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Proposed Residential Development
Land Off Hay Green Lane, Birdwell
for Harworth Group
Transport Assessment

January 2021 (Revision 3)

Quality Management

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Contents

1. Introduction.....	3
2. Existing Site Conditions.....	5
3. Proposed Development and Access Strategy.....	29
4. Mitigation Measures for Local Road Network.....	31
5. Site Accessibility and Measures to Promote Sustainable Travel.....	33
6. Development Trip Generations and Distributions.....	35
7. Traffic Flows and Development Impact.....	38
8. Summary and Conclusions.....	44

IMAGES

Image 2.1	Site Location.....	5
Image 2.2	Parking Bay on Hay Green Lane.....	6
Image 2.3	School Parking Restrictions.....	6
Image 2.4	School Parking Restrictions.....	7
Image 2.5	Pedestrian Refuge Island on A61 Sheffield Road.....	8
Image 2.6	Zebra Crossing on A61 Sheffield Road (Google Maps).....	8
Image 2.7	BMBC Public Rights of Way Extract.....	9
Image 2.8	Extract of BMBC Cycle Map.....	10
Image 2.9	Northbound Bus Stop.....	11
Image 2.10	Southbound Bus Stop.....	11
Image 2.11	Existing Bus Services.....	12
Image 2.12	Parking Beat Survey Extent.....	17
Image 2.13	A61 Sheffield Road Parking Accumulation – AM.....	18
Image 2.14	A61 Sheffield Road Parking Accumulation – PM.....	19
Image 2.15	Hay Green Lane Parking Accumulation – AM.....	19
Image 2.16	Hay Green Lane Parking Accumulation – PM.....	20
Image 2.17	Birdwell Venue Parking Accumulation – AM.....	20
Image 2.18	Birdwell Venue Parking Accumulation – PM.....	21
Image 2.19	Total Parking Accumulation – AM.....	22
Image 2.20	Total Parking Accumulation – PM.....	22
Image 2.21	Accident Data (Crashmap).....	27
Image 2.22	Accident Data (BMBC).....	27
Image 6.1	Barnsley 028C Lower Layer Super Output Area.....	36

TABLES

Table 2.1	Existing Bus Services.....	13
Table 2.2	2019 Traffic Surveys.....	14
Table 2.3	2020 Traffic Surveys.....	15
Table 2.4	Comparison between 2019-2020 PM Peak Traffic Flows.....	16
Table 2.5	Summary of SCP Results.....	25
Table 2.6	Summary of Zebra Crossing Results.....	26
Table 6.1	Summary of Trip Rates for Hay Green Lane.....	35
Table 6.2	Proposed Trip Generation.....	35
Table 6.3	Residential Mode Split – Barnsley 028C.....	35
Table 6.4	AM Peak Multi Modal Trip Generations.....	36
Table 6.5	PM Peak Multi Modal Trip Generations.....	37
Table 7.1	Summary of Modelling Outputs.....	40

TRAFFIC FLOW FIGURES

Figure 10	AM 2019 Count
Figure 11	PM 2019 Count
Figure 12	Trip Distribution
Figure 13	AM Trip Generation
Figure 14	PM Trip Generation
Figure 15	AM Committed Hotel Development
Figure 16	PM Committed Hotel Development
Figure 21	AM Total Hoyland Consented Development
Figure 22	PM Total Hoyland Consented Development
Figure 23	AM 2019 Base Flows
Figure 24	PM 2019 Base Flows
Figure 25	AM 2025 Base Flows
Figure 26	PM 2025 Base Flows
Figure 27	AM 2025 Design Flows
Figure 28	PM 2025 Design Flows
Figure 29	AM 2025 Design Sensitivity Flows
Figure 30	PM 2025 Design Sensitivity Flows
Figure 31	AM 2033 Base Flows
Figure 32	PM 2033 Base Flows
Figure 33	AM 2033 Design Flows
Figure 34	PM 2033 Design Flows
Figure 35	AM 2033 Design Sensitivity Flows
Figure 36	PM 2033 Design Sensitivity Flows

APPENDICES

Appendix A	Masterplan
Appendix B	Surveys
Appendix C	Accident Data
Appendix D	Visibility Splay at Access
Appendix E	Swept Paths
Appendix F	Mitigation Proposals
Appendix G	Census
Appendix H	Committed Development Traffic
Appendix I	Turning Movements & Modelling Outputs
Appendix J	Scoping Note

Table 7.2	Summary of Delay Information for the Hourly Flow Modelling	41
Table 7.3	Summary of Sensitivity Modelling Outputs	42
Table 7.4	Summary of Delay Information 15 min input	43

FIGURES

Figure 1	Site Location Plan
Figure 2	Pedestrian Accessibility
Figure 3	Cycle Accessibility
Figure 4	Bus Stop Accessibility

1. Introduction

1.1.1 This Transport Assessment (TA) has been prepared on behalf of Harworth Group to support a planning application for residential development on a Site to the south of Hay Green Lane, Birdwell. The scheme proposals which are illustrated on the Architects plans contained at Appendix A can be summarised as follows:

- 118 dwellings;
- Vehicular Site access via Hay Green Lane;
- Public Open Space; and
- Design of internal layout to a maximum speed of 20mph.

1.1.2 The Site is located in Birdwell, Barnsley, is currently vacant and is allocated for residential development in the Barnsley Adopted Local Plan under reference HS59.

1.1.3 The Site is rectangular in shape and is bound by both Hay Green Lane and existing dwellings to the north and west, and agricultural land to the east and south.

1.1.4 Pre-application discussions have been held with Barnsley Metropolitan Borough Council (BMBC) where the scope of the TA and TP were discussed. It is also highlighted that whilst there are no significant approved developments within the vicinity of the Site, the Hoyland North and Hoyland West masterplan sites to the west of the proposed Site could have a realistic impact on through movements along the A61, as such the potential impact of these developments has been considered.

1.1.5 The scoping response, attached at Appendix J, provided by Officers has been considered in the preparation of this report.

1.1.6 It should be noted that traffic surveys of the local highway network were undertaken in November 2019, prior to the scoping meeting held on 13th January 2020. At the scoping meeting it was requested that parking beat surveys were undertaken to determine the extent of parking associated with the school drop off. Due to COVID-19 these were unable to be undertaken until September 2020 when schools re-opened. As such this revision includes survey data that was not included in the original issue.

1.1.7 It was, and is, deemed appropriate that the mitigation measures were devised without parking beat surveys as highlighted in the original issue of this report and set out below:

- The mitigation measures proposed are considered to be the most that can be offered by the application due to the constraints faced;
- The mitigation proposals are seeking to resolve an existing issue i.e. it is not the responsibility of the application to resolve such matters but in the interest of improving the situation for all, including school users and local residents the application is seeking to assist.

1.1.8 The following sections of this Transport Assessment report cover the following topics:

- Chapter 2 – describes the Site and existing transport conditions;
- Chapter 3 – defines the development proposals including access strategy;
- Chapter 4 – Discusses the mitigation measures proposed;
- Chapter 5 – describes the accessibility of the Site and the measures to encourage sustainable travel;

- Chapter 6 – sets out the trip generation and distribution methodologies applied in the assessment of the highway network;
- Chapter 7 – describes the traffic flow information and provides an assessment of the impact of the Site on the local highway network; and
- Chapter 8 – summarises and concludes the report.

2. Existing Site Conditions

2.1.1 This chapter describes the Site and considers the baseline conditions on the surrounding highway network for a range of transport modes.

2.2 Existing Site

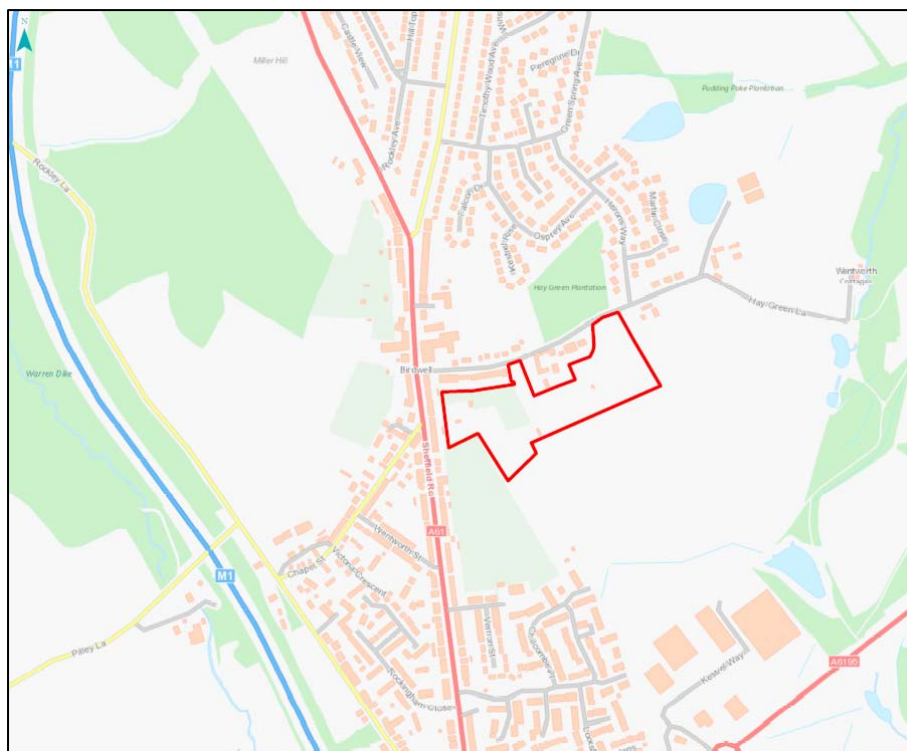
2.2.1 The development Site is located approximately 7.3km south of Barnsley, located within the Barnsley Local Authority Boundary.

2.2.2 The Site is bound by existing dwellings to the north and west and agricultural fields to the south and east.

2.2.3 The Site is allocated within the emerging core strategy under reference HS59 for residential use.

2.2.4 The Site in relation to the strategic and local transport networks is shown on Figure 1. An extract of Figure 1 is shown in Image 2.1.

Image 2.1 Site Location



2.3 Existing Local Highway Network

2.3.1 Hay Green Lane is a single carriageway road with a width of circa 5.7m with a circa 1.8m footway on either side of the highway, with the exception of the proposed Site access where the westbound carriageways footway is reduced to under 1m in width.

2.3.2 Hay Green Lane is street lit and is subject to a 30mph speed limit and has no parking restrictions in place.

2.3.3 Birdwell Primary School bounds the northern extent of Hay Green Lane on the eastbound carriageway where a side pedestrian access is provided.

2.3.4 There are two staggered parking bays on the northbound and southbound carriageway on Hay Green Lane, approximately 45m and 60m in length respectively, as shown in Image 2.2.

Image 2.2 Parking Bay on Hay Green Lane



2.3.5 A ghost island priority junction connects Hay Green Lane to A61 Sheffield Road to the east of the Site. Tactile paving and dropped crossings are provided on Hay Green Lane at the junction with A61 Sheffield Road, as seen in Image 2.3.

Image 2.3 School Parking Restrictions



2.3.6 The A61 Sheffield Road is single carriageway road with a width of circa 9.8m with a circa 2.3m footway on either side of the highway, as a local distributor highway it provides connections from the M1 J36 to the south and routes on a north-south axis to Barnsley centre through a number of residential areas including Birdwell and Worsbrough.

2.3.7 A61 Sheffield Road is subject to a 30mph speed limit and is street lit. The only parking restrictions in place are outside the school (Mon-Fri, 8am-5pm) which is located on the northern arm of the junction, as seen in Image 2.4.

Image 2.4 School Parking Restrictions



2.3.8 There is a pedestrian crossing with refuge island located approximately 35m south of the junction with Hay Green Lane with the provision of tactile paving, which is shown in Image 2.5.

Image 2.5 Pedestrian Refuge Island on A61 Sheffield Road



2.3.9 Approximately 115m south of the junction with Hay Green Lane a zebra crossing is provided on A61 Sheffield Road as shown in Image 2.6.

Image 2.6 Zebra Crossing on A61 Sheffield Road (Google Maps)



2.4 Existing School Facilities

2.4.1 As aforementioned, Birdwell Primary School bounds the northern extent of Hay Green Lane. In order to provide safe crossing for school users a school crossing patrol officer in place for the start and end of school times.

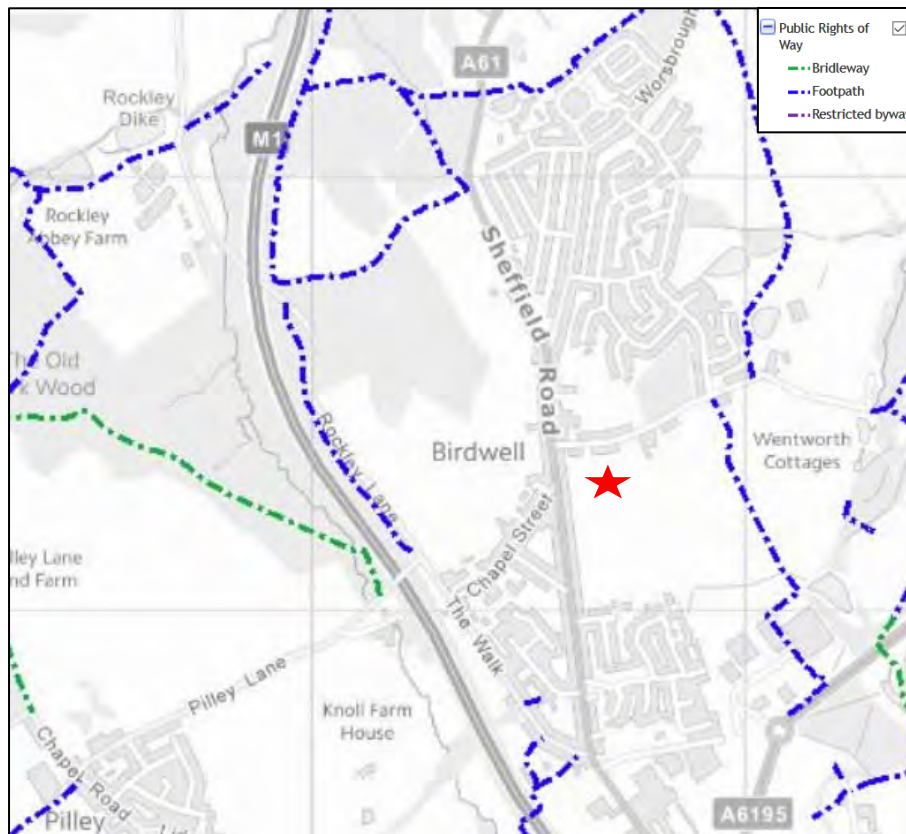
2.4.2 Whilst no designated parking is available for drop-off / pick-up facilities, the school has an agreement with the Birdwell Venue (BV) on the west of A61 Sheffield Road to utilise the parking spaces for school drop off/pick up.

