cdot t_{base}) \cdot L_{toe} \cdot \gamma_{mr};$$

$$0.9 \cdot 1.867 \cdot 0.874 \cdot 18 =$$

$$26.4 \text{ kN}$$

$$0.563 \text{ m}$$

$$14.9 \text{ kNm}$$

Friction on virtual Back of wall;

$$F_{wall \text{ frict}} = \gamma_{Gf} \cdot K_A \cdot \sin \delta_{r,d} \cdot \gamma_{mr} \cdot h_{ret}^2 / 2;$$

$$0.9 \cdot 0.304 \cdot 0.34 \cdot 18 \cdot 2^2 / 2 =$$

$$3.4 \text{ kN}$$

$$1.000 \text{ m}$$

$$3.4 \text{ kNm}$$

Vertical Force (V):

$$38.1 \text{ kN}$$

M_V :

$$20.1 \text{ kNm}$$

Loading condition 1 - Backfill to front of wall with level fill and surcharge (B)

			<u>Lever Arm</u>	<u>Moment</u>
Surcharge Load;				
$F_{sur,h} = K_a \cdot \cos(\delta_{r,d}) \cdot \gamma_Q \cdot \text{Surcharge}_Q \cdot h_{ref};$	$0.304 \cdot 0.94 \cdot 1.5 \cdot 10 \cdot 2 =$	8.6 kN	1.000 m	8.6 kNm
Active Pressure;				
$F_{ret,h} = \gamma_G \cdot K_a \cdot \cos(\delta_{r,d}) \cdot \gamma_{mr} \cdot h_{ref}^2 / 2;$	$1.1 \cdot 0.304 \cdot 0.94 \cdot 18 \cdot 2^2 / 2 =$	11.3 kN	0.667 m	7.5 kNm
Additional shear load acting at z;				
$F_z = V_z \cdot \gamma_{Vz}$	$0 \cdot 1.5 =$	0.0 kN	1.500 m	0.0 kNm
	Horizontal Force (H):	19.9 kN	$M_H;$	16.1 kNm
Weight of wall stem;				
$F_{stem} = \gamma_{Gf} \cdot h_{stem} \cdot t_{stem} \cdot \gamma_{stem};$	$0.9 \cdot 1.867 \cdot 0.126 \cdot 25 =$	5.3kN	0.937 m	5.0 kNm
Weight of base;				
$F_{base} = \gamma_{Gf} \cdot L_{base} \cdot t_{base} \cdot \gamma_{base};$	$0.9 \cdot 1 \cdot 0.133 \cdot 25 =$	3.0kN	0.500 m	1.5 kNm
Friction on virtual Back of wall;				
$F_{wall\ friction} = \gamma_{Gf} \cdot K_A \cdot \sin \delta_{r,d} \cdot \gamma_{mr} \cdot h_{ref}^2 / 2;$	$0.9 \cdot 0.304 \cdot 0.34 \cdot 18 \cdot 2^2 / 2 =$	3.4kN	1.000 m	3.4 kNm
	Vertical Force (V):	11.6 kN	$M_V;$	9.8 kNm

Loading condition 2 - Backfill to rear of wall with sloped fill and without surcharge (C)

			<u>Lever Arm</u>	<u>Moment</u>
Active Pressure;				
$F_{ret,h} = \gamma_G \cdot K_a \cdot \cos(\delta_{r,d} - (90 - \alpha)) \cdot \gamma_{mr} \cdot h_{eff1}^2 / 2;$	$1.1 \cdot 0.819 \cdot 0.934 \cdot 18 \cdot 2.505^2 / 2 =$	47.5kN	0.835 m	39.7 kNm
Additional Point Load				
$F_{x,h} = \gamma_G \cdot \gamma_{mr} \cdot \sin(\phi'_{r,d}) \cdot \cos(\phi'_{r,d}) \cdot A;$	$1.1 \cdot 18 \cdot 0.5 \cdot 0.866 \cdot 0 =$	0.0kN	2.000 m	0.0 kNm
Additional shear load acting at z;				
$F_z = V_z \cdot \gamma_{Vz}$	$0 \cdot 1.5 =$	0.0 kN	1.500 m	0.0 kNm
	Horizontal Force (H):	47.5 kN	$M_H;$	39.7 kNm
Weight of wall stem;				
$F_{stem} = \gamma_{Gf} \cdot h_{stem} \cdot t_{stem} \cdot \gamma_{stem};$	$0.9 \cdot 1.867 \cdot 0.126 \cdot 25 =$	5.3kN	0.063 m	0.3 kNm
Weight of base;				
$F_{base} = \gamma_{Gf} \cdot L_{base} \cdot t_{base} \cdot \gamma_{base};$	$0.9 \cdot 1 \cdot 0.133 \cdot 25 =$	3.0kN	0.500 m	1.5 kNm
Retained Soil;				
$F_{ret} = \gamma_{Gf} \cdot ((h_{ret} \cdot t_{base}) + (L_{toe} \cdot \tan(\beta) / 2)) \cdot L_{toe} \cdot \gamma_{mr};$	$0.9 \cdot (1.867 + (0.874 \cdot 0.577 / 2)) \cdot 0.874 \cdot 18 =$	30.0kN	0.563 m	16.9 kNm
Friction on virtual Back of wall;				
$F_{wall\ friction} = \gamma_{Gf} \cdot K_A \cdot \sin \delta_{r,d} \cdot \gamma_{mr} \cdot h_{eff1}^2 / 2;$	$0.9 \cdot 0.819 \cdot 0.34 \cdot 18 \cdot 2.505^2 / 2 =$	14.2kN	1.000 m	14.2 kNm
	Vertical Force (V):	52.5 kN	$M_V;$	32.9 kNm

Loading condition 2 - Backfill to front of wall with sloped fill and without surcharge (D)

			<u>Lever Arm</u>	<u>Moment</u>
Active Pressure;				
$F_{ret,h} = \gamma_G \cdot K_a \cdot \cos(\delta_{r,d}) \cdot \gamma_{mr} \cdot h_{eff2}^2 / 2;$	$1.1 \cdot 0.819 \cdot 0.94 \cdot 18 \cdot 2^2 / 2 =$	30.5kN	0.667 m	20.3 kNm
Additional Point Load				
$F_{x,h} = \gamma_G \cdot \gamma_{mr} \cdot \sin(\phi'_{r,d}) \cdot \cos(\phi'_{r,d}) \cdot A;$	$1.1 \cdot 18 \cdot 0.5 \cdot 0.866 \cdot 0 =$	0.0kN	2.000 m	0.0 kNm
Additional shear load acting at z;				
$F_z = V_z \cdot \gamma_{Vz}$	$0 \cdot 1.5 =$	0.0 kN	1.500 m	0.0 kNm
	Horizontal Force (H):	30.5 kN	$M_H;$	20.3 kNm
Weight of wall stem;				
$F_{stem} = \gamma_{Gf} \cdot h_{stem} \cdot t_{stem} \cdot \gamma_{stem};$	$0.9 \cdot 1.867 \cdot 0.126 \cdot 25 =$	5.3kN	0.937 m	5.0 kNm
Weight of base;				
$F_{base} = \gamma_{Gf} \cdot L_{base} \cdot t_{base} \cdot \gamma_{base};$	$0.9 \cdot 1 \cdot 0.133 \cdot 25 =$	3.0kN	0.500 m	1.5 kNm
Friction on virtual Back of wall;				
$F_{wall\ friction} = \gamma_{Gf} \cdot K_A \cdot \sin \delta_{r,d} \cdot \gamma_{mr} \cdot h_{eff2}^2 / 2;$	$0.9 \cdot 0.819 \cdot 0.34 \cdot 18 \cdot 2^2 / 2 =$	9.0kN	1.000 m	9.0 kNm
	Vertical Force (V):	17.3 kN	$M_V;$	15.5 kNm

2.2 Global Forces at Combination 1

Loading condition 1 -Backfill to rear of wall with level fill and surcharge (A)

			<u>Lever Arm</u>	<u>Moment</u>
Surcharge Load;				
$F_{sur,h} = K_a \cdot \cos(\delta_{r,d} - (90 - \alpha)) \cdot \gamma_Q \cdot \text{Surcharge}_Q \cdot h_{ret};$	$0.277 \cdot 0.923 \cdot 1.5 \cdot 10 \cdot 2 =$	7.7 kN	1.000 m	7.7 kNm
Active Pressure;				
$F_{ret,h} = \gamma_G \cdot K_a \cdot \cos(\delta_{r,d} - (90 - \alpha)) \cdot \gamma_{mr} \cdot h_{ret}^2 / 2;$	$1.35 \cdot 0.277 \cdot 0.923 \cdot 18 \cdot 2^2 / 2 =$	12.4 kN	0.667 m	8.3 kNm
Additional shear load acting at z;				
$F_z = V_z \cdot \gamma_{Vz}$	$0 \cdot 1.5 =$	0.0 kN	1.500 m	0.0 kNm
	Horizontal Force (H):	20.1 kN	$M_H:$	15.9 kNm
Weight of wall stem;				
$F_{stem} = \gamma_{Gf} \cdot h_{stem} \cdot t_{stem} \cdot \gamma_{stem};$	$1 \cdot 1.867 \cdot 0.126 \cdot 25 =$	5.9kN	0.063 m	0.4 kNm
Weight of base;				
$F_{base} = \gamma_{Gf} \cdot L_{base} \cdot t_{base} \cdot \gamma_{base};$	$1 \cdot 1 \cdot 0.133 \cdot 25 =$	3.3kN	0.500 m	1.7 kNm
Retained Soil;				
$F_{ret} = \gamma_{Gf} \cdot (h_{ret} - t_{base}) \cdot L_{toe} \cdot \gamma_{mr};$	$1 \cdot 1.867 \cdot 0.874 \cdot 18 =$	29.4kN	0.563 m	16.5 kNm
Friction on virtual Back of wall;				
$F_{wall\ friction} = \gamma_{Gf} \cdot K_A \cdot \sin \delta_{r,d} \cdot \gamma_{mr} \cdot h_{ret}^2 / 2;$	$1 \cdot 0.277 \cdot 0.37 \cdot 18 \cdot 2^2 / 2 =$	3.7kN	1.000 m	3.7 kNm
	Vertical Force (V):	42.3 kN	$M_V:$	22.3 kNm

Loading condition 1 -Backfill to front of wall with level fill and surcharge (B)

			<u>Lever Arm</u>	<u>Moment</u>
Surcharge Load;				
$F_{sur,h} = K_a \cdot \cos(\delta_{r,d}) \cdot \gamma_Q \cdot \text{Surcharge}_Q \cdot h_{ret};$	$0.277 \cdot 0.929 \cdot 1.5 \cdot 10 \cdot 2 =$	7.7 kN	1.000 m	7.7 kNm
Active Pressure;				
$F_{ret,h} = \gamma_G \cdot K_a \cdot \cos(\delta_{r,d}) \cdot \gamma_{mr} \cdot h_{ret}^2 / 2;$	$1.35 \cdot 0.277 \cdot 0.929 \cdot 18 \cdot 2^2 / 2 =$	12.5 kN	0.667 m	8.3 kNm
Additional shear load acting at z;				
$F_z = V_z \cdot \gamma_{Vz}$	$0 \cdot 1.5 =$	0.0 kN	1.500 m	0.0 kNm
	Horizontal Force (H):	20.2 kN	$M_H:$	16.1 kNm
Weight of wall stem;				
$F_{stem} = \gamma_{Gf} \cdot h_{stem} \cdot t_{stem} \cdot \gamma_{stem};$	$1 \cdot 1.867 \cdot 0.126 \cdot 25 =$	5.9kN	0.937 m	5.5 kNm
Weight of base;				
$F_{base} = \gamma_{Gf} \cdot L_{base} \cdot t_{base} \cdot \gamma_{base};$	$1 \cdot 1 \cdot 0.133 \cdot 25 =$	3.3kN	0.500 m	1.7 kNm
Friction on virtual Back of wall;				
$F_{wall\ friction} = \gamma_{Gf} \cdot K_A \cdot \sin \delta_{r,d} \cdot \gamma_{mr} \cdot h_{ret}^2 / 2;$	$1 \cdot 0.277 \cdot 0.37 \cdot 18 \cdot 2^2 / 2 =$	3.7kN	1.000 m	3.7 kNm
	Vertical Force (V):	12.9 kN	$M_V:$	10.9 kNm

Loading condition 2 - Backfill to rear of wall with sloped fill and without surcharge (C)

			<u>Lever Arm</u>	<u>Moment</u>
Active Pressure;				
$F_{ret,h} = \gamma_G \cdot K_a \cdot \cos(\delta_{r,d} - (90 - \alpha)) \cdot \gamma_{mr} \cdot h_{eff1}^2 / 2;$	$1.35 \cdot 0.539 \cdot 0.923 \cdot 18 \cdot 2.505^2 / 2 =$	37.9kN	0.835 m	31.7 kNm
Additional Point Load				
$F_{x,h} = \gamma_G \cdot \gamma_{mr} \cdot \sin(\phi'_{r,d}) \cdot \cos(\phi'_{r,d}) \cdot A;$	$1.35 \cdot 18 \cdot 0.537 \cdot 0.843 \cdot -0.261 =$	-2.9kN	2.000 m	-5.7 kNm
Additional shear load acting at z;				
$F_z = V_z \cdot \gamma_{Vz}$	$0 \cdot 1.5 =$	0.0 kN	1.500 m	0.0 kNm
	Horizontal Force (H):	35.0 kN	$M_H:$	25.9 kNm
Weight of wall stem;				
$F_{stem} = \gamma_{Gf} \cdot h_{stem} \cdot t_{stem} \cdot \gamma_{stem};$	$1 \cdot 1.867 \cdot 0.126 \cdot 25 =$	5.9kN	0.063 m	0.4 kNm
Weight of base;				
$F_{base} = \gamma_{Gf} \cdot L_{base} \cdot t_{base} \cdot \gamma_{base};$	$1 \cdot 1 \cdot 0.133 \cdot 25 =$	3.3kN	0.500 m	1.7 kNm
Retained Soil;				
$F_{ret} = \gamma_{Gf} \cdot ((h_{ret} - t_{base}) + (L_{toe} \cdot \tan(\beta) / 2)) \cdot L_{toe} \cdot \gamma_{mr};$	$1 \cdot (1.867 + (0.874 \cdot 0.577 / 2)) \cdot 0.874 \cdot 18 =$	33.3kN	0.563 m	18.8 kNm
Friction on virtual Back of wall;				
$F_{wall\ friction} = \gamma_{Gf} \cdot K_A \cdot \sin \delta_{r,d} \cdot \gamma_{mr} \cdot h_{eff1}^2 / 2;$	$1 \cdot 0.539 \cdot 0.37 \cdot 18 \cdot 2.505^2 / 2 =$	11.3kN	1.000 m	11.3 kNm
	Vertical Force (V):	53.8 kN	$M_V:$	32.1 kNm

Loading condition 2 - Backfill to front of wall with sloped fill and without surcharge (D)

			<u>Lever Arm</u>	<u>Moment</u>
Active Pressure;				
$F_{ret,h} = Y_G \cdot K_a \cdot \cos(\delta_{r,d}) \cdot Y_{mr} \cdot h_{eff2}^2 / 2;$	$1.35 \cdot 0.539 \cdot 0.929 \cdot 18 \cdot 2^2 / 2 =$	24.3kN	0.667 m	16.2 kNm
Additional Point Load				
$F_{x,h} = Y_G \cdot Y_{mr} \cdot \sin(\phi'_{r,d}) \cdot \cos(\phi'_{r,d}) \cdot A;$	$1.35 \cdot 18 \cdot 0.537 \cdot 0.843 \cdot -0.261 =$	-0.9kN	2.000 m	-1.8 kNm
Additional shear load acting at z;				
$F_z = V_z \cdot Y_{Vz}$	$0 \cdot 1.5 =$	0.0 kN	1.500 m	0.0 kNm
	Horizontal Force (H):	23.4 kN	$M_H;$	14.4 kNm
Weight of wall stem;				
$F_{stem} = Y_{Gf} \cdot h_{stem} \cdot t_{stem} \cdot Y_{stem};$	$1 \cdot 1.867 \cdot 0.126 \cdot 25 =$	5.9kN	0.937 m	5.5 kNm
Weight of base;				
$F_{base} = Y_{Gf} \cdot L_{base} \cdot t_{base} \cdot Y_{base};$	$1 \cdot 1 \cdot 0.133 \cdot 25 =$	3.3kN	0.500 m	1.7 kNm
Friction on virtual Back of wall;				
$F_{wall\ friction} = Y_{Gf} \cdot K_A \cdot \sin \delta_{r,d} \cdot Y_{mr} \cdot h_{eff2}^2 / 2;$	$1 \cdot 0.539 \cdot 0.37 \cdot 18 \cdot 2^2 / 2 =$	7.2kN	1.000 m	7.2 kNm
	Vertical Force (V):	16.4 kN	$M_V;$	14.3 kNm

2.3 Global Forces at Combination 2

Loading condition 1 -Backfill to rear of wall with level fill and surcharge (A)

			<u>Lever Arm</u>	<u>Moment</u>
Surcharge Load;				
$F_{sur,h} = K_a \cdot \cos(\delta_{r,d} - (90 - \alpha)) \cdot Y_Q \cdot \text{Surcharge}_Q \cdot h_{ret};$	$0.341 \cdot 0.948 \cdot 1.3 \cdot 10 \cdot 2 =$	8.4 kN	1.000 m	8.4 kNm
Active Pressure;				
$F_{ret,h} = Y_G \cdot K_a \cdot \cos(\delta_{r,d} - (90 - \alpha)) \cdot Y_{mr} \cdot h_{ret}^2 / 2;$	$1 \cdot 0.341 \cdot 0.948 \cdot 18 \cdot 2^2 / 2 =$	11.6 kN	0.667 m	7.8 kNm
Additional shear load acting at z;				
$F_z = V_z \cdot Y_{Vz}$	$0 \cdot 1.3 =$	0.0 kN	1.500 m	0.0 kNm
	Horizontal Force (H):	20.0 kN	$M_H;$	16.2 kNm
Weight of wall stem;				
$F_{stem} = Y_{Gf} \cdot h_{stem} \cdot t_{stem} \cdot Y_{stem};$	$1 \cdot 1.867 \cdot 0.126 \cdot 25 =$	5.9kN	0.063 m	0.4 kNm
Weight of base;				
$F_{base} = Y_{Gf} \cdot L_{base} \cdot t_{base} \cdot Y_{base};$	$1 \cdot 1 \cdot 0.133 \cdot 25 =$	3.3kN	0.500 m	1.7 kNm
Retained Soil;				
$F_{ret} = Y_{Gf} \cdot (h_{ret} - t_{base}) \cdot L_{toe} \cdot Y_{mr};$	$1 \cdot 1.867 \cdot 0.874 \cdot 18 =$	29.4kN	0.563 m	16.5 kNm
Friction on virtual Back of wall;				
$F_{wall\ friction} = Y_{Gf} \cdot K_A \cdot \sin \delta_{r,d} \cdot Y_{mr} \cdot h_{ret}^2 / 2;$	$1 \cdot 0.341 \cdot 0.303 \cdot 18 \cdot 2^2 / 2 =$	3.7kN	1.000 m	3.7 kNm
	Vertical Force (V):	42.3 kN	$M_V;$	22.3 kNm

Loading condition 1 -Backfill to front of wall with level fill and surcharge (B)

			<u>Lever Arm</u>	<u>Moment</u>
Surcharge Load;				
$F_{sur,h} = K_a \cdot \cos(\delta_{r,d}) \cdot Y_Q \cdot \text{Surcharge}_Q \cdot h_{ret};$	$0.341 \cdot 0.953 \cdot 1.3 \cdot 10 \cdot 2 =$	8.4 kN	1.000 m	8.4 kNm
Active Pressure;				
$F_{ret,h} = Y_G \cdot K_a \cdot \cos(\delta_{r,d}) \cdot Y_{mr} \cdot h_{ret}^2 / 2;$	$1 \cdot 0.341 \cdot 0.953 \cdot 18 \cdot 2^2 / 2 =$	11.7 kN	0.667 m	7.8 kNm
Additional shear load acting at z;				
$F_z = V_z \cdot Y_{Vz}$	$0 \cdot 1.3 =$	0.0 kN	1.500 m	0.0 kNm
	Horizontal Force (H):	20.1 kN	$M_H;$	16.2 kNm
Weight of wall stem;				
$F_{stem} = Y_{Gf} \cdot h_{stem} \cdot t_{stem} \cdot Y_{stem};$	$1 \cdot 1.867 \cdot 0.126 \cdot 25 =$	5.9kN	0.937 m	5.5 kNm
Weight of base;				
$F_{base} = Y_{Gf} \cdot L_{base} \cdot t_{base} \cdot Y_{base};$	$1 \cdot 1 \cdot 0.133 \cdot 25 =$	3.3kN	0.500 m	1.7 kNm
Friction on virtual Back of wall;				
$F_{wall\ friction} = Y_{Gf} \cdot K_A \cdot \sin \delta_{r,d} \cdot Y_{mr} \cdot h_{ret}^2 / 2;$	$1 \cdot 0.341 \cdot 0.303 \cdot 18 \cdot 2^2 / 2 =$	3.7kN	1.000 m	3.7 kNm
	Vertical Force (V):	12.9 kN	$M_V;$	10.9 kNm

Loading condition 2 - Backfill to rear of wall with sloped fill and without surcharge (C)

Active Pressure;

$$F_{ret,h} = \gamma_G \cdot K_a \cdot \cos(\delta_{r,d} - (90 - \alpha)) \cdot \gamma_{mr} \cdot h_{eff1}^2 / 2; \quad 1 \cdot 0.852 \cdot 0.948 \cdot 18 \cdot 2.445^2 / 2 = \quad 43.4 \text{ kN} \quad 0.815 \text{ m} \quad 35.4 \text{ kNm}$$

Additional Point Load

$$F_{x,h} = \gamma_G \cdot \gamma_{mr} \cdot \sin(\phi'_{r,d}) \cdot \cos(\phi'_{r,d}) \cdot A; \quad 1 \cdot 18 \cdot 0.454 \cdot 0.891 \cdot 0.336 = \quad 2.4 \text{ kN} \quad 2.000 \text{ m} \quad 4.9 \text{ kNm}$$

Additional shear load acting at z;

$$F_z = V_z \cdot \gamma_{Vz} \quad 0 \cdot 1.3 = \quad 0.0 \text{ kN} \quad 1.500 \text{ m} \quad 0.0 \text{ kNm}$$

$$\text{Horizontal Force (H):} \quad 45.9 \text{ kN} \quad M_H: \quad 40.3 \text{ kNm}$$

Weight of wall stem;

$$F_{stem} = \gamma_{Gf} \cdot h_{stem} \cdot t_{stem} \cdot \gamma_{stem}; \quad 1 \cdot 1.867 \cdot 0.126 \cdot 25 = \quad 5.9 \text{ kN} \quad 0.063 \text{ m} \quad 0.4 \text{ kNm}$$

Weight of base;

$$F_{base} = \gamma_{Gf} \cdot L_{base} \cdot t_{base} \cdot \gamma_{base}; \quad 1 \cdot 1 \cdot 0.133 \cdot 25 = \quad 3.3 \text{ kN} \quad 0.500 \text{ m} \quad 1.7 \text{ kNm}$$

Retained Soil;

$$F_{ret} = \gamma_{Gf} \cdot ((h_{ret} - t_{base}) + (L_{toe} \cdot \tan(\beta) / 2)) \cdot L_{toe} \cdot \gamma_{mr}; \quad 1 \cdot (1.867 + (0.874 \cdot 0.577 / 2)) \cdot 0.874 \cdot 18 = \quad 33.3 \text{ kN} \quad 0.563 \text{ m} \quad 18.8 \text{ kNm}$$

Friction on virtual Back of wall;

$$F_{wall \text{ frict}} = \gamma_{Gf} \cdot K_A \cdot \sin \delta_{r,d} \cdot \gamma_{mr} \cdot h_{eff1}^2 / 2; \quad 1 \cdot 0.852 \cdot 0.303 \cdot 18 \cdot 2.445^2 / 2 = \quad 13.9 \text{ kN} \quad 1.000 \text{ m} \quad 13.9 \text{ kNm}$$

$$\text{Vertical Force (V):} \quad 56.5 \text{ kN} \quad M_V: \quad 34.7 \text{ kNm}$$

Loading condition 2 - Backfill to front of wall with sloped fill and without surcharge (D)

Active Pressure;

$$F_{ret,h} = \gamma_G \cdot K_a \cdot \cos(\delta_{r,d}) \cdot \gamma_{mr} \cdot h_{eff2}^2 / 2; \quad 1 \cdot 0.852 \cdot 0.953 \cdot 18 \cdot 2^2 / 2 = \quad 29.2 \text{ kN} \quad 0.667 \text{ m} \quad 19.5 \text{ kNm}$$