2.4.3 Hay Green Lane has no parking restrictions nor does Sheffield Road, excluding the restrictions directly adjacent to the school enforced by zigzag markings as shown in Image 2.4, Sheffield Road within the vicinity of the school also has unrestricted parking.

2.5 Public Rights of Way

2.5.1 The public rights of way in the vicinity of the Site are shown in Image 2.7.

Image 2.7 BMBC Public Rights of Way Extract

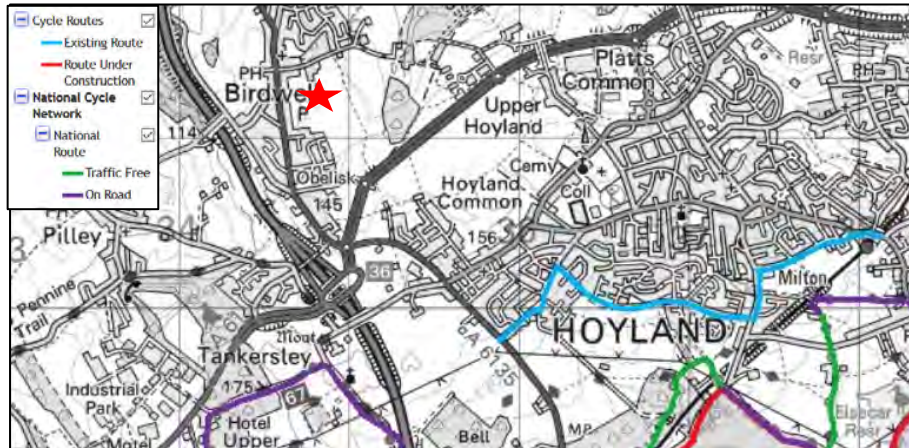


2.5.2 There is a public right of way that runs in close proximity to the eastern extent of the Site. A pedestrian link will be provided within the development proposals to connect to the public right of way, this is shown on the masterplan attached at Appendix A.

2.6 Cycle Infrastructure

2.6.1 BMBC have a network of cycle infrastructure in addition to the national cycle network. The cycle infrastructure in the vicinity of the Site is shown on Image 2.7.

Image 2.8 Extract of BMBC Cycle Map



2.7 Existing Bus Infrastructure

2.7.1 The nearest bus stops to the Site are located on A61 Sheffield Road some 350m west (as the crow flies) of the Site access junction. The northbound bus stop (37055027) is provided with a shelter, seating, raised kerbs, bus stop clearways and timetable information. The southbound bus stop (37055401) is provided with a bus shelter, raised kerbs, bus stop clearways and timetable information. The bus stops are shown in Images 2.9 and 2.10.

Image 2.9 Northbound Bus Stop



Image 2.10 Southbound Bus Stop



2.7.2 The existing services in the vicinity of the Site are shown in Image 2.10 and with a summary of the services provided in Table 2.1.

Image 2.11 Existing Bus Services

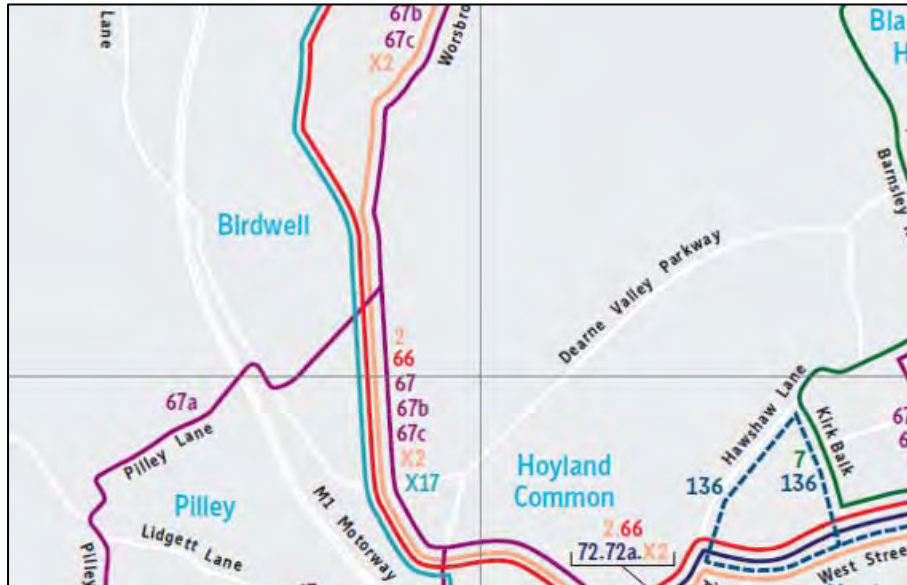


Table 2.1 Existing Bus Services

Service	Route	Day of Operation	Time of Operation	One Way Service Frequency
2 / X2	Barnsley - Sheffield	Weekday	06:07 – 22:55	2 per hour
		Saturday	07:37 – 22:55	2 per hour
		Sunday	08:49 – 22:55	1 per hour
2 / X2	Sheffield - Barnsley	Weekday	05:57 – 23:13	2 per hour
		Saturday	06:20 – 23:13	2 per hour
		Sunday	09:21 – 23:13	1 per hour
66	Barnsley Interchange – Barnsley Interchange via Elsecar	Weekday	06:00 – 23:40	4 per hour
		Saturday	06:20 – 23:40	4 per hour
		Sunday	08:30 – 23:40	2 per hour
67/67b/67c	Barnsley - Wombwell	Weekday	05:25 – 23:15	2 per hour
		Saturday	05:25 – 23:15	2 per hour
		Sunday	08:17 – 22:34	1 per hour
67/67b/67c	Wombwell - Barnsley	Weekday	04:25 – 23:15	2 per hour
		Saturday	04:25 – 23:15	2 per hour
		Sunday	08:35 – 23:32	1 per hour
X17	Barnsley – Sheffield - Matlock	Weekday	06:50 – 20:48	1 per hour
		Saturday	07:15 – 20:48	1 per hour
		Sunday	09:40 – 14:47	1 per hour
X17	Matlock – Sheffield - Barnsley	Weekday	06:05 – 22:50	1 per hour
		Saturday	08:14 – 22:50	1 per hour
		Sunday	09:14 – 19:14	1 per hour

2.7.3 During the weekday and Saturday, the services set out above combine to provide an overall frequency of 14 buses per hour travelling in each direction departing from the local bus stops on A61 Sheffield Road.

2.7.4 The bus services from these stops provide access to the following destinations:

- Barnsley – 17 minute journey, every 15 minutes;
- Sheffield – 47 minute journey, every 25 minutes;
- Elsecar – 24 minute journey, every 15 minutes; and
- Wombwell – 32 minute journey, every 60 minutes.

2.7.5 The existing bus services will provide residents of the Site with high frequency access to the centres of Barnsley and Sheffield and the large transport interchanges at both towns for travel further afield.

2.8 Existing Rail Services/Facilities

2.8.1 The nearest train station to the Site is situated at Elsecar on the Elland and Penistone Line. It is located an approximate 20 minute cycling distance from the Site as shown in Figure 3.

2.8.2 Trains from Elsecar serve destinations including Barnsley, Leeds, Wakefield, Huddersfield and Sheffield among others.

2.8.3 Elsecar Station benefits from cycle storage, ticket machines, CCTV and step-free access.

2.9 Traffic Surveys

2.9.1 Turning Count Surveys have been undertaken by an independent surveyor and are summarised in Table 2.2.

Table 2.2 2019 Traffic Surveys

Location	Type	Date	Assessment Period
Green Spring Avenue / Herons Way	Turning Counts	27 th November 2019	AM Peak (06:30 – 09:30) PM Peak (16:30 – 19:30)
Hay Green Way/ Herons Way	Turning Counts	27 th November 2019	AM Peak (06:30 – 09:30) PM Peak (16:30 – 19:30)
A61 Sheffield Road / Hay Green Way	Turning Counts	27 th November 2019	AM Peak (06:30 – 09:30) PM Peak (16:30 – 19:30)
A61 Sheffield Road / Worsbrough Road /	Turning Counts	27 th November 2019	AM Peak (06:30 – 09:30) PM Peak (16:30 – 19:30)

2.10 Updated Surveys

2.10.1 Surveys at school drop off times were not initially undertaken prior to the commencement of this TA due to COVID-19 restrictions and school closures. However, surveys were requested which were undertaken during September 2020 during a period of relaxed COVID 19 restrictions and the primary school was open. An extensive analysis of the surveys has been undertaken and is presented below.

2.10.2 Table 2.3 summarises the 2020 surveys with the full outputs available in Appendix B.

Table 2.3 2020 Traffic Surveys

Location	Type	Date	Assessment Period
Green Spring Avenue / Herons Way	Turning Counts	23 rd September 2020	PM Peak (14:30 – 18:30)
Hay Green Way/ Herons Way	Turning Counts	23 rd September 2020	PM Peak (16:30 – 19:30)
A61 Sheffield Road / Hay Green Way	Turning Counts	22 nd September 2020	PM Peak (16:30 – 19:30)
A61 Sheffield Road / Worsbrough Road /	Turning Counts	22 nd September 2020	AM Peak (06:30 – 09:30) PM Peak (16:30 – 19:30)
A61 Sheffield Road	Parking Survey	15 th September 2020	AM Peak (07:30 – 09:30) PM Peak (14:30 – 17:00)
Hay Green Lane	Parking Survey	15 th September 2020	AM Peak (07:30 – 09:30) PM Peak (14:30 – 17:00)
Birdwell Venue	Parking Survey	15 th September 2020	AM Peak (07:30 – 09:30) PM Peak (14:30 – 17:00)
A61 Sheffield Road School Crossing Patrol	Pedestrian Survey	16 th September 2020	AM Peak (08:30 – 09:00) PM Peak (15:00 – 15:40)
A61 Sheffield Road Zebra Crossing	Pedestrian Survey	16 th September 2020	AM Peak (07:30 – 09:30) PM Peak (14:30 – 17:00)

2.10.3 A comparison of the 2019 and 2020 traffic flows along A61 Sheffield Road, past Hay Green Lane is shown in Table 2.4 below, as it shows the traffic flow from 2020 is lower than that of 2019 which is likely to be as a result of reduced travel due to COVID-19.

2.10.4 As such the traffic flows from 2019 have been used in capacity modelling in this TA to provide a robust worst case scenario.

Table 2.4 Comparison between 2019-2020 PM Peak Traffic Flows

	PM Peak (16:30 – 17:30) Two Way Traffic Flow
2019 Count	1645
2020 Count	1612

2.11 Parking Accumulation

2.11.1 It is appreciated that there are concerns from local residents regarding the proposed development and the impact it could have on the current issue surrounding school drop off and pick up operation.

2.11.2 To assess the current parking accumulation comprehensive parking beat surveys were undertaken every 5 minutes on Hay Green Lane, A61 Sheffield Road and at the Birdwell Venue on Tuesday 15th September 2020 between the peak hours following hours:

- AM Peak – 7:30-09:30
- PM Peak – 14:30-17:00

2.11.3 Image 2.12 shows the Parking Beat Survey area, which is shown on 18079.IN.01 attached at Appendix B.

Image 2.12 Parking Beat Survey Extent



2.11.4 The extent of Hay Green Lane that has been surveyed is the entirety of the terraced housing where parking is available for the majority of the carriageway on both sides of the carriageway. There are laybys on either side of Hay Green Lane, initially 10 spaces are in the layby adjacent to Birdwell Primary School on the eastbound carriageway, a further 13 spaces are in the layby on the westbound carriageway.

2.11.5 This extent of the survey is deemed appropriate as it was found that there is parking demand for school drop off/pick up away from the School frontage and by including the whole street allows for the interactions between the school operation and other parking on Hay Green Lane to be shown. It also allows for an appropriate survey of the school related vehicle journeys.

2.11.6 For example, please see page 2 of the Hay Green Lane Surveys in Appendix B and the numbered Site Plan enclosed at Appendix B, parking spaces 16-28 (highlighted in a red box) which are located away from frontage of the school show

the spaces are occupied during school drop-off / pick-up time frames and so can reasonably be assumed to be associated with school drop off.

2.11.7 Car Parking Spaces 32, 33, 35 and 41 are occupied (highlighted in purple boxes) throughout the AM and PM survey period suggesting that the parking may not be related to the operation of the school.

2.11.8 In order to assess the parking accumulation throughout the survey periods the graphs in Images 2.13 – 2.18 have been produced which demonstrate the parking at each 5 minute interval on the A61 Sheffield Road, Hay Green Lane and at the Birdwell Venue throughout the survey periods.

2.11.9 A capacity line has been included to show the total capacity of parking available on each road which is as follows:

- A61 Sheffield Road: 48 Car Parking Spaces;
- Hay Green Lane: 54 Car Parking Spaces; and
- Birdwell Venue: 53 Car Parking Spaces.

Image 2.13 A61 Sheffield Road Parking Accumulation – AM

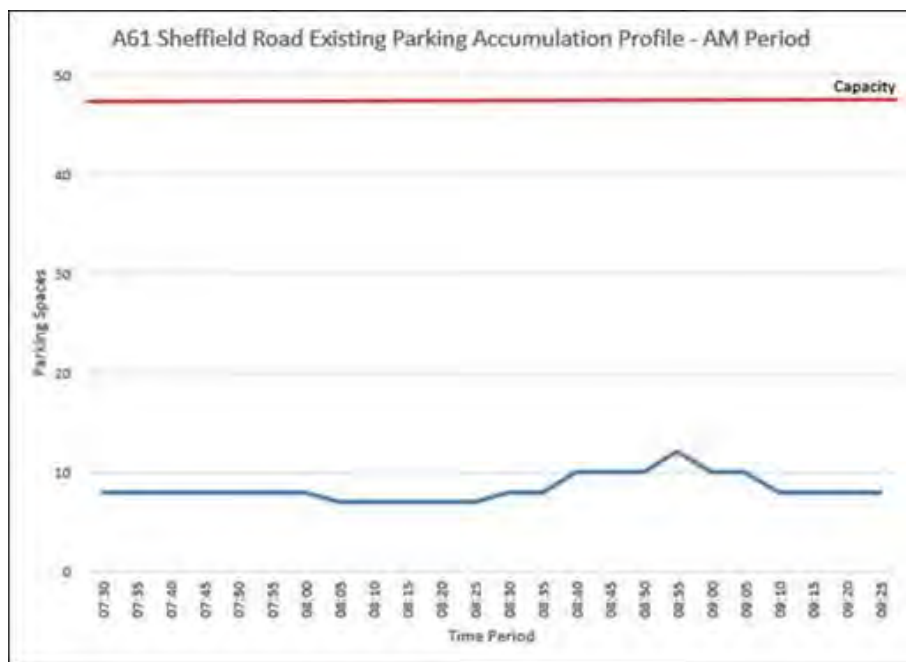


Image 2.14 A61 Sheffield Road Parking Accumulation – PM

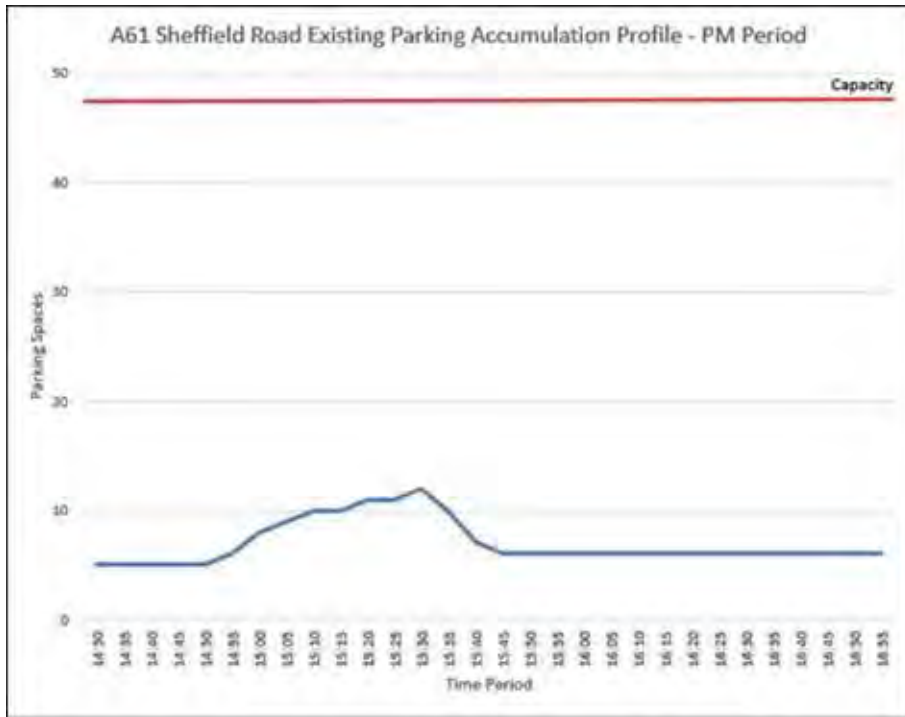


Image 2.15 Hay Green Lane Parking Accumulation – AM



Image 2.16 Hay Green Lane Parking Accumulation – PM

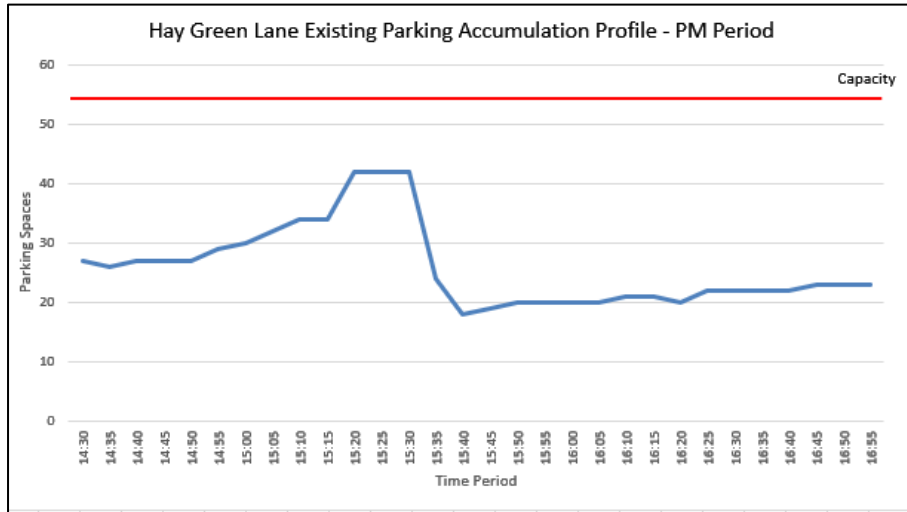


Image 2.17 Birdwell Venue Parking Accumulation – AM

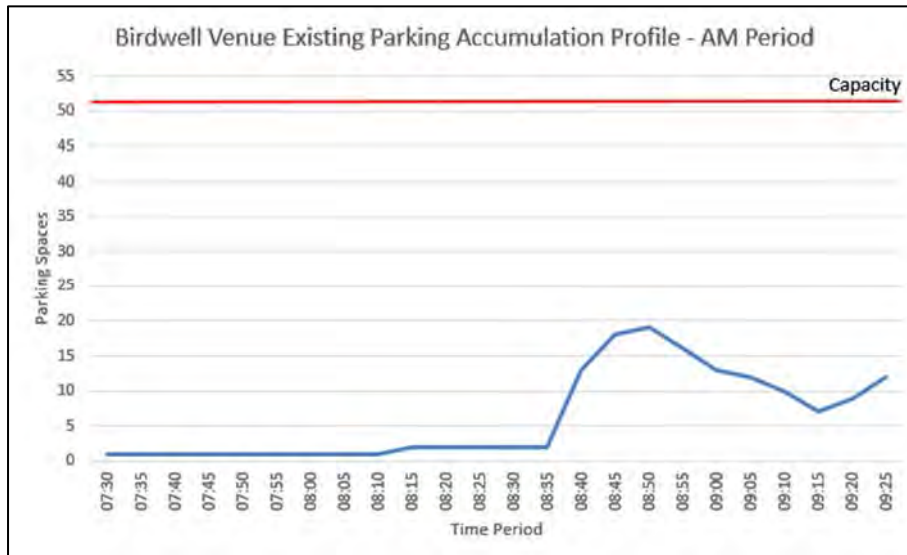
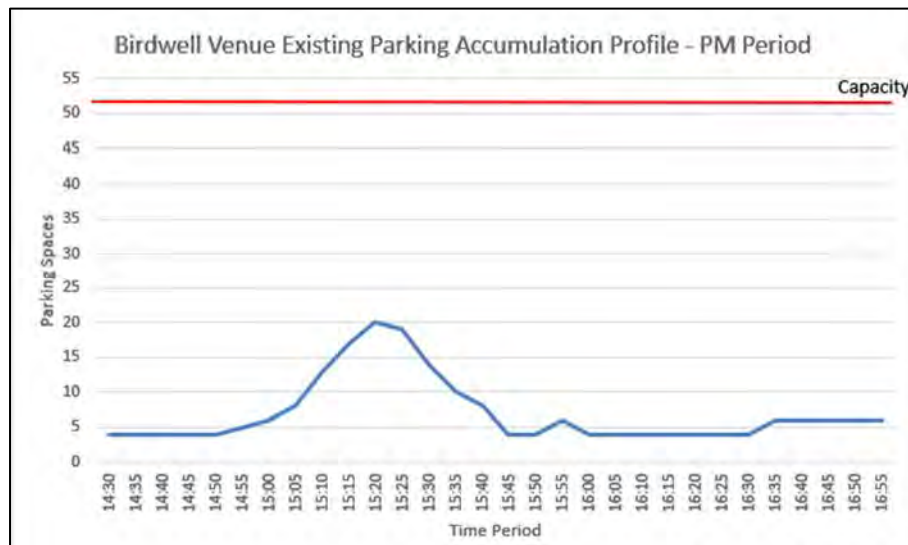


Image 2.18 Birdwell Venue Parking Accumulation – PM



2.11.10 As can be seen in the graphs, at no point in either peak periods does the parking accumulation reach or exceed the total parking capacity on the A61 Sheffield Road, Hay Green Lane or at the Birdwell Venue.

2.11.11 During the AM Peak period the peak parking demand was identified between 09:00-09:05 with 89% and 21% of the total parking accumulation being occupied on Hay Green Lane and A61 Sheffield Road respectively. The total parking accumulation at the Birdwell Venue during this peak period was 25%.

2.11.12 During the PM Peak period the peak parking demand was identified between 15:30-15:35 resulting in 86% and 25% of the total parking accumulation being occupied on Hay Green Lane and A61 Sheffield Road respectively. The total parking accumulation at the Birdwell Venue during this peak period was 26%.

2.11.13 Outside of the school drop off peak there is significant spare capacity with parking accumulation being circa 50% across both the AM and PM survey period on Hay Green Lane. On Sheffield road this is circa 15%, and 12% at the Birdwell Venue. This suggests that there is demand for parking outside the operation of the school's drop-off/pick-up and demonstrates that the school is not the sole parking generator in the area.

2.11.14 To remove school related parking from the accumulation would in effect increase the spare capacity as such the average parking accumulation includes these time periods, and is as follows:

- AM Survey Period – A61 Sheffield Road: 8
- PM Survey Period – A61 Sheffield Road: 7
- AM Survey Period – Hay Green Lane: 23
- PM Survey Period – Hay Green Lane: 26
- AM Survey Period – Bidwell Venue: 6
- PM Survey Period – Birdwell Venue: 7

2.11.15 During the AM Maximum Parking Demand period (09:00-09:05) there are 38 and 6 spare parking spaces on A61 Sheffield Road and Hay Green Lane

respectively, and 40 at the Birdwell Venue. Across the AM surveyed period the average number of spare parking spaces available is as follows:

- A61 Sheffield Road: 40
- Hay Green Lane: 33
- Birdwell Venue: 47

2.11.16 During the PM Maximum Parking Demand period(15:30-15:35) there are 36 and 14 spare parking spaces on A61 Sheffield Road and Hay Green Lane respectively, and 39 at the Birdwell Venue. Across the PM surveyed period the average number of spare parking spaces available is as follows:

- A61 Sheffield Road: 41
- Hay Green Lane: 30
- Birdwell Venue: 46

2.11.17 To summarise, Figures 2.19 and 2.20 below show the total parking accumulation across the three surveys site areas in the AM and PM Peak.

Image 2.19 Total Parking Accumulation – AM

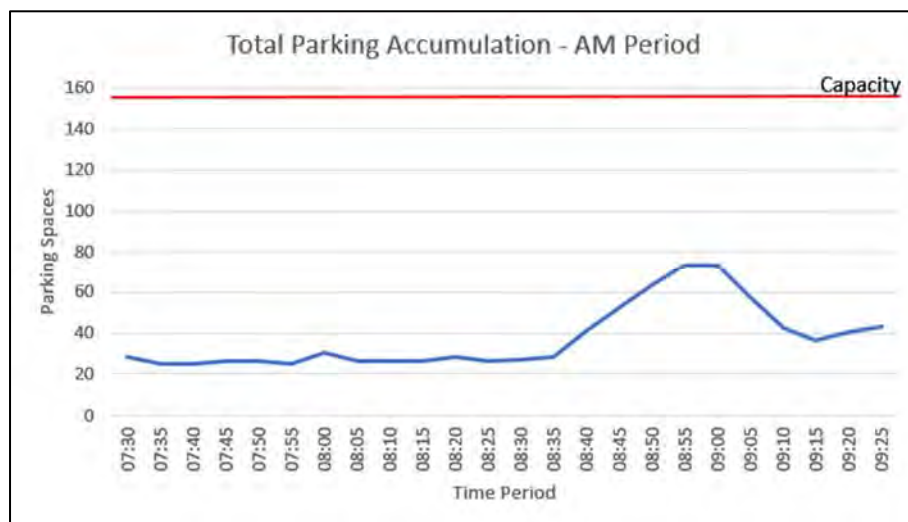
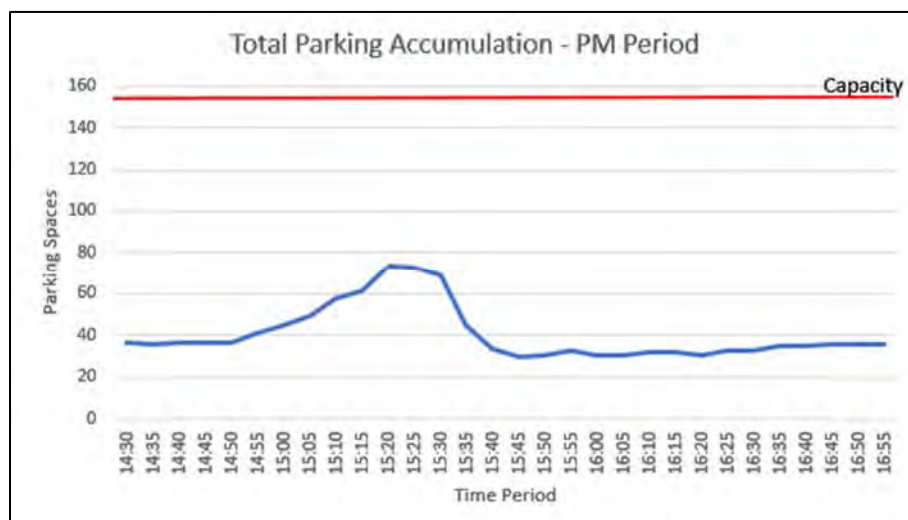


Image 2.20 Total Parking Accumulation – PM



2.11.18 The figures above demonstrate that across all three parking locations surveyed, at no point does the parking accumulation reach or exceed the total capacity available, moreover there is significant spare capacity in the survey periods

2.11.19 The above summary shows that the parking associated with the school is isolated in a very small window and is considered to represent a situation that is completely normal for a primary school located in a residential area. The accident data attached shows that no PIC's that have occurred coincide with the school opening and closing time periods.

2.11.20 The survey data for the parking beats is available in Appendix B. For clarity on how the data can be interpreted a worked example is provided. Please. For example the column that is highlighted yellow (parking space 11) in Appendix B, shows that a car was parked in this space in the AM Peak between 08:00-08:05 and 08:50-09:10 and again in the PM Peak between 14:30 and 15:40.

2.11.21 For GDPR reasons and sensitivities relating to the recording of information around primary schools, registration plates were not recorded (nor were they needed) during the parking beat survey. This resulted in detailed duration of stay per vehicle and parking purpose of that parked vehicle not being recorded.

2.11.22 However, it is possible to make a realistic and informed interpretation of the data to determine if a vehicle was parking to drop off/pick up from the school based on the time and duration a parking space was occupied. For example, see page 2 of the attached Birdwell Parking Surveys for Hay Green Lane attached. Note the column highlighted green (parking space 1) shows that a vehicle was parked in that space throughout the survey period and as such is very unlikely to be by a pickup/drop off. However, for spaces 12 through to 26 it can reasonably concluded that the parking activity in these spaces relates to school drop off, particularly in the morning peak.

2.11.23 Along Hay Green Lane there are 23 spaces situated in laybys on both sides of the carriageway. These are shown in the parking surveys (Appendix B) as spaces 1-10 and spaces 29-41.

2.11.24 The attached parking surveys shows through a highlighted blue band where it can be concluded that the majority of parking activity is associated with school drop off/pick up.

2.11.25 The activity around the bell mouth of the junction has also been analysed. Please see attached the Numbered Birdwell Site Plan. This shows the parking number references the correspond to the spreadsheets. Focus has been placed on on spaces 36 to 41 on Sheffield Road and 1 and 54 on Hay Green Lane.

Sheffield Road

2.11.26 Parking Spaces 36 to 39 – no activity at all in the survey period. 38 and 39 are on the junction radii. 40 has activity in the PM peak with vehicles being parked between 15:05 and 15:20 and then 15:25 to 15:35.