Additional Point Load

$$F_{x,h} = \gamma_G \cdot \gamma_{mr} \cdot \sin(\phi'_{r,d}) \cdot \cos(\phi'_{r,d}) \cdot A; \quad 1 \cdot 18 \cdot 0.454 \cdot 0.891 \cdot 0.114 = \quad 0.8 \text{ kN} \quad 2.000 \text{ m} \quad 1.7 \text{ kNm}$$

Additional shear load acting at z;

$$F_z = V_z \cdot \gamma_{Vz} \quad 0 \cdot 1.3 = \quad 0.0 \text{ kN} \quad 1.500 \text{ m} \quad 0.0 \text{ kNm}$$

$$\text{Horizontal Force (H):} \quad 30.1 \text{ kN} \quad M_H: \quad 21.1 \text{ kNm}$$

Weight of wall stem;

$$F_{stem} = \gamma_{Gf} \cdot h_{stem} \cdot t_{stem} \cdot \gamma_{stem}; \quad 1 \cdot 1.867 \cdot 0.126 \cdot 25 = \quad 5.9 \text{ kN} \quad 0.937 \text{ m} \quad 5.5 \text{ kNm}$$

Weight of base;

$$F_{base} = \gamma_{Gf} \cdot L_{base} \cdot t_{base} \cdot \gamma_{base}; \quad 1 \cdot 1 \cdot 0.133 \cdot 25 = \quad 3.3 \text{ kN} \quad 0.500 \text{ m} \quad 1.7 \text{ kNm}$$

Friction on virtual Back of wall;

$$F_{wall \text{ frict}} = \gamma_{Gf} \cdot K_A \cdot \sin \delta_{r,d} \cdot \gamma_{mr} \cdot h_{eff2}^2 / 2; \quad 1 \cdot 0.852 \cdot 0.303 \cdot 18 \cdot 2^2 / 2 = \quad 9.3 \text{ kN} \quad 1.000 \text{ m} \quad 9.3 \text{ kNm}$$

$$\text{Vertical Force (V):} \quad 18.5 \text{ kN} \quad M_V: \quad 16.5 \text{ kNm}$$

2.4 Summary of forces and moments on wall

Equilibrium	H	M _H	V	M _V
A: LC1 - Backfill to rear:	19.8 kN	16.0 kNm	38.1 kN	20.1 kNm
B: LC1 - Backfill to front:	19.9 kN	16.1 kNm	11.6 kN	9.8 kNm
C: LC2 - Backfill to rear:	47.5 kN	39.7 kNm	52.5 kN	32.9 kN
D: LC2 - Backfill to front:	30.5 kN	20.3 kNm	17.3 kN	15.5 kN
Combination 1	H	M _H	V	M _V
A: LC1 - Backfill to rear:	20.1 kN	15.9 kNm	42.3 kN	22.3 kNm
B: LC1 - Backfill to front:	20.2 kN	16.1 kNm	12.9 kN	10.9 kNm
C: LC2 - Backfill to rear:	35.0 kN	25.9 kNm	53.8 kN	32.1 kN
D: LC2 - Backfill to front:	23.4 kN	14.4 kNm	16.4 kN	14.3 kN
Combination 2	H	M _H	V	M _V
A: LC1 - Backfill to rear:	20.0 kN	16.2 kNm	42.3 kN	22.3 kNm
B: LC1 - Backfill to front:	20.1 kN	16.2 kNm	12.9 kN	10.9 kNm
C: LC2 - Backfill to rear:	45.9 kN	40.3 kNm	56.5 kN	34.7 kN
D: LC2 - Backfill to front:	30.1 kN	21.1 kNm	18.5 kN	16.5 kNm

2.5 Resultant actions

Sliding force

Assuming the characteristic base friction (μ) is equal to the vertical force multiplied by **0.4**

		<u>Equilibrium</u>	<u>Combination 1</u>	<u>Combination 2</u>
V• μ -H	LC1 - Backfill to rear:	4.5 kN 0.77 ULS Ratio	3.2 kN 0.84 ULS Ratio	3.1 kN 0.84 ULS Ratio
	LC1 - Backfill to front:	15.2 kN 0.23 ULS Ratio	15.1 kN 0.25 ULS Ratio	15.0 kN 0.26 ULS Ratio
	LC2 - Backfill to rear:	26.6 kN 0.44 ULS Ratio	13.5 kN 0.61 ULS Ratio	23.3 kN 0.49 ULS Ratio
	LC2 - Backfill to front:	23.6 kN 0.23 ULS Ratio	16.9 kN 0.28 ULS Ratio	22.7 kN 0.25 ULS Ratio

Resultant moment

*Positive values indicate a stabilising moment and negative values indicate a destabilising moment

		<u>Equilibrium</u>	<u>Combination 1</u>	<u>Combination 2</u>
M_V-M_H	LC1 - Backfill to rear:	4.0 kNm 1.25 ULS Ratio	6.3 kNm 1.40 ULS Ratio	6.1 kNm 1.38 ULS Ratio
	LC1 - Backfill to front:	-6.3 kNm 0.61 ULS Ratio	-5.2 kNm 0.68 ULS Ratio	-5.3 kNm 0.67 ULS Ratio
	LC2 - Backfill to rear:	-6.8 kNm 0.83 ULS Ratio	6.1 kNm 1.24 ULS Ratio	-5.6 kNm 0.86 ULS Ratio
	LC2 - Backfill to front:	-4.8 kNm 0.76 ULS Ratio	-0.1 kNm 0.99 ULS Ratio	-4.7 kNm 0.78 ULS Ratio

2.6 Design Values

The stability of the structure is defined by the EQU load combination and therefore the stability of the foundation design should be based upon the following maximum values which relate to this specific load combination. These values are pro-rated to take account of the width of the retaining wall. The loading condition will determine the direction of the sliding force and whether the moments are clockwise or anti-clockwise.

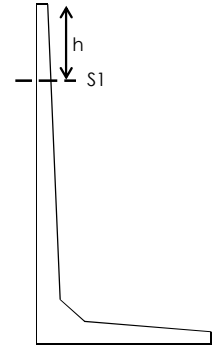
Resultant sliding force;	26.6 kN/m	- Loading condition 2 (Backfill to the rear)
Vertical load;	52.5 kN/m	- Loading condition 2 (Backfill to the rear)
Resultant destabilising moment;	6.8 kNm/m	- Loading condition 2 (Backfill to the rear)

The structural design of the retaining wall is based upon the following maximum pro-rated values from Combinations 1 & 2.

Resultant sliding force;	23.3 kN/m	- Combination 2 with loading condition 2 (Backfill to the rear)
Resultant anti-clockwise destabilising moment;	5.6 kNm/m	- Combination 2 with loading condition 2 (Backfill to the rear)
Resultant clockwise destabilising moment;	5.3 kNm/m	- Combination 2 with loading condition 1 (Backfill to the front)

3.1 Analysis Height 1

Analysis height 1 (S1) 650 mm
Soil depth at S1 650 mm



Area Related to Additional Line Load	Comb' 1	Comb' 2
Property A	0.522m	0.616m
Property B	0.301m	0.356m
Property C	0.332m	0.314m
Property D	-0.026m	0.037m
Property E	-0.048m	0.061m
Property F	-0.017m	0.019m
Area of Soil (A)	-0.009 m ²	0.012 m ²

Loading condition 1 -Backfill to rear of wall with level fill and surcharge (combination 1) (A)

			Lever Arm	Moment
Surcharge Load	$F_{sur_h} = K_a \cdot \cos(\delta r.d - (90-a)) \cdot \gamma Q \cdot \text{Surcharge} Q \cdot h;$	$0.277 \cdot 0.923 \cdot 1.5 \cdot 10 \cdot 0.65 =$	2.5 kN	0.325 m
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d - (90-a)) \cdot \gamma m r \cdot h^2 / 2;$	$1.35 \cdot 0.277 \cdot 0.923 \cdot 18 \cdot 0.65^2 / 2 =$	1.3 kN	0.217 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.5 =$	0.0 kN	0.150 m
Moment at z	$M_z = M_z \cdot \gamma M_z$		0.0 kN	0.000 m
Horizontal Force (H):		3.8 kN	$M_H:$	1.1 kNm

Loading condition 1 -Backfill to front of wall with level fill and surcharge (combination 1) (B)

			Lever Arm	Moment
Surcharge Load	$F_{sur_h} = K_a \cdot \cos(\delta r.d) \cdot \gamma Q \cdot \text{Surcharge} Q \cdot h;$	$0.277 \cdot 0.929 \cdot 1.5 \cdot 10 \cdot 0.65 =$	2.5 kN	0.325 m
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d) \cdot \gamma m r \cdot h^2 / 2;$	$1.35 \cdot 0.277 \cdot 0.929 \cdot 18 \cdot 0.65^2 / 2 =$	1.3 kN	0.217 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.5 =$	0.0 kN	0.150 m
Moment at z	$M_z = M_z \cdot \gamma M_z$		0.0 kN	0.000 m
Horizontal Force (H):		3.8 kN	$M_H:$	1.1 kNm

Loading condition 2 - Backfill to rear of wall with sloped fill and without surcharge (combination 1) (C)

			Lever Arm	Moment
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d - (90-a)) \cdot \gamma m r \cdot h^2 / 2;$	$1.35 \cdot 0.539 \cdot 0.923 \cdot 18 \cdot 0.65^2 / 2 =$	2.6kN	0.217 m
Additional Point Load	$F_{x_h} = \gamma G \cdot \gamma m r \cdot \sin(\phi' r.d) \cdot \cos(\phi' r.d) \cdot A;$	$1.35 \cdot 18 \cdot 0.537 \cdot 0.843 \cdot -0.009 =$	-0.1kN	0.650 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.5 =$	0.0 kN	0.150 m
Moment at z	$M_z = M_z \cdot \gamma M_z$		0.0 kN	0.000 m
Horizontal Force (H):		2.5 kN	$M_H:$	0.5 kNm

Loading condition 2 - Backfill to front of wall with sloped fill and without surcharge (combination 1) (D)

			Lever Arm	Moment
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d) \cdot \gamma m r \cdot h^2 / 2;$	$1.35 \cdot 0.539 \cdot 0.929 \cdot 18 \cdot 0.65^2 / 2 =$	2.6kN	0.217 m
Additional Point Load	$F_{x_h} = \gamma G \cdot \gamma m r \cdot \sin(\phi' r.d) \cdot \cos(\phi' r.d) \cdot A;$	$1.35 \cdot 18 \cdot 0.537 \cdot 0.843 \cdot -0.009 =$	-0.1kN	0.650 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.5 =$	0.0 kN	0.150 m
Moment at z	$M_z = M_z \cdot \gamma M_z$		0.0 kN	0.000 m
Horizontal Force (H):		2.5 kN	$M_H:$	0.5 kNm

Loading condition 1 -Backfill to rear of wall with level fill and surcharge (combination 2) (A)

			Lever Arm	Moment
Surcharge Load	$F_{sur_h} = K_a \cdot \cos(\delta r.d - (90-a)) \cdot \gamma Q \cdot \text{Surcharge} Q \cdot h;$	$0.341 \cdot 0.948 \cdot 1.3 \cdot 10 \cdot 0.65 =$	2.7 kN	0.325 m
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d - (90-a)) \cdot \gamma m r \cdot h^2 / 2;$	$1 \cdot 0.341 \cdot 0.948 \cdot 18 \cdot 0.65^2 / 2 =$	1.2 kN	0.217 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.3 =$	0.0 kN	0.150 m
Moment at z	$M_z = M_z \cdot \gamma M_z$		0.0 kN	0.000 m
Horizontal Force (H):		4.0 kN	$M_H:$	1.2 kNm

Loading condition 1 -Backfill to front of wall with level fill and surcharge (combination 2) (B)

			Lever Arm	Moment
Surcharge Load	$F_{sur_h} = K_a \cdot \cos(\delta r.d) \cdot \gamma Q \cdot \text{Surcharge} Q \cdot h;$	$0.341 \cdot 0.953 \cdot 1.3 \cdot 10 \cdot 0.65 =$	2.7 kN	0.325 m
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d) \cdot \gamma m r \cdot h^2 / 2;$	$1 \cdot 0.341 \cdot 0.953 \cdot 18 \cdot 0.65^2 / 2 =$	1.2 kN	0.217 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.3 =$	0.0 kN	0.150 m
Moment at z	$M_z = M_z \cdot \gamma M_z$		0.0 kN	0.000 m
Horizontal Force (H):		4.0 kN	$M_H:$	1.2 kNm