2.11.27 There is no parking in the visibility splays to the north and to the south there is activity in space 41 and 43 (south of the junction) in the morning peak but it is short lived and not of a concern because of the position of the pedestrian refuge meaning that visibility splays can be measured to the opposite running lane because vehicles are unlikely to be running on the wrong wide of the carriageway.

2.11.28 In the PM peak there is activity in parking spaces 40 and 41 but again this is south of the junction and so not of concerns.

2.11.29 The parking activity in described above occurs at school drop off times with short stay duration and is likely to be school related.

2.11.30 The improvement proposals in this area including measures to prevent parking on the highway and footway will only serve to enhance the smooth flow of vehicles and increase unobstructed visibility.

Hay Green Lane

2.11.31 Space 1 is occupied throughout the survey period and is therefore likely to be not likely to be related to the school drop-off/pick-up. Space 54 is occupied throughout the survey period and is therefore likely to be not likely to be related to the school drop-off/pick-up.

2.11.32 Based on the activity around the bell mouth of the junction this can partially be attributed to the school drop off but not on Hay Green Lane. Furthermore, there isn't activity on the radii and spaces 1 and 54 are situated behind the radii albeit within 10m of the junction and therefore cars are not parked in accordance with guidance set out in the Highway Code Rule 243.

2.11.33 It should be noted on the Birdwell Numbered Site Plan that there is a hatched area that is noted as A, B and C. This is to record drop off taking place from the centre of the carriageway. This occurred 5 times during the AM peak and twice during the PM peak.

2.11.34 This is likely to be associated with people running late and I do not propose there is a means of mitigating this. This frequency is small and focused around the isolated peak and does not require addressing through the development.

2.11.35 As part of the Travel Plan, information will be distributed via the school with the aim of decreasing parking within the area which will be accompanied by detail relating to traffic regulation orders, failure to comply could result in being reported to the South Yorkshire Police.

2.11.36 In summary the surveys have identified that there is parking activity particularly on Hay Green Lane but as demonstrated this is generally focused around a 15 minute period of the day during the school drop off / pickup in the AM and PM. At no time of the day does the parking reach the total capacity available on A61 Sheffield Road or Hay Green Lane.

2.11.37 The issues surrounding the parking are existing and the additional trips generated from the development are not considered material at 1 trip per minute.

Birdwell Venue

2.11.38 Parking demand was also been recorded at the Birdwell Venue on Wednesday 16th September 2020 at 5 minute intervals for the following time periods:

- AM Peak: 07:30 – 09:30
- PM Peak: 14:30 – 17:00

2.11.39 The raw data is available in Appendix B and summarised as follows:

- In the AM the peak accumulation of parked vehicles occurred between 08:50-08:55 where 19 vehicles were parked.
- In the PM the peak accumulation of parked vehicles occurred between 15:20-15:25 where 20 vehicles were parked

2.11.40 The first two tables in Appendix B labelled Birdwell Venue Car Park Survey (Total) include all surveyed vehicles entering and parking in the car park. The second two tables labelled Birdwell Venue Car Park Survey (North) account for the vehicles entering and exiting the car park from the north access only to demonstrate the use, this data is also included in the total tables.

2.12 Pedestrian Crossing Summary

2.12.1 To assess the use of the School Crossing Patrol Crossing (SCP) outside Birdwell Primary School and the existing Zebra Crossing south of the proposed Site crossing surveys were undertaken on Wednesday 16th September during the following peak hours:

SCP Crossing:

- AM Peak: 08:30 – 09:00
- PM Peak: 15:00 – 15:40

Zebra Crossing

- AM Peak: 07:30 – 09:00
- PM Peak: 14:30 - 17:00

2.12.2 Note, the SCP was surveyed for the duration of which it was in operation in both the AM and PM period, a record was also taken of the number of people using the SCP who were parked in the Birdwell Venue car park to assess the usage level. The full results are available in Appendix B with a summary provided in Table 2.5.

Table 2.5 Summary of SCP Results

AM (08:30-09:00)			PM (15:00-15:40)		
Adults	Children	Total	Adults	Children	Total
40	42	82	55	50	105

2.12.3 Overall, the SCP was used 15 times in the AM Peak and 30 times in the PM peak, during the operation of the SCP it held on average 10 vehicles in the AM with a maximum queue of 20 vehicles and an average of 7 vehicles in the PM with a maximum queue of 15 vehicles.

2.12.4 It is likely that the SCP would allow queued vehicles to clear in the majority of instances. However, there is not considered to be a relationship between the proposed development impact and the operation of the SCP. The proposed development is predicted to result in only a single extra vehicle per minute.

2.12.5 Throughout the survey period it has been demonstrated that 85% (AM Peak) and 62% (PM Peak) of people crossing via the SCP were parked in the Birdwell Venue Car Park.

2.12.6 A summary of the utilisation of the Zebra Crossing is shown in Table 2.6 with the full results available in Appendix B.

Table 2.6 Summary of Zebra Crossing Results

AM (08:30-09:00)			PM (14:30-17:00)		
Adults	Children	Total	Adults	Children	Total
41	11	52	43	26	29

2.12.7 Overall, the Zebra Crossing was used 23 times in the AM Peak and 58 times in the PM Peak, holding on average 2 vehicles in the AM Peak and 3 vehicles in the PM Peak whilst pedestrians crossed.

2.12.8 School attendance data was obtained from the Headteacher who confirmed that on the 15th/16th attendance was 93.9% and 97.8% respectively which is confirmed by the school as the broadly the same as pre-COVID-19 attendance levels.

2.12.9 At the time of the surveys, the arrival and departures of pupils into the school is staggered. In the morning arrivals are 08:45 and 08:55 which then transfer to 3:20 and 3:30 exits in the afternoon.

2.12.10 The staggered arrival/departures could impact traffic levels and congestion when the staggering ends. It is also noted that due to higher levels of parents/guardians working from home this may contribute to travelling to school via other means than the car.

2.12.11 However, the mitigation package proposed is not dependant on the surveys as set out in the original TA.

2.13 Existing Accident Data

2.13.1 Personal injury accident data has been obtained from www.crashmap.co.uk for the five year period between 1st January 2015 and 31st December 2019, and BMBC for the five year period between July 2016 and July 2020 for the highway network in the vicinity of the Site. The study area includes Hay Green Lane and the A61 Sheffield Road.

2.13.2 For the 5-year period covered, there has been 4 accidents within the study area, 2 of which were serious in nature and the remaining were slight accidents. Image 2.21 shows the location of 3 of the 4 accidents (taken from crashmap) and Image 2.22 shows the location of the 4th accident (taken from BMBC), it should be noted that accident reference 1831 1489 is duplicated on both plans.

2.13.3 The full accident data is available in Appendix C.

Image 2.21 Accident Data (Crashmap)

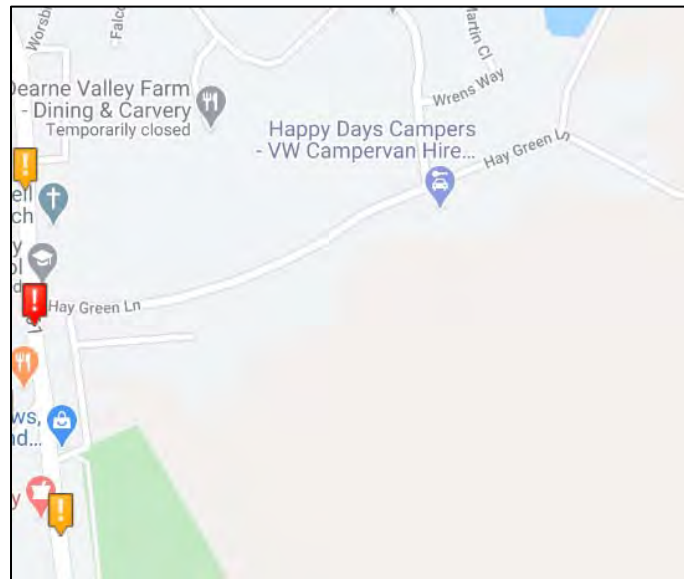
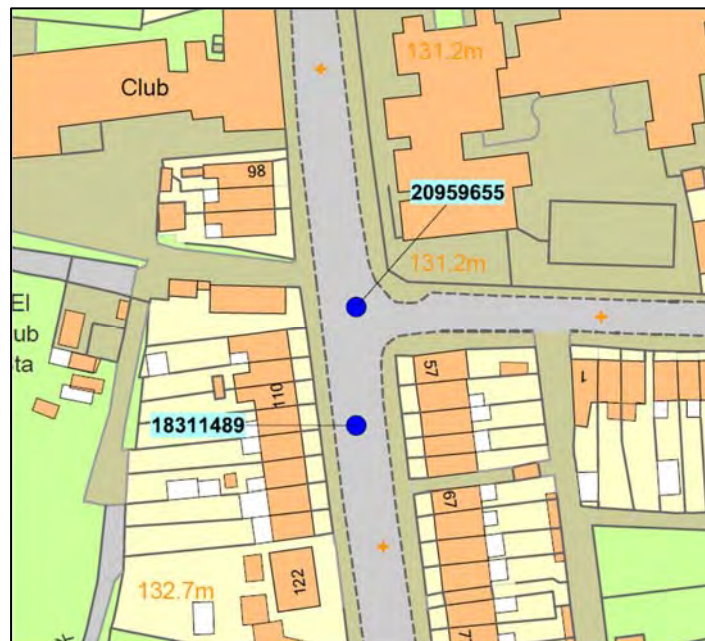


Image 2.22 Accident Data (BMBC)



2.13.4 No accidents have occurred at the proposed Site Access junction on Hay Green Lane.

2.13.5 The serious accident that occurred on the junction with Hay Green Lane and Sheffield Road involved a motorcycle and vehicle at 21:58 under dark conditions. The incident occurred when a vehicle turned right onto Hay Green Lane into the path of the motorcycle

2.13.6 The second serious accident that occurred on the A61 Sheffield Road approximately 20m south of the junction with Hay Green Lane also involved a motorcycle that was driving erratically and carelessly (riding on the back wheel only) not in accordance with the highway code in terms of driving practise.

2.13.7 No accidents involved pedestrians. As previously highlighted, no accidents occurred during school drop-off/pick-up hours.

2.13.8 Whilst any accident is regrettable, having reviewed the accidents in detail there is no evidence to suggest that any of the accidents can be attributed to problems with the highway layout or the school drop off.

3. Proposed Development and Access Strategy

3.1 Development Proposals

3.1.1 The Site layout is shown in illustrative form in Appendix A and show a total of 118 units.

3.2 Vehicular Site Access

3.2.1 Vehicular access is proposed from Hay Green Lane via a simple priority junction. The access is provided to a width of 5.5m with two 2m footways to be provided on either side of the access road.

3.2.2 Visibility Splays of 2.4m x 43.0m are provided as shown on Drawing No. 18039.IN.13 attached in Appendix D.

3.3 Pedestrian / Cycle Site Access

3.3.1 Pedestrian and cycle access to the Site is via a separate link on Hay Green Lane, west of the vehicular access point. There is also a pedestrian/cyclist access on the eastern boundary of the Site which connects to the current public right of way.

3.3.2 In addition, there is a pedestrian access on the south west boundary of the Site that provides access to the community park and A61 Sheffield Road.

3.3.3 All movements from future residents of the Site are facilitated by the existing provisions available and the aforementioned accesses within the proposed development.

3.3.4 It is envisaged that anyone travelling on foot or via cycle will use the ped/cycle access to the west of the development onto A61 Sheffield Road. For residents on the parcel of land adjacent to Hay Green Lane who may use the vehicular access for pedestrian and cycle movements the footway on the northern flank is sufficient, this would involve crossing of Hay Green Lane for which a dropped crossing could be provided.

3.3.5 Hay Green Lane is not of sufficient width to create a formalised footway on the southern flank for this allocated development. Therefore, consideration was given to the movement of people through the Site.

3.3.6 At the pedestrian and cycle access a dropped kerb will be provided to allow cyclists to enter the carriageway on Hay Green Lane. Appropriate signage and tactile paving will be provided at this location to ensure that all users are aware of the other users that may be using the facilities in this location.

3.4 Parking Provision

3.4.1 The BMBC Parking Standards set out in the Supplementary Planning Document (2012) are as follows for residential dwellings:

- 1 or 2 bed dwellings – 1 space per dwelling;
- 3 or more bed dwellings – 2 spaces per dwellings;

3.4.2 All adoptable roads within the layout are provided to a width of 5.5m and can therefore accommodate visitor parking.

3.4.3 Parking will provided to accords with the adopted standards although exact numbers would be confirmed at reserved matters stage.

3.5 Servicing Provision

3.5.1 The internal layout has been designed so that the Barnsley design refuse vehicle can access all properties.

3.5.2 Swept path drawings are provided on the drawings included at Appendix E.

4. Mitigation Measures for Local Road Network

4.1 Introduction

4.1.1 Following a public consultation event and several site visits existing issues were identified on the local highway network surrounding the Site relating to Birdwell Primary School during drop-off/pick-up times.

4.1.2 The public consultation was held on 12th February 2020 and the Planning Case officer was advised of the date to ensure that officers could attend if they wanted to.

4.1.3 It is understood at present, the school has an agreement with the Birdwell Venue (BV) across the road to utilise the parking spaces for school drop off/pick up, however it is scarcely used.

4.1.4 The main issues identified by members of the public at the consultation event:

- The A61 Sheffield Road is a perceived barrier discouraging people from parking at BV and walking across to the school, despite crossing patrol;
- The walking distance between the Birdwell Venue to the School;
- Some children are dropped off on Hay Green Lane and therefore not escorted on foot to the gate;
- Some people park irresponsibly in the car park meaning that it is difficult for Birdwell Venue to receive deliveries

4.1.5 It has been noted that traffic issues are caused by inconsiderate driving (not exclusive to school drop off / pick up times) and the existing geometry of Hay Green Lane, most notably that it is not wide enough to accommodate parking on both sides of the road and maintain two-way traffic.

4.1.6 The proposed development itself is not going to materially increase the existing issue as the issues are not congestion related.

4.1.7 At a pre-application meeting it was envisaged that a study would be undertaken to evidence the scale of the parking issue at school drop-off/pick-up times, however due to the COVID-19 related disruption and schools being closed this was not possible prior to the submission of the original TA. However, schools reopened in September 2020 and parking surveys were undertaken at school drop off/pick up times.

4.2 Mitigation Measures - Infrastructure

4.2.1 Bollards are proposed to be installed on Hay Green Lane on both sides of the carriageway at the junction with A61 Sheffield Road to prevent parking on the pavement as shown on the drawing attached at Appendix F.

4.2.2 On A61 Sheffield Road South of the junction with Hay Green Lane it is proposed that double yellow lines are provided around the junction radii in visibility splays. People should not park within 10m of junction radii but this is occurring and a traffic regulation order would prevent this. It is proposed that a contribution towards a traffic regulation order will be made by the developer.