Loading condition 2 - Backfill to rear of wall with sloped fill and without surcharge (combination 2) (C)

			Lever Arm	Moment
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d - (90-a)) \cdot \gamma m r \cdot h^2 / 2;$	$1 \cdot 0.852 \cdot 0.948 \cdot 18 \cdot 0.65^2 / 2 =$	3.1kN	0.217 m
Additional Point Load	$F_{x_h} = \gamma G \cdot \gamma m r \cdot \sin(\phi' r.d) \cdot \cos(\phi' r.d) \cdot A;$	$1 \cdot 18 \cdot 0.454 \cdot 0.891 \cdot 0.012 =$	0.1kN	0.650 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.3 =$	0.0 kN	0.150 m
Moment at z	$M_z = M_z \cdot \gamma M_z$		0.0 kN	0.000 m
Horizontal Force (H):		3.2 kN	$M_H:$	0.7 kNm

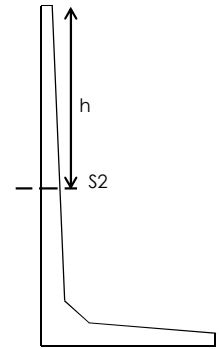
Loading condition 2 - Backfill to front of wall with sloped fill and without surcharge (combination 2) (D)

			Lever Arm	Moment
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d) \cdot \gamma m r \cdot h^2 / 2;$	$1 \cdot 0.852 \cdot 0.953 \cdot 18 \cdot 0.65^2 / 2 =$	3.1kN	0.217 m
Additional Point Load	$F_{x_h} = \gamma G \cdot \gamma m r \cdot \sin(\phi' r.d) \cdot \cos(\phi' r.d) \cdot A;$	$1 \cdot 18 \cdot 0.454 \cdot 0.891 \cdot 0.012 =$	0.1kN	0.650 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.3 =$	0.0 kN	0.150 m
Moment at z	$M_z = M_z \cdot \gamma M_z$		0.0 kN	0.000 m
Horizontal Force (H):		3.2 kN	$M_H:$	0.7 kNm

3.2 Analysis Height 2

Analysis height 2 (S2) 1395 mm
Soil depth at S2 1395 mm

Area Related to Additional Line Load	Comb' 1	Comb' 2
Property A	1.120m	1.323m
Property B	0.647m	0.764m
Property C	0.714m	0.674m
Property D	-0.056m	0.080m
Property E	-0.103m	0.130m
Property F	-0.036m	0.041m
Area of Soil (A)	-0.039 m ²	0.056 m ²



Loading condition 1 -Backfill to rear of wall with level fill and surcharge (combination 1) (A)

			Lever Arm	Moment
Surcharge Load	$F_{sur_h} = K_a \cdot \cos(\delta r.d - (90 - \alpha)) \cdot \gamma Q \cdot \text{Surcharge} Q \cdot h;$	$0.277 \cdot 0.923 \cdot 1.5 \cdot 10 \cdot 1.395 =$	5.3 kN	0.698 m
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d - (90 - \alpha)) \cdot \gamma m r \cdot h^2 / 2;$	$1.35 \cdot 0.277 \cdot 0.923 \cdot 18 \cdot 1.395^2 / 2 =$	6.0 kN	0.465 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.5 =$	0.0 kN	0.895 m
Moment at z	$M_z = M_z \cdot \gamma M_z$	$0.0 =$	0.0 kN	0.000 m
Horizontal Force (H):		11.4 kN	$M_H:$	6.5 kNm

Loading condition 1 -Backfill to front of wall with level fill and surcharge (combination 1) (B)

			Lever Arm	Moment
Surcharge Load	$F_{sur_h} = K_a \cdot \cos(\delta r.d) \cdot \gamma Q \cdot \text{Surcharge} Q \cdot h;$	$0.277 \cdot 0.929 \cdot 1.5 \cdot 10 \cdot 1.395 =$	5.4 kN	0.698 m
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d) \cdot \gamma m r \cdot h^2 / 2;$	$1.35 \cdot 0.277 \cdot 0.929 \cdot 18 \cdot 1.395^2 / 2 =$	6.1 kN	0.465 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.5 =$	0.0 kN	0.895 m
Moment at z	$M_z = M_z \cdot \gamma M_z$	$0.0 =$	0.0 kN	0.000 m
Horizontal Force (H):		11.5 kN	$M_H:$	6.6 kNm

Loading condition 2 - Backfill to rear of wall with sloped fill and without surcharge (combination 1) (C)

			Lever Arm	Moment
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d - (90 - \alpha)) \cdot \gamma m r \cdot h^2 / 2;$	$1.35 \cdot 0.539 \cdot 0.923 \cdot 18 \cdot 1.395^2 / 2 =$	11.8kN	0.465 m
Additional Point Load	$F_{x_h} = \gamma G \cdot \gamma m r \cdot \sin(\phi' r.d) \cdot \cos(\phi' r.d) \cdot A;$	$1.35 \cdot 18 \cdot 0.537 \cdot 0.843 \cdot -0.039 =$	-0.4kN	1.395 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.5 =$	0.0 kN	0.895 m
Moment at z	$M_z = M_z \cdot \gamma M_z$	$0.0 =$	0.0 kN	0.000 m
Horizontal Force (H):		11.3 kN	$M_H:$	4.9 kNm

Loading condition 2 - Backfill to front of wall with sloped fill and without surcharge (combination 1) (D)

			Lever Arm	Moment
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d) \cdot \gamma m r \cdot h^2 / 2;$	$1.35 \cdot 0.539 \cdot 0.929 \cdot 18 \cdot 1.395^2 / 2 =$	11.8kN	0.465 m
Additional Point Load	$F_{x_h} = \gamma G \cdot \gamma m r \cdot \sin(\phi' r.d) \cdot \cos(\phi' r.d) \cdot A;$	$1.35 \cdot 18 \cdot 0.537 \cdot 0.843 \cdot -0.039 =$	-0.4kN	1.395 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.5 =$	0.0 kN	0.895 m
Moment at z	$M_z = M_z \cdot \gamma M_z$	$0.0 =$	0.0 kN	0.000 m
Horizontal Force (H):		11.4 kN	$M_H:$	4.9 kNm

Loading condition 1 -Backfill to rear of wall with level fill and surcharge (combination 2) (A)

			Lever Arm	Moment
Surcharge Load	$F_{sur_h} = K_a \cdot \cos(\delta r.d - (90 - \alpha)) \cdot \gamma Q \cdot \text{Surcharge} Q \cdot h;$	$0.341 \cdot 0.948 \cdot 1.3 \cdot 10 \cdot 1.395 =$	5.9 kN	0.698 m
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d - (90 - \alpha)) \cdot \gamma m r \cdot h^2 / 2;$	$1 \cdot 0.341 \cdot 0.948 \cdot 18 \cdot 1.395^2 / 2 =$	5.7 kN	0.465 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.3 =$	0.0 kN	0.895 m
Moment at z	$M_z = M_z \cdot \gamma M_z$	$0.0 =$	0.0 kN	0.000 m
Horizontal Force (H):		11.5 kN	$M_H:$	6.7 kNm

Loading condition 1 -Backfill to front of wall with level fill and surcharge (combination 2) (B)

			Lever Arm	Moment
Surcharge Load	$F_{sur_h} = K_a \cdot \cos(\delta r.d) \cdot \gamma Q \cdot \text{Surcharge} Q \cdot h;$	$0.341 \cdot 0.953 \cdot 1.3 \cdot 10 \cdot 1.395 =$	5.9 kN	0.698 m
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d) \cdot \gamma m r \cdot h^2 / 2;$	$1 \cdot 0.341 \cdot 0.953 \cdot 18 \cdot 1.395^2 / 2 =$	5.7 kN	0.465 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.3 =$	0.0 kN	0.895 m
Moment at z	$M_z = M_z \cdot \gamma M_z$	$0.0 =$	0.0 kN	0.000 m
Horizontal Force (H):		11.6 kN	$M_H:$	6.8 kNm

Loading condition 2 - Backfill to rear of wall with sloped fill and without surcharge (combination 2) (C)

			Lever Arm	Moment
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d - (90 - \alpha)) \cdot \gamma m r \cdot h^2 / 2;$	$1 \cdot 0.852 \cdot 0.948 \cdot 18 \cdot 1.395^2 / 2 =$	14.1kN	0.465 m
Additional Point Load	$F_{x_h} = \gamma G \cdot \gamma m r \cdot \sin(\phi' r.d) \cdot \cos(\phi' r.d) \cdot A;$	$1 \cdot 18 \cdot 0.454 \cdot 0.891 \cdot 0.056 =$	0.4kN	1.395 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.3 =$	0.0 kN	0.895 m
Moment at z	$M_z = M_z \cdot \gamma M_z$	$0.0 =$	0.0 kN	0.000 m
Horizontal Force (H):		14.5 kN	$M_H:$	7.1 kNm

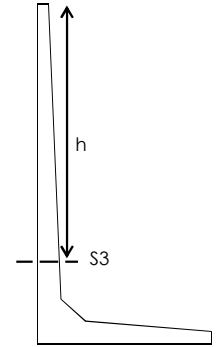
Loading condition 2 - Backfill to front of wall with sloped fill and without surcharge (combination 2) (D)

			Lever Arm	Moment
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d) \cdot \gamma m r \cdot h^2 / 2;$	$1 \cdot 0.852 \cdot 0.953 \cdot 18 \cdot 1.395^2 / 2 =$	14.2kN	0.465 m
Additional Point Load	$F_{x_h} = \gamma G \cdot \gamma m r \cdot \sin(\phi' r.d) \cdot \cos(\phi' r.d) \cdot A;$	$1 \cdot 18 \cdot 0.454 \cdot 0.891 \cdot 0.056 =$	0.4kN	1.395 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.3 =$	0.0 kN	0.895 m
Moment at z	$M_z = M_z \cdot \gamma M_z$	$0.0 =$	0.0 kN	0.000 m
Horizontal Force (H):		14.6 kN	$M_H:$	7.2 kNm

3.3 Analysis Height 3

Analysis height 3 (S3) 1730 mm
Soil depth at S3 1730 mm

Area Related to Additional Line Load	Comb' 1	Comb' 2
Property A	1.389m	1.640m
Property B	0.802m	0.947m
Property C	0.885m	0.836m
Property D	-0.070m	0.099m
Property E	-0.128m	0.161m
Property F	-0.045m	0.050m
Area of Soil (A)	-0.061 m ²	0.086 m ²



Loading condition 1 -Backfill to rear of wall with level fill and surcharge (combination 1) (A)

Surcharge Load	$F_{sur_h}=K_a \cdot \cos(\delta r.d-(90-\alpha)) \cdot \gamma Q \cdot \text{Surcharge} Q \cdot h;$	$0.277 \cdot 0.923 \cdot 1.5 \cdot 10 \cdot 1.73=$	6.6 kN	0.865 m	5.7 kNm
Active Pressure	$F_{ret_h}=\gamma G \cdot K_a \cdot \cos(\delta r.d-(90-\alpha)) \cdot \gamma m r \cdot h^2/2;$	$1.35 \cdot 0.277 \cdot 0.923 \cdot 18 \cdot 1.73^2/2=$	9.3 kN	0.577 m	5.4 kNm
Shear at z	$F_z=V_z \cdot \gamma V_z$	$0 \cdot 1.5=$	0.0 kN	1.230 m	0.0 kNm
Moment at z	$M_z=M_z \cdot \gamma M_z$		0.0 kN	0.000 m	0.0 kNm
Horizontal Force (H):			15.9 kN	$M_H:$	11.1 kNm

Loading condition 1 -Backfill to front of wall with level fill and surcharge (combination 1) (B)

Surcharge Load	$F_{sur_h}=K_a \cdot \cos(\delta r.d) \cdot \gamma Q \cdot \text{Surcharge} Q \cdot h;$	$0.277 \cdot 0.929 \cdot 1.5 \cdot 10 \cdot 1.73=$	6.7 kN	0.865 m	5.8 kNm
Active Pressure	$F_{ret_h}=\gamma G \cdot K_a \cdot \cos(\delta r.d) \cdot \gamma m r \cdot h^2/2;$	$1.35 \cdot 0.277 \cdot 0.929 \cdot 18 \cdot 1.73^2/2=$	9.4 kN	0.577 m	5.4 kNm
Shear at z	$F_z=V_z \cdot \gamma V_z$	$0 \cdot 1.5=$	0.0 kN	1.230 m	0.0 kNm
Moment at z	$M_z=M_z \cdot \gamma M_z$		0.0 kN	0.000 m	0.0 kNm
Horizontal Force (H):			16.0 kN	$M_H:$	11.2 kNm

Loading condition 2 - Backfill to rear of wall with sloped fill and without surcharge (combination 1) (C)

Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d - (90 - \alpha)) \cdot \gamma m r \cdot h^2 / 2;$	$1.35 \cdot 0.539 \cdot 0.923 \cdot 18 \cdot 1.73^2 / 2 =$	18.1kN	0.577 m	10.4 kNm
Additional Point Load	$F_{x,h} = \gamma G \cdot \gamma m r \cdot \sin(\phi' r.d) \cdot \cos(\phi' r.d) \cdot A;$	$1.35 \cdot 18 \cdot 0.537 \cdot 0.843 \cdot -0.061 =$	-0.7kN	1.730 m	-1.2 kNm
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.5 =$	0.0 kN	1.230 m	0.0 kNm
Moment at z	$M_z = M_z \cdot \gamma M_z$		0.0 kN	0.000 m	0.0 kNm
Horizontal Force (H):			17.4 kN	$M_H:$	9.3 kNm

Loading condition 2 - Backfill to front of wall with sloped fill and without surcharge (combination 1) (D)

Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d) \cdot \gamma m r \cdot h^2 / 2;$	$1.35 \cdot 0.539 \cdot 0.929 \cdot 18 \cdot 1.73^2 / 2 =$	18.2kN	0.577 m	10.5 kNm
Additional Point Load	$F_{x,h} = \gamma G \cdot \gamma m r \cdot \sin(\phi' r.d) \cdot \cos(\phi' r.d) \cdot A;$	$1.35 \cdot 18 \cdot 0.537 \cdot 0.843 \cdot -0.061 =$	-0.7kN	1.730 m	-1.2 kNm
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.5 =$	0.0 kN	1.230 m	0.0 kNm
Moment at z	$M_z = M_z \cdot \gamma M_z$		0.0 kN	0.000 m	0.0 kNm
Horizontal Force (H):			17.5 kN	$M_H:$	9.3 kNm

Loading condition 1 -Backfill to rear of wall with level fill and surcharge (combination 2) (A)

Surcharge Load	$F_{sur_h}=K_a \cdot \cos(\delta r.d-(90-\alpha)) \cdot \gamma Q \cdot \text{Surcharge} Q \cdot h;$	$0.341 \cdot 0.948 \cdot 1.3 \cdot 10 \cdot 1.73=$	7.3 kN	0.865 m	6.3 kNm
Active Pressure	$F_{ret_h}=\gamma G \cdot K_a \cdot \cos(\delta r.d-(90-\alpha)) \cdot \gamma m r \cdot h^2/2;$	$1 \cdot 0.341 \cdot 0.948 \cdot 18 \cdot 1.73^2/2=$	8.7 kN	0.577 m	5.0 kNm
Shear at z	$F_z=V_z \cdot \gamma V_z$	$0 \cdot 1.3=$	0.0 kN	1.230 m	0.0 kNm
Moment at z	$M_z=M_z \cdot \gamma M_z$		0.0 kN	0.000 m	0.0 kNm
Horizontal Force (H):			16.0 kN	$M_H:$	11.3 kNm

Loading condition 1 -Backfill to front of wall with level fill and surcharge (combination 2) (B)

Surcharge Load	$F_{sur_h}=K_a \cdot \cos(\delta r.d) \cdot \gamma Q \cdot \text{Surcharge} Q \cdot h;$	$0.341 \cdot 0.953 \cdot 1.3 \cdot 10 \cdot 1.73=$	7.3 kN	0.865 m	6.3 kNm
Active Pressure	$F_{ret_h}=\gamma G \cdot K_a \cdot \cos(\delta r.d) \cdot \gamma m r \cdot h^2/2;$	$1 \cdot 0.341 \cdot 0.953 \cdot 18 \cdot 1.73^2/2=$	8.8 kN	0.577 m	5.0 kNm
Shear at z	$F_z=V_z \cdot \gamma V_z$	$0 \cdot 1.3=$	0.0 kN	1.230 m	0.0 kNm
Moment at z	$M_z=M_z \cdot \gamma M_z$		0.0 kN	0.000 m	0.0 kNm
Horizontal Force (H):			16.1 kN	$M_H:$	11.4 kNm

Loading condition 2 - Backfill to rear of wall with sloped fill and without surcharge (combination 2) (C)

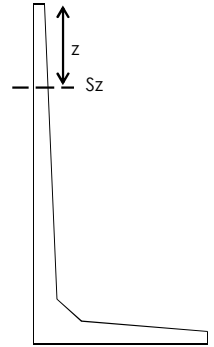
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d - (90 - \alpha)) \cdot \gamma m r \cdot h^2 / 2;$	$1 \cdot 0.852 \cdot 0.948 \cdot 18 \cdot 1.73^2 / 2 =$	21.7kN	0.577 m	12.5 kNm
Additional Point Load	$F_{x,h} = \gamma G \cdot \gamma m r \cdot \sin(\phi' r.d) \cdot \cos(\phi' r.d) \cdot A;$	$1 \cdot 18 \cdot 0.454 \cdot 0.891 \cdot 0.086 =$	0.6kN	1.730 m	1.1 kNm
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.3 =$	0.0 kN	1.230 m	0.0 kNm
Moment at z	$M_z = M_z \cdot \gamma M_z$		0.0 kN	0.000 m	0.0 kNm
Horizontal Force (H):			22.4 kN	$M_H:$	13.6 kNm

Loading condition 2 - Backfill to front of wall with sloped fill and without surcharge (combination 2) (D)

Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d) \cdot \gamma m r \cdot h^2 / 2;$	$1 \cdot 0.852 \cdot 0.953 \cdot 18 \cdot 1.73^2 / 2 =$	21.9kN	0.577 m	12.6 kNm
Additional Point Load	$F_{x,h} = \gamma G \cdot \gamma m r \cdot \sin(\phi' r.d) \cdot \cos(\phi' r.d) \cdot A;$	$1 \cdot 18 \cdot 0.454 \cdot 0.891 \cdot 0.086 =$	0.6kN	1.730 m	1.1 kNm
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.3 =$	0.0 kN	1.230 m	0.0 kNm
Moment at z	$M_z = M_z \cdot \gamma M_z$		0.0 kN	0.000 m	0.0 kNm
Horizontal Force (H):			22.5 kN	$M_H:$	13.7 kNm

3.3 Analysis Height Z

Height - Variable z (Sz) 500 mm
Soil depth at Sz 500 mm



Area Related to Additional Line Load	Comb' 1	Comb' 2
Property A	0.401m	0.474m
Property B	0.232m	0.274m
Property C	0.256m	0.242m
Property D	-0.020m	0.029m
Property E	-0.037m	0.047m
Property F	-0.013m	0.015m
Area of Soil (A)	-0.005 m ²	0.007 m ²

Loading condition 1 -Backfill to rear of wall with level fill and surcharge (combination 1) (A)

			Lever Arm	Moment
Surcharge Load	$F_{sur_h} = K_a \cdot \cos(\delta r.d - (90 - a)) \cdot \gamma Q \cdot \text{Surcharge} Q \cdot h;$	$0.277 \cdot 0.923 \cdot 1.5 \cdot 10 \cdot 0.5 =$	1.9 kN	0.250 m
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d - (90 - a)) \cdot \gamma m r \cdot h^2 / 2;$	$1.35 \cdot 0.277 \cdot 0.923 \cdot 18 \cdot 0.5^2 / 2 =$	0.8 kN	0.167 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.5 =$	0.0 kN	0.000 m
Moment at z	$M_z = M_z \cdot \gamma M_z$	0.0 kN	0.000 m	0.0 kNm
Horizontal Force (H):		2.7 kN	$M_H:$	0.6 kNm

Loading condition 1 -Backfill to front of wall with level fill and surcharge (combination 1) (B)

			Lever Arm	Moment
Surcharge Load	$F_{sur_h} = K_a \cdot \cos(\delta r.d) \cdot \gamma Q \cdot \text{Surcharge} Q \cdot h;$	$0.277 \cdot 0.929 \cdot 1.5 \cdot 10 \cdot 0.5 =$	1.9 kN	0.250 m
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d) \cdot \gamma m r \cdot h^2 / 2;$	$1.35 \cdot 0.277 \cdot 0.929 \cdot 18 \cdot 0.5^2 / 2 =$	0.8 kN	0.167 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.5 =$	0.0 kN	0.000 m
Moment at z	$M_z = M_z \cdot \gamma M_z$	0.0 kN	0.000 m	0.0 kNm
Horizontal Force (H):		2.7 kN	$M_H:$	0.6 kNm

Loading condition 2 - Backfill to rear of wall with sloped fill and without surcharge (combination 1) (C)

			Lever Arm	Moment
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d - (90 - a)) \cdot \gamma m r \cdot h^2 / 2;$	$1.35 \cdot 0.539 \cdot 0.923 \cdot 18 \cdot 0.5^2 / 2 =$	1.5kN	0.167 m
Additional Point Load	$F_{x_h} = \gamma G \cdot \gamma m r \cdot \sin(\phi' r.d) \cdot \cos(\phi' r.d) \cdot A;$	$1.35 \cdot 18 \cdot 0.537 \cdot 0.843 \cdot -0.005 =$	-0.1kN	0.500 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.5 =$	0.0 kN	0.000 m
Moment at z	$M_z = M_z \cdot \gamma M_z$	0.0 kN	0.000 m	0.0 kNm
Horizontal Force (H):		1.5 kN	$M_H:$	0.2 kNm

Loading condition 2 - Backfill to front of wall with sloped fill and without surcharge (combination 1) (D)

			Lever Arm	Moment
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d) \cdot \gamma m r \cdot h^2 / 2;$	$1.35 \cdot 0.539 \cdot 0.929 \cdot 18 \cdot 0.5^2 / 2 =$	1.5kN	0.167 m
Additional Point Load	$F_{x_h} = \gamma G \cdot \gamma m r \cdot \sin(\phi' r.d) \cdot \cos(\phi' r.d) \cdot A;$	$1.35 \cdot 18 \cdot 0.537 \cdot 0.843 \cdot -0.005 =$	-0.1kN	0.500 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.5 =$	0.0 kN	0.000 m
Moment at z	$M_z = M_z \cdot \gamma M_z$	0.0 kN	0.000 m	0.0 kNm
Horizontal Force (H):		1.5 kN	$M_H:$	0.2 kNm

Loading condition 1 -Backfill to rear of wall with level fill and surcharge (combination 2) (A)

			Lever Arm	Moment
Surcharge Load	$F_{sur_h} = K_a \cdot \cos(\delta r.d - (90 - a)) \cdot \gamma Q \cdot \text{Surcharge} Q \cdot h;$	$0.341 \cdot 0.948 \cdot 1.3 \cdot 10 \cdot 0.5 =$	2.1 kN	0.250 m
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d - (90 - a)) \cdot \gamma m r \cdot h^2 / 2;$	$1 \cdot 0.341 \cdot 0.948 \cdot 18 \cdot 0.5^2 / 2 =$	0.7 kN	0.167 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.3 =$	0.0 kN	0.000 m
Moment at z	$M_z = M_z \cdot \gamma M_z$	0.0 kN	0.000 m	0.0 kNm
Horizontal Force (H):		2.8 kN	$M_H:$	0.6 kNm

Loading condition 1 -Backfill to front of wall with level fill and surcharge (combination 2) (B)

			Lever Arm	Moment
Surcharge Load	$F_{sur_h} = K_a \cdot \cos(\delta r.d) \cdot \gamma Q \cdot \text{Surcharge} Q \cdot h;$	$0.341 \cdot 0.953 \cdot 1.3 \cdot 10 \cdot 0.5 =$	2.1 kN	0.250 m
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d) \cdot \gamma m r \cdot h^2 / 2;$	$1 \cdot 0.341 \cdot 0.953 \cdot 18 \cdot 0.5^2 / 2 =$	0.7 kN	0.167 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.3 =$	0.0 kN	0.000 m
Moment at z	$M_z = M_z \cdot \gamma M_z$	0.0 kN	0.000 m	0.0 kNm
Horizontal Force (H):		2.8 kN	$M_H:$	0.6 kNm

Loading condition 2 - Backfill to rear of wall with sloped fill and without surcharge (combination 2) (C)

			Lever Arm	Moment
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d - (90 - a)) \cdot \gamma m r \cdot h^2 / 2;$	$1 \cdot 0.852 \cdot 0.948 \cdot 18 \cdot 0.5^2 / 2 =$	1.8kN	0.167 m
Additional Point Load	$F_{x_h} = \gamma G \cdot \gamma m r \cdot \sin(\phi' r.d) \cdot \cos(\phi' r.d) \cdot A;$	$1 \cdot 18 \cdot 0.454 \cdot 0.891 \cdot 0.007 =$	0.1kN	0.500 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.3 =$	0.0 kN	0.000 m
Moment at z	$M_z = M_z \cdot \gamma M_z$	0.0 kN	0.000 m	0.0 kNm
Horizontal Force (H):		1.9 kN	$M_H:$	0.3 kNm

Loading condition 2 - Backfill to front of wall with sloped fill and without surcharge (combination 2) (D)

			Lever Arm	Moment
Active Pressure	$F_{ret_h} = \gamma G \cdot K_a \cdot \cos(\delta r.d) \cdot \gamma m r \cdot h^2 / 2;$	$1 \cdot 0.852 \cdot 0.953 \cdot 18 \cdot 0.5^2 / 2 =$	1.8kN	0.167 m
Additional Point Load	$F_{x_h} = \gamma G \cdot \gamma m r \cdot \sin(\phi' r.d) \cdot \cos(\phi' r.d) \cdot A;$	$1 \cdot 18 \cdot 0.454 \cdot 0.891 \cdot 0.007 =$	0.1kN	0.500 m
Shear at z	$F_z = V_z \cdot \gamma V_z$	$0 \cdot 1.3 =$	0.0 kN	0.000 m
Moment at z	$M_z = M_z \cdot \gamma M_z$	0.0 kN	0.000 m	0.0 kNm
Horizontal Force (H):		1.9 kN	$M_H:$	0.3 kNm

3.4 Summary of forces and moments on wall

These values are pro-rated to take account of the width of the retaining wall.