4.2.3 The developer has also agreed to renew the existing Zebra crossing at Chapel Street.

4.3 Summary

4.3.1 Whilst Harworth are committed to doing everything they can to improve and mitigate the perceived parking/traffic flow issues on the highway network surrounding the Site, there is a limitation of what can be done in a condensed area. It should also be recognised that this is an existing issue.

4.3.2 It was envisaged that a study would be undertaken to analyse the scale of the issue. However, due to ongoing effects of COVID-19 in relation to normal school and work procedures additional surveys were undertaken in September to provide further evidence and the school attendance was comparable with the same dates in 2019, highlighting the mitigations measures provide a robust package and that the issues are existing issues that will not be exacerbated by the proposed development.

4.3.3 The mitigation measures outlined above include comprehensive mitigation measures in terms of an infrastructure package.

4.3.4 Measures to encourage sustainable travel by residents of the proposed development are detailed in the Travel Plan.

5. Site Accessibility and Measures to Promote Sustainable Travel

5.1.1 This chapter describes the accessibility of the Site by non-car modes and sets out how the additional improvements and measures proposed will further enhance accessibility and minimise car trips. The measures proposed accord with policy objectives contained within national and local documentation.

5.2 Pedestrian Accessibility

5.2.1 As described in the TA the measures proposed which will positively influence trips by foot include:

- Internal layout designed to restrict vehicular speeds to less than 20mph;
- Footway connection to the north and east;
- Travel Plan Measures.

5.2.2 The residential design guide "Manual for Streets" (MfS) advises that "walkable neighbourhoods are typically characterised by having a range of facilities within ten minutes (up to about 800m) walking distance of residential areas..." (ref para 4.4.1). However, this is not regarded as an upper limit in MfS and reference is also made to walking offering "the greatest potential to replace short car trips, particularly those under 2km". The acceptability of walking trips up to 2km (an approximate 25 minute walk time) is also supported in the IHT document 'Providing for Journeys on Foot'.

5.2.3 Using GIS software typical walk times (up to 25 mins) from the proposed Site centre are shown on Figure 2. This figure demonstrates that:

- Two supermarkets are within a 20 minute walking distance;
- There are five bus stops within a 200m walking distance of the western boundary of the Site;
- Two Primary Schools are within a 20 minute walking distance. With one being opposite the Site; and
- One Secondary School is within a 25 minute walking distance.

5.2.4 It is therefore concluded that the Site is located in a sustainable location with opportunities to walk to schools, GPs, and public transport modes within a short walk distance.

5.3 Cycle Accessibility

5.3.1 The measures proposed which will positively influence cycle trips are detailed in the pedestrian section above. An acceptable and comfortable distance for general cycling trips is considered to be up to 5 kilometres as referred to in Local Transport Note 2/08 (published by the DfT). However, the same guidance also refers to commuting cycle trips of up to 8km. Using GIS Network Analyst software typical cycle times from the Site are shown on Figure 3. This figure shows that:

- The Site is within a 25 minute cycle ride from 5 train stations;
- Barnsley Centre is within a 25 minute cycle ride of the Site;
- The Site is located within a 15 minute cycle distance of the national cycle network, providing access across Barnsley and surrounding areas;
- The Hoyland North Masterplan area is within a 5 minute cycle.

5.3.2 It is therefore concluded that the proposed Site, will provide cycle accessibility to a range of local services within a 25 minute cycle, whilst large employment areas are accessible in a 30 minute cycle ride.

5.4 Accessibility by Bus

5.4.1 As detailed in Chapter 2 bus stops are located in close proximity to the Site on A61 Sheffield Road. Figure 4 shows that the majority of the Site is within 400m of the bus stops, whilst all the Site is within a 600m walk distance.

5.4.2 The bus services from these stops provide access to the following destinations:

- Barnsley – 17 minute journey, every 15 minutes;
- Sheffield – 47 minute journey, every 25 minutes;
- Elsecar – 24 minute journey, every 15 minutes; and
- Wombwell – 32 minute journey, every 60 minutes.

5.4.3 The existing bus services will provide residents of the Site with high frequency access to the city and town centres of Barnsley and Sheffield and the large transport interchanges at both towns for travel further afield.

5.4.4 Furthermore, the travel planning measures will have a positive influence on the sustainable travel choices made by users of the proposed Site.

5.5 Accessibility by Rail

5.5.1 The nearest train station to the Site is situated at Elsecar on the Elland and Penistone Line. It is located an approximate 20 minute cycling distance from the Site as shown in Figure 3.

5.5.2 Trains from Elsecar serve destinations including Barnsley, Leeds, Wakefield, Huddersfield and Sheffield among others.

5.5.3 Elsecar Station benefits from cycle storage, ticket machines, CCTV and step-free access.

6. Development Trip Generations and Distributions

6.1.1 This chapter sets out the trip generations and distribution methodologies associated with the proposed development Site. The trip generations for the residential development used in this assessment are taken from the approved Transport and Access Appraisal Report (TAAR) issued by Fore in May 2019 for the Hoyland North Masterplan.

6.2 Residential Trip Rate

6.2.1 In order to calculate a trip rate for the residential units, trip rates have been taken from the Fore TAAR. Table 6.1 shows the trip rates for the AM and PM Peak.

Table 6.1 Summary of Trip Rates for Hay Green Lane

AM Peak			PM Peak		
Arrival	Departure	Two Way	Arrival	Departure	Two Way
0.134	0.381	0.515	0.330	0.162	0.492

6.3 Residential Trip Generation

6.3.1 A total of 118 dwellings are proposed at the Site, as such the trip rates in Table 5.1 have been applied to the number of units to create the vehicular trip generations from the Site, shown in Table 6.2.

Table 6.2 Proposed Trip Generation

AM Peak (08:30-09:30)			PM Peak (16:30-17:30)		
Arrival	Departure	Two Way	Arrival	Departure	Two Way
16	45	62	40	19	59

6.4 Residential Trip Generations

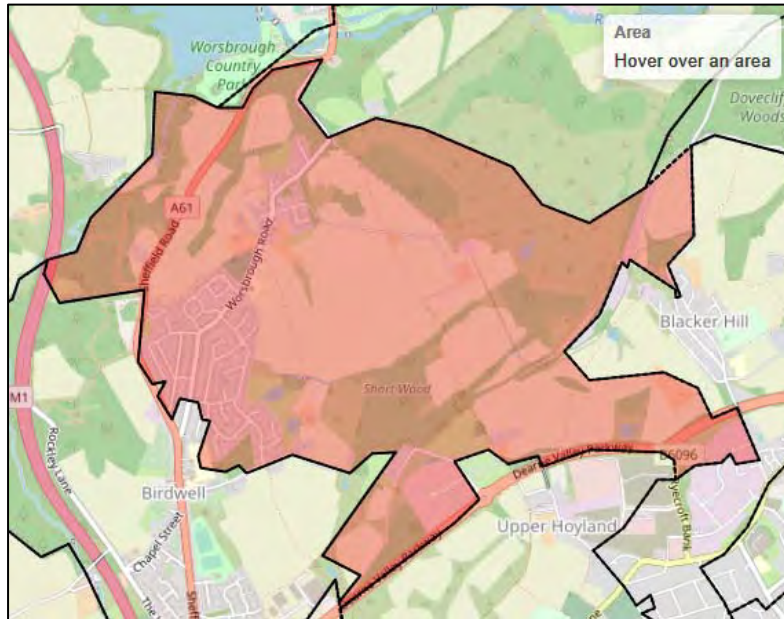
6.4.1 To establish the trip generations by mode for the development the 2011 Census Data has been interrogated for the residential area of Barnsley 028C Middle Layer Super Output Area as shown in Image 6.1, which is considered to reflect the proposed Site characteristics.

6.4.2 The census mode splits for the selected area is shown in Table 6.3.

Table 6.3 Residential Mode Split – Barnsley 028C

Method of Travel to Work	Total Residents	Percentage
Work mainly at or from home	144	3.7%
Train/Tram	34	0.9%
Bus, minibus or coach	265	6.8%
Taxi	11	0.3%
Motorcycle, scooter or moped	27	0.7%
Driving a car or van	2,927	74.7%
Passenger in a car or van	268	6.8%
Bicycle	25	0.6%
On foot	215	5.5%
Total	3,916	100.0%

Image 6.1 Barnsley 028C Lower Layer Super Output Area



6.4.3 Having established the total vehicular trip generations and the census modal split for existing residents, it is possible to calculate the number of trips by mode. To calculate trips by mode the total vehicular arrivals and departures have been multiplied by a factor of the Car/Van Driver and the modal splits shown in Table 6.3.

6.4.4 The resulting predicted number of development trips by different modes is shown in Tables 6.4 and 6.5 for the AM and PM peaks respectively

Table 6.4 AM Peak Multi Modal Trip Generations

Trip Type	Arrivals	Departures	Total
Work mainly at or from home	1	2	3
Train/Tram	0	1	1
Bus, minibus or coach	1	4	6
Taxi	0	0	0
Motorcycle, scooter or moped	0	0	1
Driving a car or van	16	45	62
Passenger in a car or van	1	4	6
Bicycle	0	0	1
On foot	1	3	5
Total	21	60	83

Table 6.5 PM Peak Multi Modal Trip Generations

Trip Type	Arrivals	Departures	Total
Work mainly at or from home	2	1	3
Train/Tram	0	0	1
Bus, minibus or coach	4	2	5
Taxi	0	0	0
Motorcycle, scooter or moped	0	0	1
Driving a car or van	40	19	59
Passenger in a car or van	4	2	5
Bicycle	0	0	1
On foot	3	1	4
Total	54	25	79

6.5 Residential Trip Distribution and Assignment

6.5.1 Having established the development traffic distribution using the 2011 Census Data, an assignment exercise has been completed to predict the assignment of these trips onto the highway network.

6.5.2 The existing distribution at Hay Green Lane / A61 Sheffield Road priority junction has been used as a basis to establish the distribution to/from the Site. The further distribution has been applied based on the shortest journey time.

6.5.3 The proposed residential distribution for the Site is shown in Figure 12. Applying the trip generations in Table 5.2 to the distributions produces the following figures:

- Figure 13 - AM Peak Hour Development Trip Generation; and
- Figure 14 - PM Peak Hour Development Trip Generation.

6.5.4 The analysis of the 2011 Census Data is available in Appendix G.

7. Traffic Flows and Development Impact

7.1 Introduction

7.1.1 This chapter sets out the traffic flow assumptions and provides an assessment of the development traffic impact on the existing highway network. It also includes an assessment of the impact of the development traffic on the local highway network.

7.2 Peak Hour Traffic Flows

7.2.1 The November 2019 traffic surveys identified the following existing weekday peak hour periods:

- Weekday AM Peak – 08:30 to 09:30; and
- Weekday PM Peak – 16:30 to 17:30.

7.2.2 The traffic count flows for these periods are shown on Figures 10 and 11 for the morning and evening peak hours respectively and are used to provide an insight into the existing operation of the network.

7.3 Committed Development Flows

7.3.1 One plot on Rockingham Phase 1 (Ref: 2014/1452) has not yet been built out and this has approval for a 2,700sqm hotel. The trips associated with this plot have been taken from the associated Transport Assessment. An extract of the flows is attached in Appendix H. The consented trips have been distributed across the network for the AM and PM Peak period and are shown in Figures 15 and 16 respectively.

7.3.2 These have been added to the Development Trip Generations to create 2019 Base Flows as shown in the following figures:

- Figure 23 2019 AM Base; and
- Figure 24 2019 PM Base.

7.4 Future Assessment Year and Traffic Growth

7.4.1 Traffic growth is based on a combination of proposed future developments, car ownership and changing attitudes in the way people use and have access to their vehicles.

7.4.2 A development design year of 2025 is proposed in order to provide sufficient time for construction and occupation of the Site.

7.4.3 In addition to this, as the Site is allocated within the current adopted Local Plan to 2033, BMBC have requested that a design year of 2033 is assessed. This is included below.

7.4.4 Traffic growth rates, between 2019 and 2025 and 2019 and 2033 have been obtained from TEMPro v7.2 using Barnsley 028 area and these values are as follows:

- 2019-2025 AM growth rate of 1.0577
- 2019-2025 PM growth rate of 1.0578
- 2019-2033 AM growth rate of 1.1255
- 2019-2033 PM growth rate of 1.1265

7.4.5 The 2025 and 2033 Base traffic flows have been calculated by applying the TEMPro growth factors to the 2019 count. These Base flows are shown in the following Figures:

- 2025 AM Base Flow – Figure 25
- 2025 PM Base Flow – Figure 26
- 2033 AM Base Flow – Figure 31
- 2033 PM Base Flow – Figure 32

7.5 Design Traffic Flows

7.5.1 Adding the proposed development trips described in Chapter 5 and shown in Figures 13 and 14 to the 2025 and 2033 base traffic flows produces the design traffic flows as shown in the following figures:

- 2025 AM Design Flow – Figure 27
- 2025 PM Design Flow – Figure 28
- 2033 AM Design Flow – Figure 33
- 2033 PM Design Flow – Figure 34

7.6 Allocated Development Flows

7.6.1 As requested in the pre-application the Hoyland North Masterplan Site has been considered in this assessment, the anticipated traffic flows associated with Hoyland North (which includes the Rockingham Development and the Wider Masterplan) for the AM and PM are shown in Figures 20 and 21 respectively.

7.6.2 These have then been combined with the 2025 and 2033 Design Flows to create Design Sensitivity Flows which are shown in the following figures:

- 2025 AM Design Sensitivity Flow – Figure 28
- 2025 PM Design Sensitivity Flow – Figure 29
- 2033 AM Design Sensitivity Flow – Figure 35
- 2033 PM Design Sensitivity Flow – Figure 36

7.7 Capacity Assessment of Hay Green Lane / A61 Sheffield Road Priority Junction

7.7.1 Hay Green Lane / A61 Sheffield Road Priority Junction has been modelled to assess the impact of the development using the PICADY function in the Junctions 9 software for the following scenarios:

- 2019 AM and PM Count;
- 2019 AM and PM Base;
- 2025 AM and PM Base;
- 2025 AM and PM Design;
- 2025 AM and PM Design Sensitivity;
- 2033 AM and PM Base;
- 2033 AM and PM Design; and
- 2033 AM and PM Design Sensitivity.

7.7.2 The junction has been modelled as a priority junction and the outputs are in Appendix I and the results summarised in Table 7.1.

7.7.3 The modelling summarised in Table 7.1 has been undertaken using hourly input data. Using this method is an industry standard method when junction capacity is not considered a concern and the traffic flows across the hour are generally flat in profile i.e. approximately the same volume of traffic is present in each 15 minute of the peak hour periods.