Combination 1	Analysis 1 @ 650		Analysis 2 @ 1395		Analysis 3 @ 1730		Analysis z @ 500	
	H	M _H	H	M _H	H	M _H	H	M _H
Rear level fill & surcharge (A)	3.8 kN	1.1 kNm	11.4 kN	6.5 kNm	15.9 kN	11.1 kNm	2.7 kN	0.6 kNm
Front level fill & surcharge (B)	3.8 kN	1.1 kNm	11.5 kN	6.6 kNm	16.0 kN	11.2 kNm	2.7 kN	0.6 kNm
Rear sloped fill without surcharge (C)	2.5 kN	0.5 kNm	11.3 kN	4.9 kNm	17.4 kN	9.3 kNm	1.5 kN	0.2 kNm
Front sloped fill without surcharge (D)	2.5 kN	0.5 kNm	11.4 kN	4.9 kNm	17.5 kN	9.3 kNm	1.5 kN	0.2 kNm
Combination 2	H	M _H	H	M _H	H	M _H	H	M _H
Rear level fill & surcharge (A)	4.0 kN	1.2 kNm	11.5 kN	6.7 kNm	16.0 kN	11.3 kNm	2.8 kN	0.6 kNm
Front level fill & surcharge (B)	4.0 kN	1.2 kNm	11.6 kN	6.8 kNm	16.1 kN	11.4 kNm	2.8 kN	0.6 kNm
Rear sloped fill without surcharge (C)	3.2 kN	0.7 kNm	14.5 kN	7.1 kNm	22.4 kN	13.6 kNm	1.9 kN	0.3 kNm
Front sloped fill without surcharge (D)	3.2 kN	0.7 kNm	14.6 kN	7.2 kNm	22.5 kN	13.7 kNm	1.9 kN	0.3 kNm

Back face is in tension values	Analysis 1 @ 650		Analysis 2 @ 1395		Analysis 3 @ 1730		Analysis z @ 500	
	H	M _H	H	M _H	H	M _H	H	M _H
Max sloped & horizontal comb' 1 & 2	4.0 kN	0.7 kNm	14.5 kN	7.1 kNm	22.4 kN	13.6 kNm	1.9 kN	0.3 kNm

Front face is in tension values	Analysis 1 @ 650		Analysis 2 @ 1395		Analysis 3 @ 1730		Analysis z @ 500	
	H	M _H	H	M _H	H	M _H	H	M _H
Max sloped & horizontal comb' 1 & 2	4.0 kN	1.2 kNm	14.6 kN	7.2 kNm	22.5 kN	13.7 kNm	2.8 kN	0.6 kNm

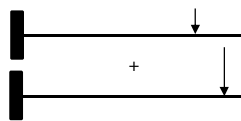
4.0 Wall Base Design

Analysis of the wall base is carried out at the following lengths (measured from the stem face of the wall):

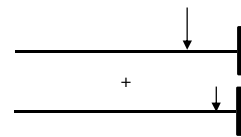
Analysis length 1 (B1)	240 mm
Analysis length 2 (B2)	630 mm
Analysis length 3 (B3)	955 mm

The number of rows of bolts will determine how the forces are distributed in the base.
This design is done on the basis of an anchor system using: **1 rows of bolts**

Since the overturning forces must be balanced by the force in the anchors there can be no rotation at the pivot point. Therefore the base can be modelled as having a fixed support at the pivot point as shown below.



Anti-Clockwise rotation (two Anchor system)



Clockwise rotation (two Anchor system)

Forces in single row anchor system

$$M = FS_1 \cdot lb_1$$

Forces in two row anchor system

$$M = FS_1 \cdot lb_1 + FS_2 \cdot lb_2 \quad (1)$$

$$FS_1 / lb_1 = FS_2 / lb_2 \quad (2)$$

$$FS_1 = lb_1 \cdot FS_2 / lb_2 \quad \text{Rearrange (2)}$$

$$M = (lb_1^2) \cdot FS_2 / lb_2 + FS_2 \cdot lb_2 \quad \text{Substituting into (1)}$$

$$FS_2 = M / [(lb_1^2) / lb_2 + lb_2] \quad \text{Factorise \& rearrange}$$

4.1 Anchored Anticlockwise Rotation

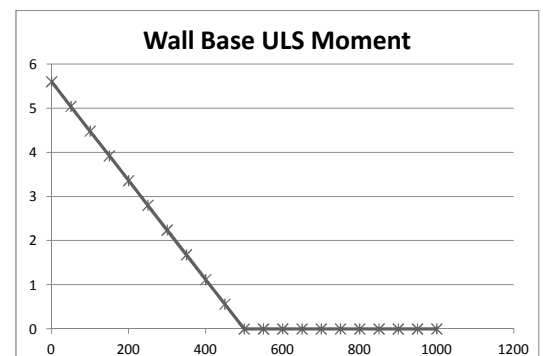
Maximum overturning moment	5.6 kNm
Lever arm to closest anchor (lb1)	500 mm
Lever arm to furthest anchor (lb2)	N/A

From the above equations:

$$\text{Tension force in closest anchors (FS1)} = 11.2 \text{ kN}$$

$$\text{Tension force in furthest anchors (FS2)} = 0.0 \text{ kN}$$

	X:	240mm	630mm	955mm
Tension Moment Top face:		2.9 kNm	0.0 kNm	0.0 kNm
Shear:		11.2 kN	0.0 kN	0.0 kN



4.2 Anchored Clockwise Rotation

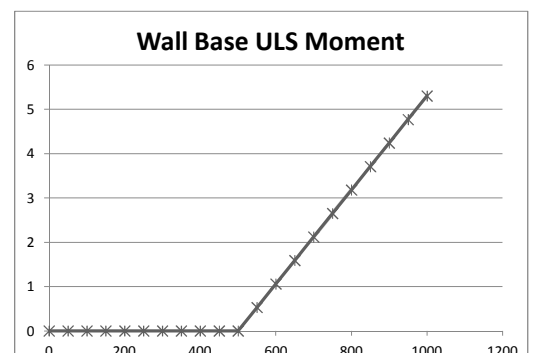
Maximum overturning moment	5.3 kNm
Lever arm to closest anchor (lb1)	500 mm
Lever arm to furthest anchor (lb2)	N/A

From the above equations:

$$\text{Tension force in closest anchors (FS1)} = 10.6 \text{ kN}$$

$$\text{Tension force in furthest anchors (FS2)} = 0.0 \text{ kN}$$

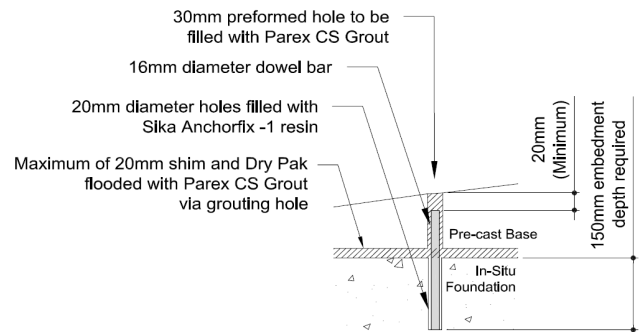
	X:	240mm	630mm	955mm
Tension Moment Top face:		0.0 kNm	1.4 kNm	4.8 kNm
Shear:		0.0 kN	10.6 kN	10.6 kN



5.0 Anchor Design

The anchor design, shown right, is considered for the effects of tension and shear. The dowel connections in the first row (closest to the face) and the second row are checked independently at ultimate limit state.

Char' ultimate tensile strength of dowel (f_{uk});	550 N/mm²
Dowel bar diameter(ϕ)	16 mm
Height of Shims	20 mm
Standard Embedment Length	150 mm
First Row: Dowel Bar Length (D_L)	275 mm
First Row: Effective Depth In Precast (h_{eff1});	105 mm
First Row: Effective Depth In Foundation (h_{eff2});	150 mm
Second Row: Dowel Bar Length (D_L)	N/A
Second Row: Effective Depth In Precast (h_{eff1});	N/A
Second Row: Effective Depth In Foundation (h_{eff2});	N/A



Determination of the bond resistance of grout

Conbextra GP grout has been tested in tension by the manufacturer in accordance with EN1504 Part 6 - Anchoring of reinforcing steel bar yielding the following results:

	Deformation 75kN	Maximum Load ($N_{u,m}$)
Dry test	0.49 mm	92.8 kN
Wet Test	0.44 mm	106.3 kN

Contact area of 16mm diameter test bar with 150mm embedment is equal to: 7540mm²

From the results of the tension tests in dry conditions the average bond resistance is calculated according to the following equation.

Average bond resistance;
 $f_{bm} = N_{u,m} / A;$
 $92.8 / 7540 = 12.3 \text{ N/mm}^2$

5.1 Anchorage in Precast

5.1.1 Resistance to Tension Loads

The resistance to tension loads in the pre-cast base shall be defined by the point of steel failure and the initial basic resistance of the anchor to pull-out. The design assumes that concrete cone failure and splitting failure cannot occur in the base due to the reinforcement cage used. The resistance to tension loads in the foundation is based on test data.

Maximum Tensile Load per Anchor (N);	First Row	Second Row
$N = \text{Max}\{\text{Clockwise Force, Anticlockwise Force}\} / \text{No.}$	$\text{Max}\{11.2, 0\} / 2 = 5.6 \text{ kN}$	N/A

5.1.1.1 Steel Failure

Area of steel;	
$A_s = \pi \cdot \phi^2 / 4;$	$\pi \cdot 16^2 / 4 = 201 \text{ mm}^2$
Characteristic resistance of an anchor in the case of steel failure;	
$N_{Rk,s} = A_s \cdot f_{uk};$	$201 \cdot 550 / 1000 = 110.6 \text{ kN}$
Design resistance of an anchor in the case of steel failure;	
$N_{Rd,s} = N_{Rk,s} / \gamma_{Ms};$	$110.6 / 1.15 = 96.1 \text{ kN}$

Ratio between design action and design resistance	First Row	Second Row
$\beta_N = N / N_{Rd,s}$	$5.6 / 96.1 = 0.06$	N/A

5.1.1.2 Pull-Out Failure to Pre-Cast Base

Characteristic resistance to pull-out;

$$N_{Rk,p} = \pi \cdot \phi \cdot H_{eff} \cdot f_{dm}$$

$$\pi \cdot 16 \cdot 105 \cdot 12.3 = 64.9 \text{ kN}$$

$$\text{Second Row}$$

Design tension capacity of anchorage;

$$N_{Rd,p} = N_{Rk,p} / \gamma_{grout}$$

$$64.9/3 = 21.6 \text{ kN}$$

$$\text{N/A}$$

Ratio between design action and design resistance

$$\beta_N = N / N_{Rd,s}$$

$$5.6/21.6 = 0.26$$

$$\text{N/A}$$

5.1.2 Resistance to Shear Loads

The resistance to shear loads is defined by the point of steel failure. The design assumes that the shear load acts at the failure surface and therefore there is no lever arm and that concrete pry out failure or edge failure cannot occur in the pre-cast base due to the reinforcement cage used.