Table 7.1 Summary of Modelling Outputs

Scenario	Lane	AM Peak Hour		PM Peak Hour	
		RFC	Ave Q	RFC	Ave Q
2019 Count Validation	A61 Sheffield Road	0.13	0.2	0.08	0.1
	Hay Green Lane	0.10	0.1	0.08	0.1
2019 Base Assessment	A61 Sheffield Road	0.13	0.2	0.08	0.1
	Hay Green Lane	0.10	0.1	0.08	0.1
2025 Base Assessment	A61 Sheffield Road	0.15	0.2	0.09	0.1
	Hay Green Lane	0.11	0.1	0.09	0.1
2025 Design Assessment	A61 Sheffield Road	0.34	0.5	0.17	0.2
	Hay Green Lane	0.11	0.1	0.16	0.2
2025 Design Sensitivity Assessment	A61 Sheffield Road	0.36	0.6	0.18	0.2
	Hay Green Lane	0.11	0.1	0.16	0.2
2033 Base Assessment	A61 Sheffield Road	0.18	0.2	0.10	0.1
	Hay Green Lane	0.12	0.1	0.09	0.1
2033 Design Assessment	A61 Sheffield Road	0.40	0.6	0.19	0.2
	Hay Green Lane	0.15	0.2	0.17	0.2
2033 Design Sensitivity	A61 Sheffield Road	0.41	0.7	0.20	0.2
	Hay Green Lane	0.13	0.1	0.17	0.2

7.7.4 A Ratio of Flow to Capacity value below 0.85 indicates that a junction or arm operates within its predicted capacity. An RFC value between 0.85 and 1.00 indicates that there may be occasions during the period modelled when queues will develop and delays will occur. An RFC value greater than 1.00 indicates that the junction or arm operates beyond its theoretical capacity.

7.7.5 Table 7.1 demonstrates that the junction operates comfortably within capacity in all scenarios. The worst case RFC in a 2033 design scenario is just 0.41 which is significantly below the desired threshold of 0.85, the worst case operation of the junction occurs in the AM peak which is expected given that the peak hour for background traffic overlaps with the school drop off.

7.7.6 Another measure of junction efficiency is the average delay experienced by a driver travelling through the junction on difference arms and this measure is more easily understandable for a reader that does not have a highways engineering background. Therefore, in this updated Transport Assessment Table 7.2 has been created to summarise delay information.

7.7.7 The following assessment of delay is brief and focuses on the worst case AM peak.

Table 7.2 Summary of Delay Information for the Hourly Flow Modelling

		Stream B-AC (Hay Green Lane)		Stream C-AB (Right Turn to Hay Green Lane)	
		Max Delay	Average of Max Delay	Max Delay	Average of Max Delay
AM	2019 Count	13.38	11.33	9.10	8.32
	2033 Base	16.25	12.93	9.84	8.84
	2033 Design	24.27	17.20	10.22	9.10

7.7.8 It can be seen from Table 7.2 that the validated model shows that in the 2019 AM peak the average delay for vehicles turning right into Hay Green Lane is 8.42 seconds in the existing scenario, the average delay for vehicles exiting Hay Green Lane to Sheffield Road is 11.33 seconds in the morning peak.

7.7.9 However, to understand the impact of the development in the 2033 future design year (scenario requested by BMBC) a comparison has been done between the without (base) and with (design) scenarios in the 2033. It can be seen that in AM peak that for traffic turning out of Hay Green Lane the average delay increases from 12.93 seconds to 17.20 seconds i.e. an increase of 4.27 seconds in the worst case peak hour. It is acknowledged that some cars will experience greater delays, but 4.27 second increase is the average increase in time it will take to turn out of Hay Green Lane.

7.7.10 For cars turning right into the junction from Sheffield Road the delay increases from 8.84 to 9.1 seconds which is an increase of just 0.26 seconds.

7.7.11 The increases in time to travel through the junction are not material and are not severe when compared against the National Planning Policy Framework (2019).

7.8 Sensitivity Test Requested by BMBC

7.8.1 Following the issue of the initial revision of this document it has been requested by BMBC that a sensitivity test is undertaken. This assessment is very robust and applies the turning proportions from the 2019 survey by 15 minute period to the development flows, the background traffic that is growthed and also committed development. Whilst exceptionally robust it is agreed appropriate to provide a level of comfort that highway capacity is not an issue in relation to this planning application.

7.8.2 Table 7.3 summarises the modelling outputs when the model is run at 15 minute intervals, the breakdown of which is available in Appendix I, along with the full output summary and the spreadsheet that shows how the traffic flows have been derived. This has been completed to identify any potential peaks within the peak hour. As can be seen the junction operates comfortably below the desired capacity threshold of 0.85 RFC value in all scenarios.

Table 7.3 Summary of Sensitivity Modelling Outputs

Scenario	Lane	PM Peak Hour			
		RFC	Ave Q	RFC	Ave Q
2019 Count Validation	A61 Sheffield Road	0.22	0.3	0.10	0.1
	Hay Green Lane	0.11	0.1	0.08	0.1
2019 Base Assessment	A61 Sheffield Road	0.23	0.3	0.10	0.1
	Hay Green Lane	0.12	0.1	0.08	0.1
2025 Base Assessment	A61 Sheffield Road	0.25	0.3	0.10	0.1
	Hay Green Lane	0.13	0.1	0.09	0.1
2025 Design Assessment	A61 Sheffield Road	0.62	1.5	0.20	0.2
	Hay Green Lane	0.16	0.2	0.15	0.2
2025 Design Sensitivity Assessment	A61 Sheffield Road	0.64	1.6	0.20	0.3
	Hay Green Lane	0.16	0.2	0.15	0.2
2033 Base Assessment	A61 Sheffield Road	0.30	0.4	0.12	0.1
	Hay Green Lane	0.14	0.2	0.09	0.1
2033 Design Assessment	A61 Sheffield Road	0.68	1.8	0.21	0.3
	Hay Green Lane	0.16	0.2	0.16	0.2
2033 Design Sensitivity	A61 Sheffield Road	0.70	2.0	0.22	0.3
	Hay Green Lane	0.17	0.2	0.17	0.2

7.8.3 The summary of the delays associated with the sensitivity modelling is shown in Table 7.4.

Table 7.4 Summary of Delay Information

		Stream B-AC (Hay Green Lane)		Stream C-AB (Right Turn to Hay Green Lane)	
		Max Delay	Average of Max Delay	Max Delay	Average of Max Delay
AM	2019 Count	13.97	11.22	8.89	8.67
	2033 Base	18.30	13.23	9.81	9.50
	2033 Design	37.03	19.70	9.95	9.75

7.8.4 It can be seen from Table 7.2 that the validated model shows that in the 2019 AM peak that the average delay for vehicles turning right into Hay Green Lane is 8.92 seconds in the existing scenario, the average delay for vehicles exiting Hay Green Lane to Sheffield Road is 11.22 seconds in the morning peak.

7.8.5 It should be noted that it is not unusual for delay values to go up as well as down between the values in Table 7.2 and 7.4. This is because the flows that oppose turning movements change in proportion through each 15 minute period.

7.8.6 To understand the impact of the development in the 2033 future design year it can be seen that in AM peak that for traffic turning out of Hay Green Lane the average delay increases from 13.23 seconds to 19.7 seconds i.e. an increase of 6.47 seconds in the worst case peak hour. It is acknowledged that some cars will increase greater delays but this is the average.

7.8.7 For cars turning right into the junction from Sheffield Road the delay increases from 9.45 to 9.75 seconds which is an increase of just 0.3 seconds.

7.8.8 The increases in time to travel through the junction are not material and are not severe when compared against the National Planning Policy Framework (2019).

8. Summary and Conclusions

8.1.1 Mosodi have been appointed to produce this Transport Assessment (TA) on behalf of Harworth Group to support a planning application for a residential development on a Site to the south of Hay Green Lane, Birdwell.

8.1.2 The proposed layout is shown on the architects plans in Appendix A and comprises:

- 118 dwellings;
- Vehicular Site access via Hay Green Lane;
- Public Open Space; and
- Design of internal layout to a maximum speed of 20mph.

8.1.3 The Site is located in Birdwell, Barnsley and is currently vacant and is allocated for residential development in the Barnsley Adopted Local Plan under reference HS59.

8.1.4 The report has identified what measures will be taken to deal with any anticipated impacts of the scheme proposals and has defined what improvements and initiatives will be implemented to improve accessibility to the Site by all modes of travel.

8.1.5 Following a consultation and subsequent Site Visit a number of existing issues were identified on the local highway network surrounding the Site, in particular at the Birdwell Primary School during drop-off/pick-up times. This report has been updated to include survey data that could not have been collected at the time of initial issues due to school closures resulting from the COVID-19 pandemic. As such Chapter 4 outlines measures to improve traffic flow

8.1.6 The proposed development is also supported by a robust travel plan.

8.1.7 A detailed assessment of the accessibility of the site has been completed in Chapter 5 concluding the following:

- Two primary schools are within a 20 minute walk;
- A secondary school is within a 30 minute walk;
- Two supermarkets are within a 20 minute walking distance;
- There are 5 bus stops within a 200m walking distance of the Site;
- The Site is within a 25 minute cycle ride from 5 train stations; and
- Barnsley City Centre is within a 25 minute cycle ride of the Site.

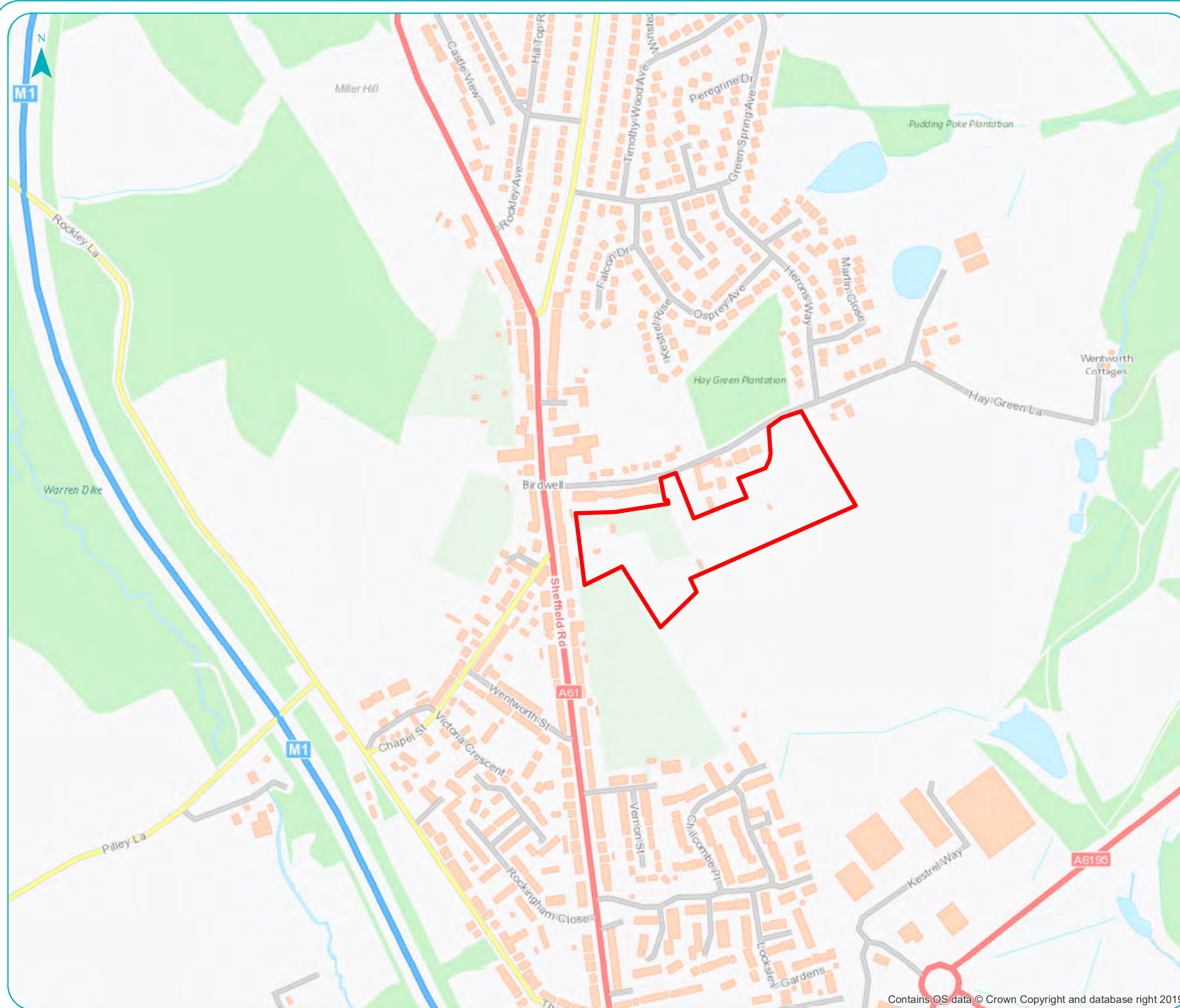
8.1.8 It is therefore concluded that the proposed Site will provide excellent accessibility by foot and cycle to a vast range of services, facilities and employment opportunities.

8.1.9 The distribution and assignment of potential development traffic shows that the increase in traffic is modest. Capacity assessments have demonstrated that the development traffic can be comfortably accommodated by the highway network.

8.1.10 Robust sensitivity tests have been undertaken which provide further comfort that traffic generated by the proposed development can be comfortably accommodated by the proposed development.

8.1.11 From the work undertaken it is concluded that there are no reasons on highways or transport grounds why the development Site should not be granted planning permission for change of use.