Maximum shearing force per anchor;

$$V = \text{Max Shear Force} / \text{No. Dowels}$$

$$23.3 / (2 \cdot 1) = 11.7 \text{ kN}$$

5.1.2.1 Steel Failure, Shear Load without Lever Arm

Area of steel;

$$A_s = \pi \cdot \phi^2 / 4$$

$$\pi \cdot 16^2 / 4 = 201 \text{ mm}^2$$

Characteristic resistance of an anchor in the case of steel failure;

$$V_{Rk,s} = 0.5 \cdot A_s \cdot f_{yk}$$

$$0.5 \cdot 201 \cdot 550 / 1000 = 55.3 \text{ kN}$$

Design resistance of an anchor in the case of steel failure;

$$V_{Rd,s} = V_{Rk,s} / \gamma_{Ms}$$

$$55.3 / 1.15 = 48.1 \text{ kN}$$

Ratio between design action and design resistance

$$\beta_V = V / V_{Rd,s}$$

$$\text{First Row}$$

$$11.7 / 48.1 = 0.24$$

$$\text{Second Row}$$

$$\text{N/A}$$

5.1.3 Resistance to combined tension and shear loads

$$(N_{ED} / N_{RD, \text{Dowel}})^{1.5} + (V_{ED} / V_{RD, \text{dowel}})^{1.5} < 1.0;$$

$$\text{First Row}$$

$$0.13$$

$$\text{Second Row}$$

$$(N_{ED} / N_{RD})^{1.5} + (V_{ED} / V_{RD})^{1.5} < 1.0;$$

$$0.25$$

$$\text{N/A}$$

$$(N_{ED} / N_{RD}) + (V_{ED} / V_{RD}) < 1.2;$$

$$0.50$$

$$\text{N/A}$$

5.1 Anchorage in Foundation

From Sika Technical information - Characteristic resistance to pull-out of deformed bar;

$$N_{Rk,p} = 30 \text{ kN}$$

$$30.0 \text{ kN}$$

Design tension capacity of anchorage;

$$N_{Rd,p} = N_{Rk,p} / \gamma_{resin}$$

$$30/3 = 10.0 \text{ kN}$$

Ratio between design action and design resistance

$$\beta_N = N / N_{Rd,s}$$

$$\text{First Row}$$

$$5.6/10 = 0.56$$

$$\text{Second Row}$$

$$\text{N/A}$$

Anchorage provided is adequate

6.0 Reinforcement Design

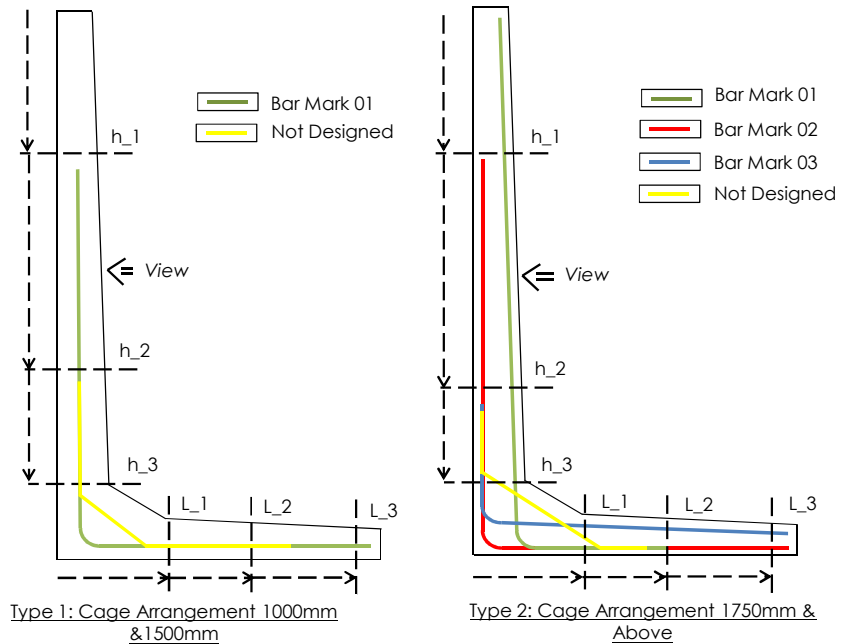
All the standard retaining walls have different reinforcement cages, however, they follow one of two patterns. The 1000mm and 1500mm are a type one and 1750mm and above are of a type 2, these are shown (right). A reinforcement check is then done on the three heights of the stem and three lengths of the base which are located to give the maximum moment and shear force with the minimum reinforcement.

Cage Provided

This retaining wall has a Type 2 cage.

Reinforcement Provided

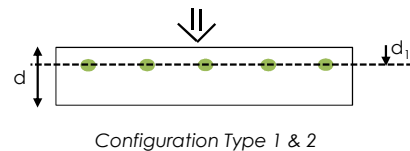
	Diameter	Spacing	As,prov
Bar Mark 1	11.05	200	479
Bar Mark 2	11.05	200	479
Bar Mark 3	11.05	200	479



6.1 Section Analysis

Analysis height 1 (650mm)

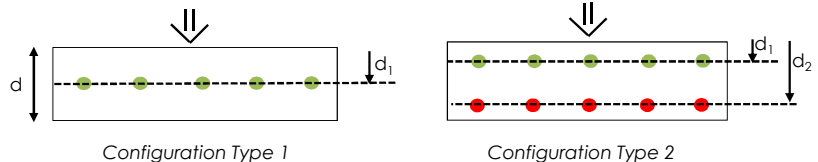
Depth @ S1 (hS1)	121 mm
hS1 - Depth to rein' (d1)	51 mm



	As,prov	Eff' Depth	As',prov	Eff' Depth
Back face in tension	479	71	0	0
Front face in tension	479	51	0	0

Analysis height 2 (1395mm)

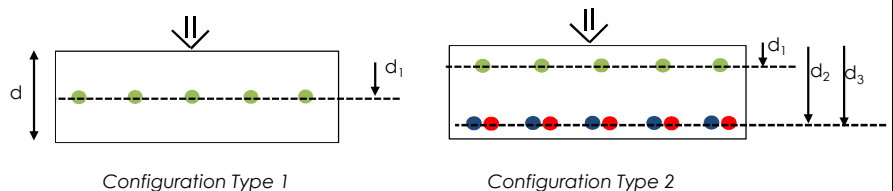
Depth @ S2 (hS2)	134 mm
hS2 - Depth to rein' (d1)	51 mm
hS2 - Depth to rein' (d2)	83 mm



	As,prov	Eff' Depth	As',prov	Eff' Depth
Back face in tension	479	83	479	51
Front face in tension	479	83	479	51

Analysis height 3 (1730mm)

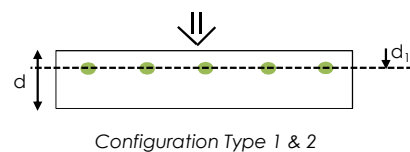
Depth @ S3 (hS3)	139 mm
hS3 - Depth to rein' (d1)	51 mm
hS3 - Depth to rein' (d2)	89 mm
hS3 - Depth to rein' (d3)	89 mm



	As,prov	Eff' Depth	As',prov	Eff' Depth
Back face in tension	479	89	958	51
Front face in tension	958	89	479	51

Analysis height z (500mm)

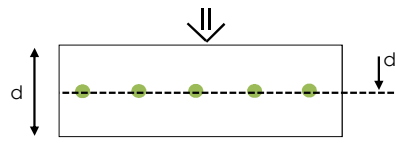
Depth @ Sz (hSz)	119 mm
Depth to rein' (d1)	48 mm



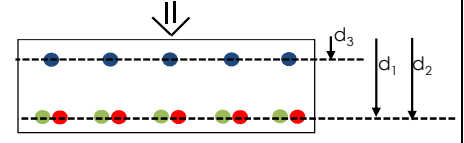
	As,prov	Eff' Depth	As',prov	Eff' Depth
Back face in tension	479	71	0	0
Front face in tension	479	48	0	0

Analysis length 1 (240mm)

Depth @ B1 (hB1)	140 mm
hB1 - Depth to rein' (d1)	89 mm
hB1 - Depth to rein' (d2)	89 mm
hB1 - Depth to rein' (d3)	51 mm



Configuration Type 1

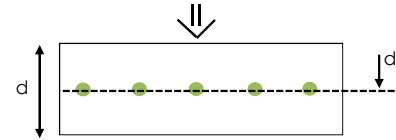


Configuration Type 2

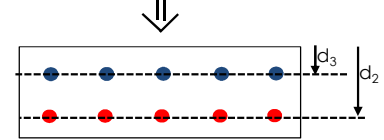
	As,prov	Eff' Depth	As',prov	Eff' Depth
Top face in tension	479	89	958	51
Bottom face in tension	958	89	479	51

Analysis length 2 (630mm)

Depth @ B2 (hB2)	130 mm
hB2 - Depth to rein' (d1)	0 mm
hB2 - Depth to rein' (d2)	79 mm
hB2 - Depth to rein' (d3)	51 mm



Configuration Type 1

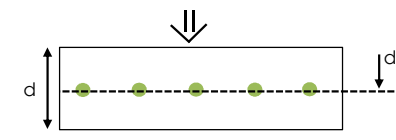


Configuration Type 2

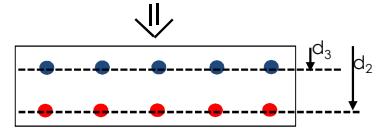
	As,prov	Eff' Depth	As',prov	Eff' Depth
Top face in tension	479	79	479	51
Bottom face in tension	479	79	479	51

Analysis length 3 (955mm)

Depth @ B3 (hB3)	121 mm
hB3 - Depth to rein' (d1)	0 mm
hB3 - Depth to rein' (d2)	71 mm
hB3 - Depth to rein' (d3)	51 mm



Configuration Type 1



Configuration Type 2

	As,prov	Eff' Depth	As',prov	Eff' Depth
Top face in tension	479	71	479	51
Bottom face in tension	479	71	479	51

6.2 Concrete Properties

Concrete details & design

Concrete strength class
Characteristic compressive cylinder strength
Characteristic compressive cube strength
Mean value of compressive cylinder strength
Mean value of axial tensile strength
5% fractile of axial tensile strength
Secant modulus of elasticity
Partial modulus of elasticity of concrete
Compressive strength coefficient - cl.3.1.6(1)
Design compressive concrete strength - cl.3.1.6(1)
Maximum aggregate size
Deviation
Required concrete class
Maximum concrete strut capacity
Minimum concrete strut capacity

Pre-Cast

C45/55

$$\begin{aligned} f_{ck} &= 45 \text{ N/mm}^2 \\ f_{ck,cube} &= 55 \text{ N/mm}^2 \\ f_{cm} &= 53 \text{ N/mm}^2 \\ f_{ctm} &= 4 \text{ N/mm}^2 \\ f_{ctk,0.05} &= 3 \text{ N/mm}^2 \\ E_{cm} &= 22 \cdot (f_{cm}/10)^{0.3} = 36283 \text{ N/mm}^2 \\ \gamma_c &= 1.50 \\ \alpha_{cc} &= 0.85 \\ f_{cd} &= \alpha_{cc} \cdot f_{ck} / \gamma_c = 25.5 \text{ N/mm}^2 \\ h_{agg} &= 20 \text{ mm} \\ \Delta C_{dev} &= 5 \text{ mm} \\ \text{Class} &= \mathbf{XC3/4} \\ VRd, \max(\cot(\theta) = 2.5) &= 5.08 \\ VRd, \max(\cot(\theta) = 1.0) &= 7.38 \end{aligned}$$

Reinforcement details

Characteristic yield strength of reinforcement
Modulus of elasticity of reinforcement
Partial factor for reinforcing steel - Table 2.1N
Design yield strength of reinforcement

$$\begin{aligned} f_{yk} &= 500 \text{ N/mm}^2 \\ E_s &= 210000 \text{ N/mm}^2 \\ \gamma_s &= 1.15 \\ f_{yd} &= f_{yk} / \gamma_s = 435 \text{ N/mm}^2 \end{aligned}$$

6.3 Stem Reinforcement Design

Moment

Moment capacity of section

Design Bending Moment

Area of tension reinforcement provided

Area of compression reinforcement provided

Effective depth to tension rein' (d_{n1})= $d-\Delta c$;