Figures



- Key**
- Indicative Site Boundary
 - ★ Site Location



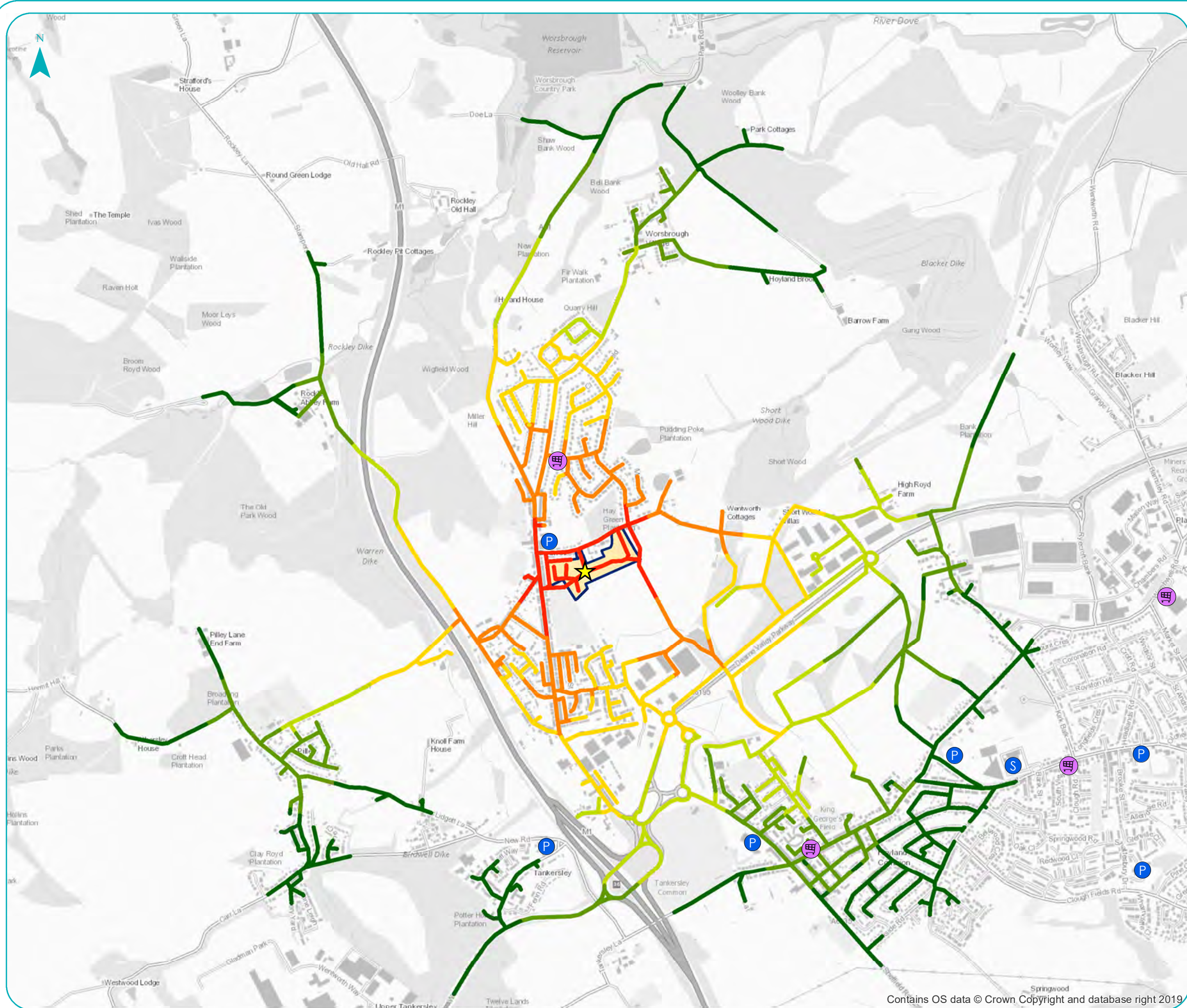
Hay Green Lane, Birdwell Residential Development Site Location Plan

Figure 1

Scale @ A3 1:5,000



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- Key**
- Indicative Site Boundary
 - ★ Site Location
 - ⌘ Supermarkets
 - + GP
- Schools**
- All Through
 - N Nursery
 - P Primary
 - S Secondary
 - C College
- Walk Accessibility (4.8kph)**
- 0-5 Minutes
 - 5-10 Minutes
 - 10-15 Minutes
 - 15-20 Minutes
 - 20-25 Minutes
 - 25-30 Minutes

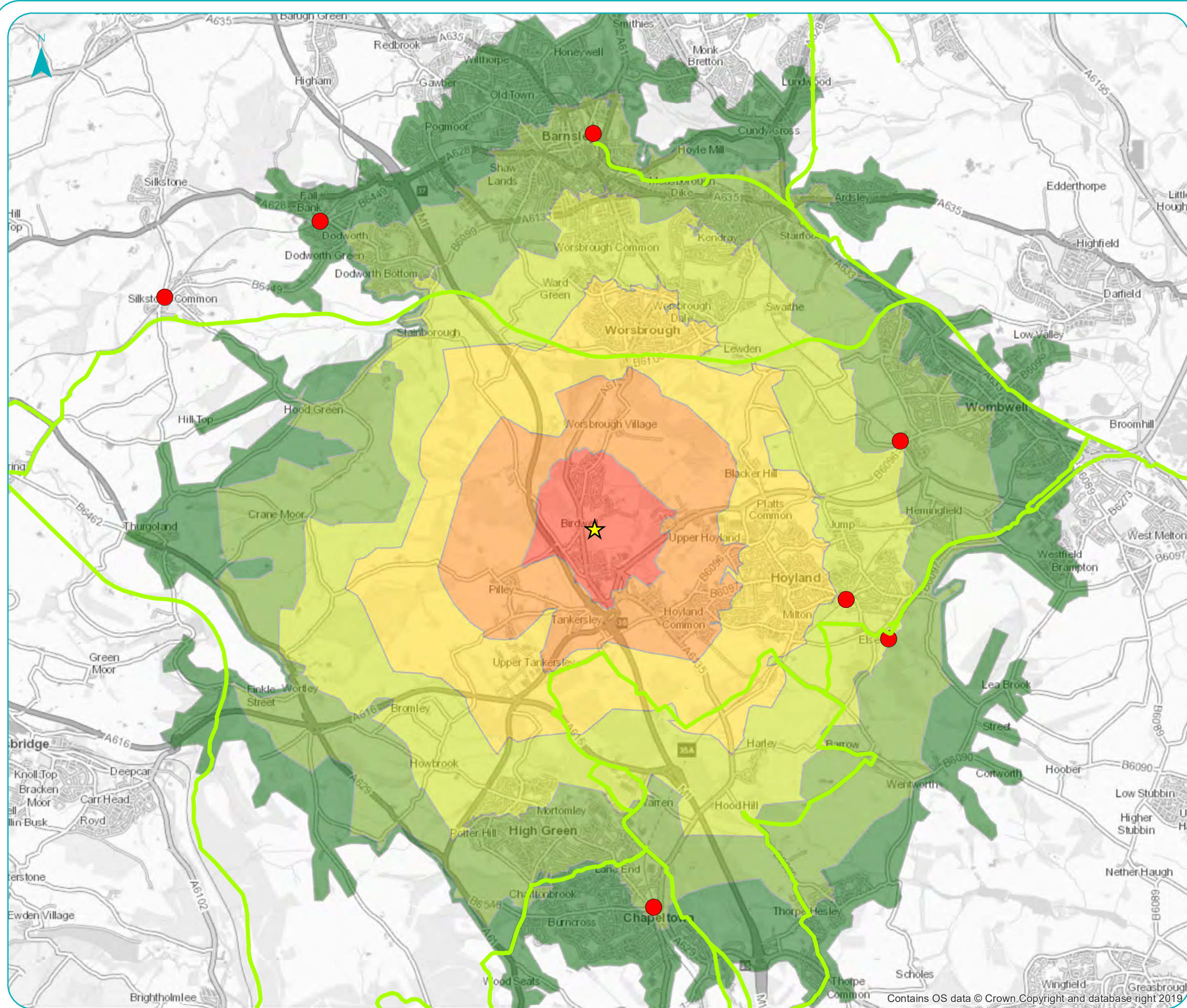
Hay Green Lane, Birdwell Residential Development Pedestrian Accessibility

Figure 2

Scale @ A3 1:15,000



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Key

- ★ Site Location
 - Railway Stations
 - National Cycle Routes
- Cycle Accessibility (16kph)**
- 0-5 Minutes
 - 5-10 Minutes
 - 10-15 Minutes
 - 15-20 Minutes
 - 20-25 Minutes
 - 25-30 Minutes

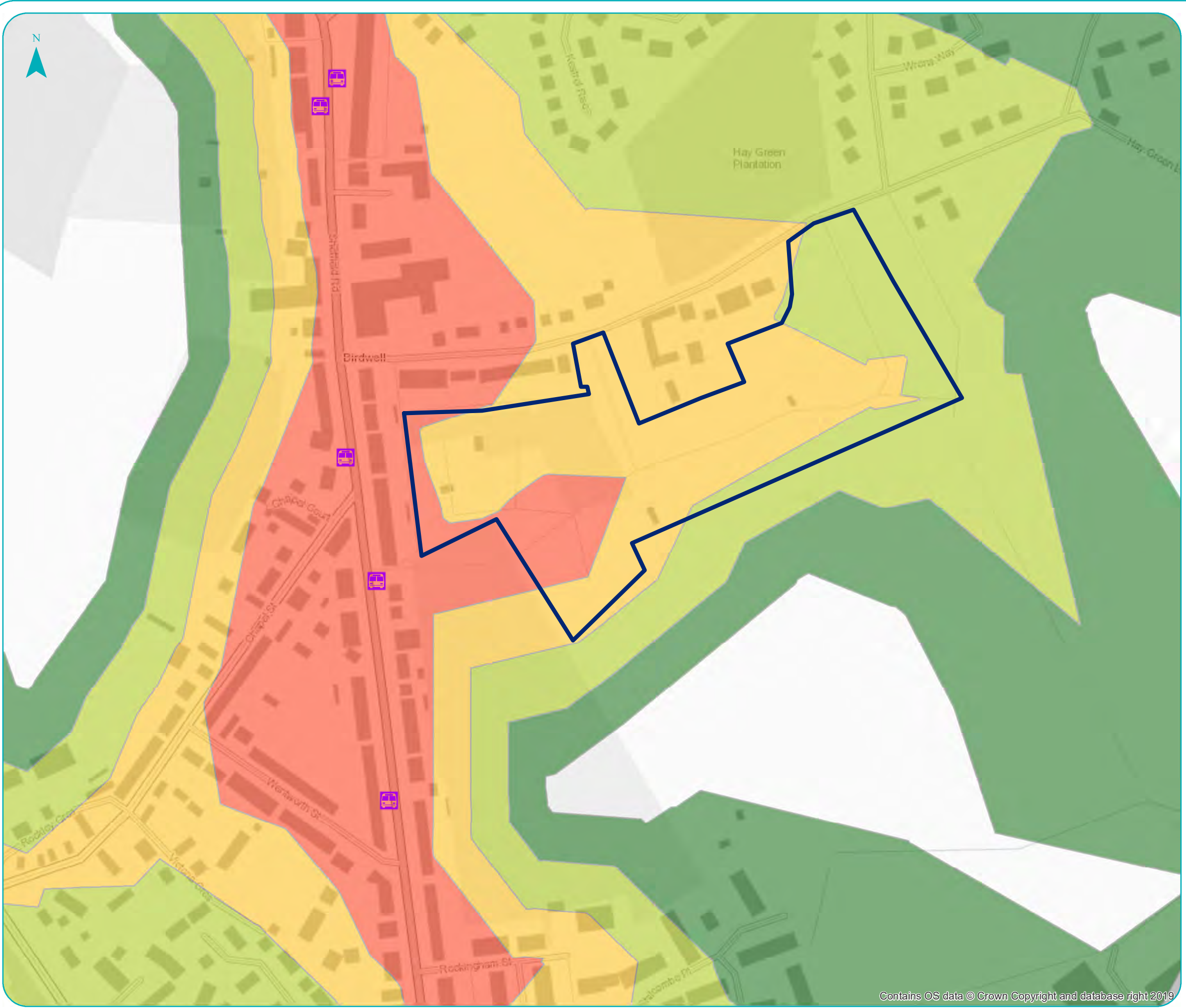
Hay Green Lane, Birdwell Residential Development Cycle Accessibility

Figure 3

Scale @ A3 1:50,000



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Key

- Indicative Site Boundary
- 🚌 Local Bus Stops

Bus Stop Distance

- 0-200m
- 200-400m
- 400-600m
- 600-800m

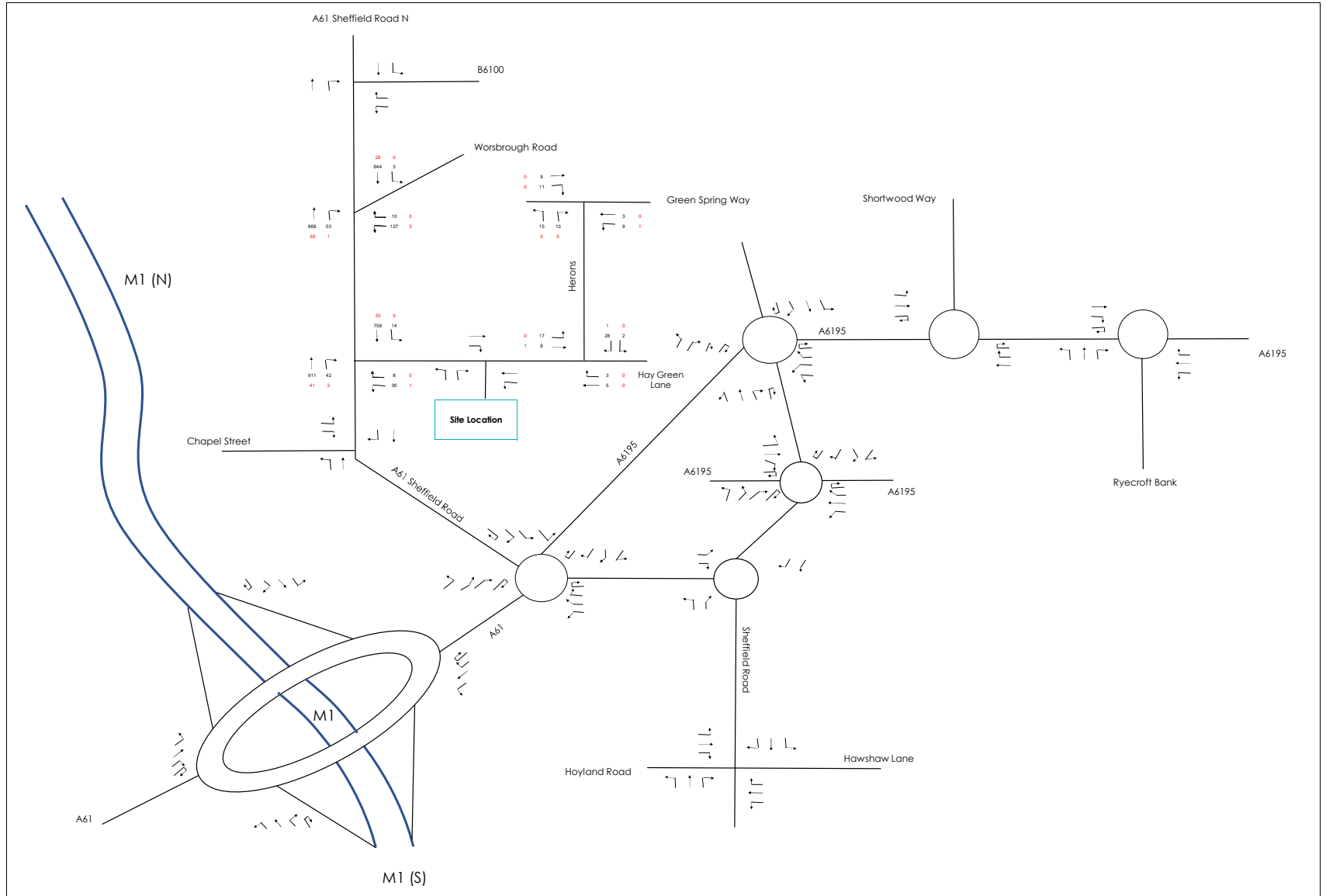
Hay Green Lane, Birdwell Residential Development Bus Stop Accessibility