K Factor = $M/(b \cdot d_{n1}^2 \cdot f_{ck})$;

Limiting k Factor = 0.5986-0.1862-0.21;

Lever arm (z) = $\text{Min}(0.95 \cdot d_{n1}, d_{n1}/2 \cdot (1 + \sqrt{1-3.53k}))$;

Area of rein' reqd for bending ($A_{s,req,b}$) = $M/(f_{yd} \cdot z)$;

Min area reqd ($A_{s,min}$) = $\max(0.26 \cdot (f_{ctm}/f_{yk}), 0.0013) \cdot b \cdot d_{n1}$;

where $k > k'$ Lever arm (z) = $d_{n1}/2 \cdot (1 + \sqrt{1-3.53k'})$;

Depth to compression reinforcement (d_{n2})= $d-\Delta c$;

Compression Moment (M')= $b \cdot d_{n1}^2 \cdot f_{ck} \cdot (k-K')$;

Compression reinforcement reqd (A_s')= $M'/(f_{yd} \cdot (d_{n1}-d_{n2}))$;

Tension reinforcement required (A_s) = $(M-M')/f_{yd} \cdot z + A_s'$;

Tension reinforcement required at section ($A_{s,req}$)

Compression reinforcement required at section ($A_{s',req}$)

Back face in Tension

h_1	h_2	h_3	h_Z
12.1 kNm	14.6 kNm	15.6 kNm	12.1 kNm
0.7 kNm	7.1 kNm	13.6 kNm	0.3 kNm
479 mm ²	479 mm ²	479 mm ²	479 mm ²
0 mm ²	479 mm ²	958 mm ²	0 mm ²
61 mm	73 mm	79 mm	61 mm
0.004	0.030	0.049	0.002
0.208	0.208	0.208	0.208
58 mm	70 mm	75 mm	58 mm
29 mm ²	235 mm ²	418 mm ²	13 mm ²
120 mm ²	145 mm ²	156 mm ²	120 mm ²
Pass (3.9)	Pass (2)	Pass (1.1)	Pass (3.9)
0.1N/mm ²	0.1N/mm ²	0.1N/mm ²	0.1N/mm ²
2.0000	2.0000	2.0000	2.0000
0.0080	0.0070	0.0060	0.0080
40.2 kN	48.6 kN	52.4 kN	40.2 kN
47.9 kN	55.5 kN	56.8 kN	47.9 kN
4.0 kN	14.5 kN	22.4 kN	1.9 kN
Pass (9.9)	Pass (3.8)	Pass (2.5)	Pass (9.9)

Shear

Shear resistance constant ($C_{Rd,c}$) = $0.18/\gamma_c$;

Effective Depth Factor (k) = $\text{min}(2.0, 1 + \sqrt{200/d_{n1}})$;

Reinforcement Ratio (ρ_1) = $\text{min}(0.02, A_s/b \cdot d_{n1})$;

Minimum shear resistance ($V_{Rd,c,min}$) = $0.035 \cdot k^{1.5} \cdot f_{ck}^{0.5} \cdot b \cdot d_{n1}$;

Shear resistance ($V_{Rd,c,1}$) = $\max(V_{Rd,c,min}, C_{Rd,c} \cdot k \cdot (100 \cdot \rho_1 \cdot (f_{ck}/1))^{0.333} \cdot b \cdot d_{n1})$;

Shear resistance required (V)

Front face in Tension

Moment

Moment capacity of section

Design Bending Moment

Area of tension reinforcement provided

Area of compression reinforcement provided

Effective depth to tension rein' (d_{n1})= $d-\Delta c$;

K Factor = $M/(b \cdot d_{n1}^2 \cdot f_{ck})$;

Limiting k Factor = 0.5986-0.1862-0.21;

Lever arm (z) = $\text{Min}(0.95 \cdot d_{n1}, d_{n1}/2 \cdot (1 + \sqrt{1-3.53k}))$;

Area of rein' reqd for bending ($A_{s,req,b}$) = $M/(f_{yd} \cdot z)$;

Min area reqd ($A_{s,min}$) = $\max(0.26 \cdot (f_{ctm}/f_{yk}), 0.0013) \cdot b \cdot d_{n1}$;

where $k > k'$ Lever arm (z) = $d_{n1}/2 \cdot (1 + \sqrt{1-3.53k'})$;

Depth to compression reinforcement (d_{n2})= $d-\Delta c$;

Compression Moment (M')= $b \cdot d_{n1}^2 \cdot f_{ck} \cdot (k-K')$;

Compression reinforcement reqd (A_s')= $M'/(f_{yd} \cdot (d_{n1}-d_{n2}))$;

Tension reinforcement required (A_s) = $(M-M')/f_{yd} \cdot z + A_s'$;

Tension reinforcement required at section ($A_{s,req}$)

Compression reinforcement required at section ($A_{s',req}$)

h_1	h_2	h_3	h_Z
7.9 kNm	14.6 kNm	31.3 kNm	7.5 kNm
1.2 kNm	7.2 kNm	13.7 kNm	0.6 kNm
479 mm ²	479 mm ²	958 mm ²	479 mm ²
0 mm ²	479 mm ²	479 mm ²	0 mm ²
41 mm	73 mm	79 mm	38 mm
0.016	0.030	0.049	0.010
0.208	0.208	0.208	0.208
38 mm	70 mm	75 mm	36 mm
70 mm ²	236 mm ²	420 mm ²	41 mm ²
80 mm ²	145 mm ²	156 mm ²	75 mm ²
Pass (5.9)	Pass (2)	Pass (2.2)	Pass (6.3)
0.1N/mm ²	0.1N/mm ²	0.1N/mm ²	0.1N/mm ²
2.0000	2.0000	2.0000	2.0000
0.0120	0.0070	0.0120	0.0130
26.9 kN	48.6 kN	52.4 kN	25.2 kN
36.7 kN	55.5 kN	71.5 kN	35.3 kN
4.0 kN	14.6 kN	22.5 kN	2.8 kN
Pass (9.2)	Pass (3.7)	Pass (3.1)	Pass (9.9)

Shear

Shear resistance constant ($C_{Rd,c}$) = $0.18/\gamma_c$;

Effective Depth Factor (k) = $\text{min}(2.0, 1 + \sqrt{200/d_{n1}})$;

Reinforcement Ratio (ρ_1) = $\text{min}(0.02, A_s/b \cdot d_{n1})$;

Minimum shear resistance ($V_{Rd,c,min}$) = $0.035 \cdot k^{1.5} \cdot f_{ck}^{0.5} \cdot b \cdot d_{n1}$;

Shear resistance ($V_{Rd,c,1}$) = $\max(V_{Rd,c,min}, C_{Rd,c} \cdot k \cdot (100 \cdot \rho_1 \cdot (f_{ck}/1))^{0.333} \cdot b \cdot d_{n1})$;

Shear resistance required (V)

6.4 Base Reinforcement Design

	Top face in Tension		
	L_1	L_2	L_3
Moment			
Moment capacity of section	15.8 kNm	13.8 kNm	12.1 kNm
Design Bending Moment	2.9 kNm	1.4 kNm	4.8 kNm
Area of tension reinforcement provided	479 mm²	479 mm²	479 mm²
Area of compression reinforcement provided	958 mm²	479 mm²	479 mm²
Effective depth to tension rein' (d _{n1})=d-Δc;	79 mm	69 mm	61 mm
K Factor =M/(b•dn1²•f _{ck});	0.010	0.006	0.029
Limiting k Factor = 0.5986-0.1862-0.21;	0.208	0.208	0.208
Lever arm (z) =Min(0.95•d _{n1} ,d _{n1} /2•(1+√(1-3.53k)));	76 mm	66 mm	58 mm
Area of rein' reqd for bending (A _{s,req,b}) =M/(f _{yd} •z);	88 mm²	48 mm²	191 mm²
Min area reqd (A _{s,min}) = max(0.26•(f _{ctm} /f _{yk}),0.0013)•b•d _{n1} ;	157 mm²	137 mm²	120 mm²
where k>k'	Lever arm (z) =d _{n1} /2•(1+√(1-3.53k'));		
	Depth to compression reinforcement (d _{n2})=d-Δc;		
	Compression Moment (M')=b•d _{n1} ²•f _{ck} •(k-K');		
	Compression reinforcement reqd (A _{s'})=M'/(f _{yd} •(d _{n1} -d _{n2}));		
	Tension reinforcement required (A _s) =(M-M')/f _{yd} •z+As';		
Tension reinforcement required at section (A _{s,req})	157 mm²	137 mm²	191 mm²
Compression reinforcement required at section (A _{s',req})	0 mm²	0 mm²	0 mm²
	Pass (3)	Pass (3.4)	Pass (2.5)
Shear			
Shear resistance constant (C _{Rd,c}) =0.18/γ _c ;	0.1N/mm²	0.1N/mm²	0.1N/mm²
Effective Depth Factor (k) =min(2.0,1+√(200/dn1));	2.0000	2.0000	2.0000
Reinforcement Ratio (ρ ₁) =min(0.02,A _s /b•d _{n1});	0.0060	0.0070	0.0080
Minimum shear resistance (V _{Rd,c,min})= 0.035•k ^{1.5} •f _{ck} ^{0.5} •b•d _{n1} ;	52.8 kN	46.0 kN	40.3 kN
Shear resistance (V _{Rd,c,1}) = max(V _{Rd,c,min} ,C _{Rd,c} •k•(100•ρ ₁ •(f _{ck} /1))0.333•b•d _{n1});	57.2 kN	52.4 kN	48.0 kN
Shear resistance required (V)	11.2 kN	10.6 kN	10.6 kN
	Pass (5.1)	Pass (4.9)	Pass (4.5)
	Bottom face in Tension		
	L_1	L_2	L_3
Moment capacity of section	31.7 kNm	13.8 kNm	12.1 kNm
Design Bending Moment	0.0 kNm	0.0 kNm	0.0 kNm
Area of tension reinforcement provided	958 mm²	479 mm²	479 mm²
Area of compression reinforcement provided	479 mm²	479 mm²	479 mm²
Effective depth to tension rein' (d _{n1})=d-Δc;	79 mm	69 mm	61 mm
K Factor =M/(b•dn1²•f _{ck});	0.000	0.000	0.000
Limiting k Factor = 0.5986-0.1862-0.21;	0.208	0.208	0.208
Lever arm (z) =Min(0.95•d _{n1} ,d _{n1} /2•(1+√(1-3.53k)));	76 mm	66 mm	58 mm
Area of rein' reqd for bending (A _{s,req,b}) =M/(f _{yd} •z);	0 mm²	0 mm²	0 mm²
Min area reqd (A _{s,min}) = max(0.26•(f _{ctm} /f _{yk}),0.0013)•b•d _{n1} ;	157 mm²	137 mm²	120 mm²
where k>k'	Lever arm (z) =d _{n1} /2•(1+√(1-3.53k'));		
	Depth to compression reinforcement (d _{n2})=d-Δc;		
	Compression Moment (M')=b•d _{n1} ²•f _{ck} •(k-K');		
	Compression reinforcement reqd (A _{s'})=M'/(f _{yd} •(d _{n1} -d _{n2}));		
	Tension reinforcement required (A _s) =(M-M')/f _{yd} •z+As';		
Tension reinforcement required at section (A _{s,req})	157 mm²	137 mm²	120 mm²
Compression reinforcement required at section (A _{s',req})	0 mm²	0 mm²	0 mm²
	Pass (6.1)	Pass (3.4)	Pass (3.9)
Shear			
Shear resistance constant (C _{Rd,c}) =0.18/γ _c ;	0.1N/mm²	0.1N/mm²	0.1N/mm²
Effective Depth Factor (k) =min(2.0,1+√(200/dn1));	2.0000	2.0000	2.0000
Reinforcement Ratio (ρ ₁) =min(0.02,A _s /b•d _{n1});	0.0120	0.0070	0.0080
Minimum shear resistance (V _{Rd,c,min})= 0.035•k ^{1.5} •f _{ck} ^{0.5} •b•d _{n1} ;	52.8 kN	46.0 kN	40.3 kN
Shear resistance (V _{Rd,c,1}) = max(V _{Rd,c,min} ,C _{Rd,c} •k•(100•ρ ₁ •(f _{ck} /1))0.333•b•d _{n1});	72.0 kN	52.4 kN	48.0 kN
Shear resistance required (V)	11.2 kN	10.6 kN	10.6 kN
	Pass (6.4)	Pass (4.9)	Pass (4.5)

Concrete section is sufficiently reinforced