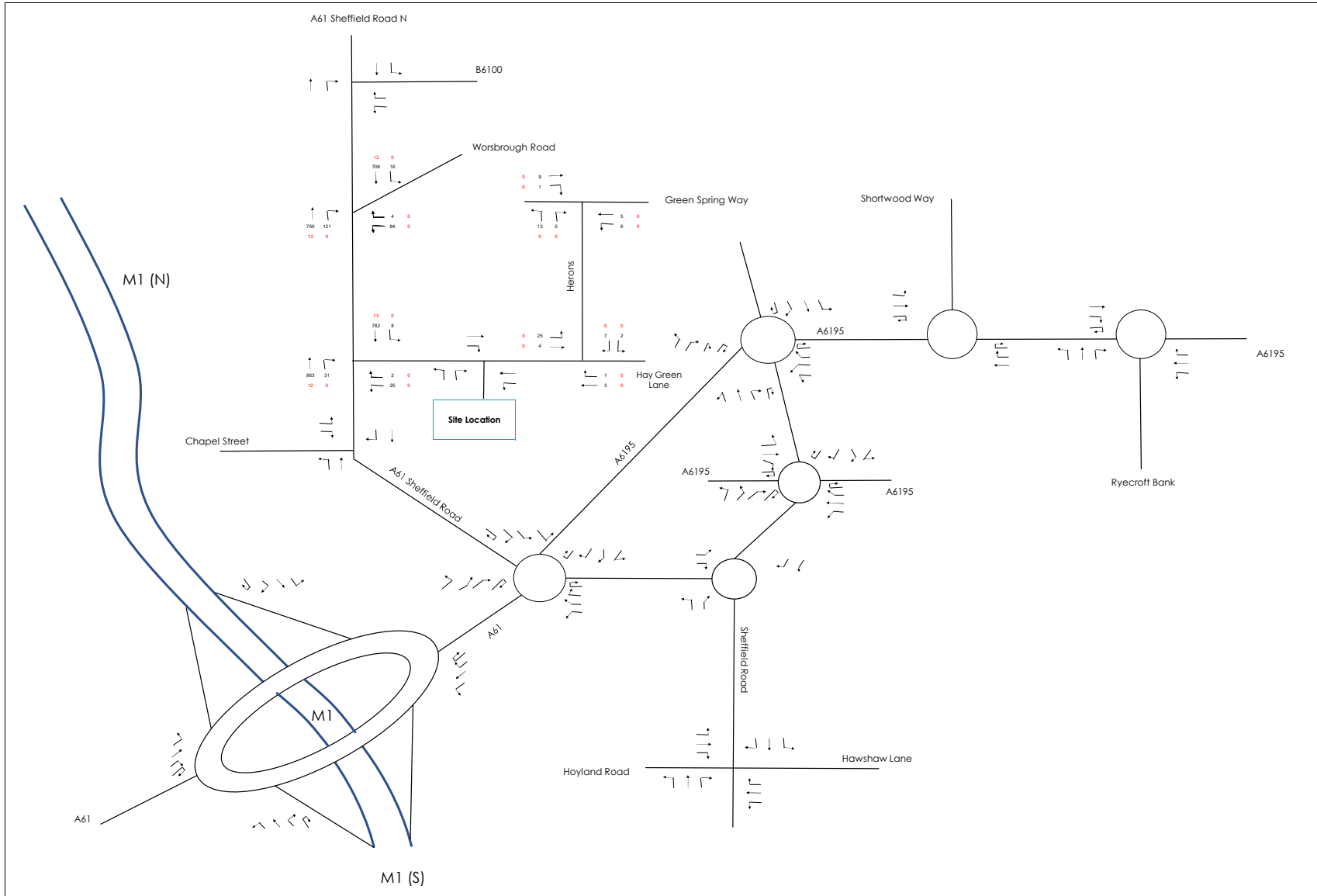
Figure 4

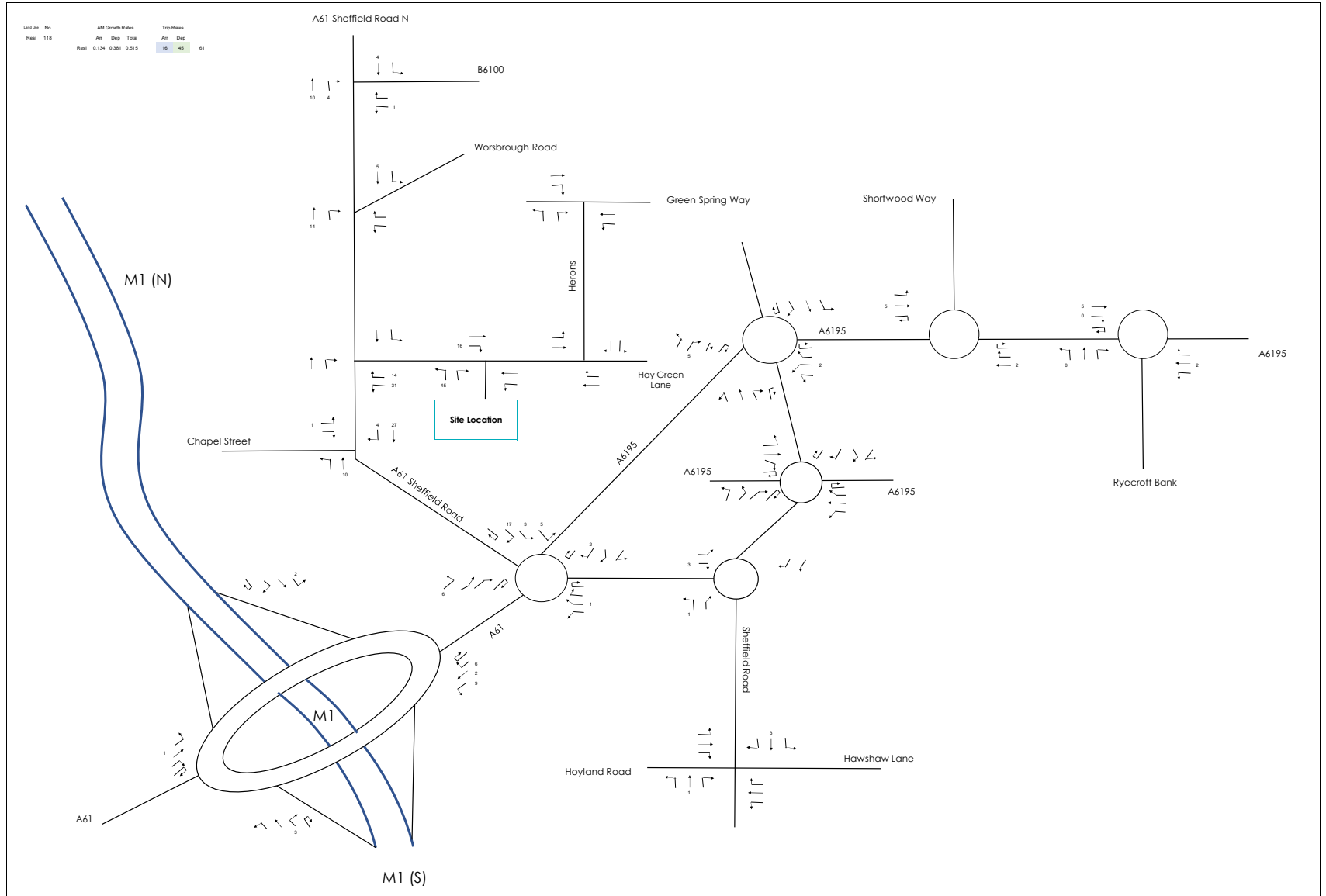
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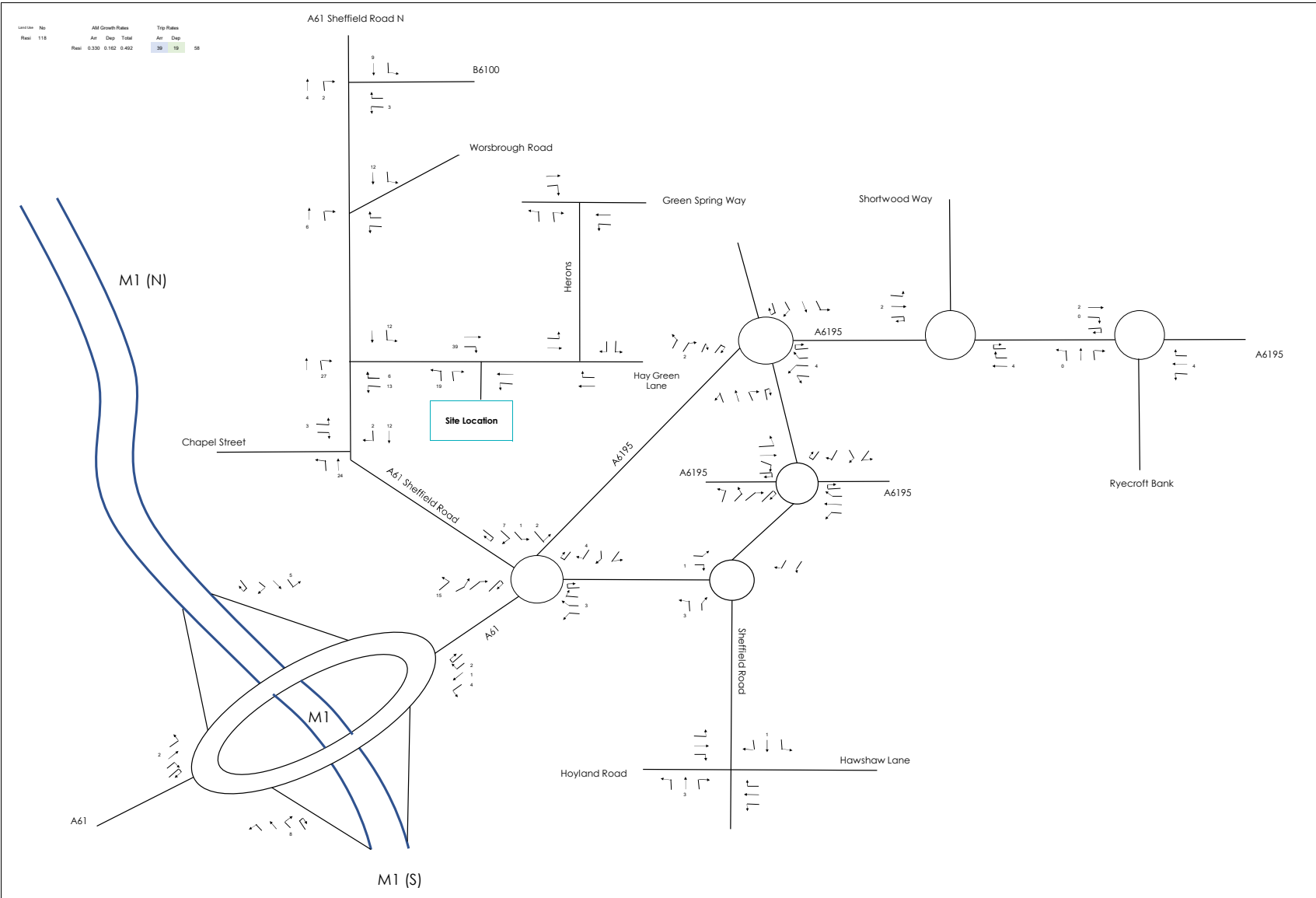
Level	No	AM Growth Rates			Trip Rates		
		Arr	Dep	Total	Arr	Dep	Total
Resid	118	0.154	0.381	0.515	16	45	61

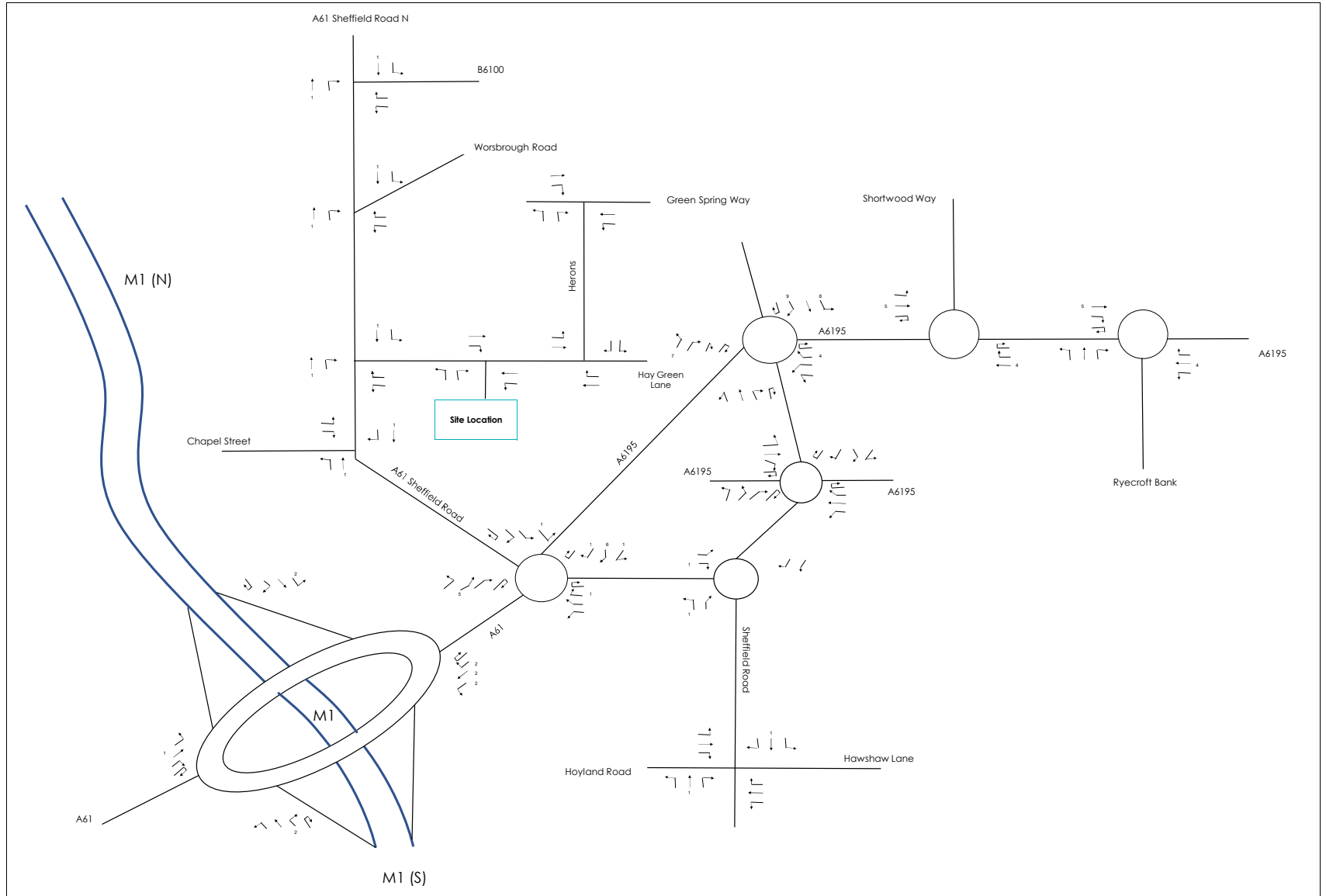


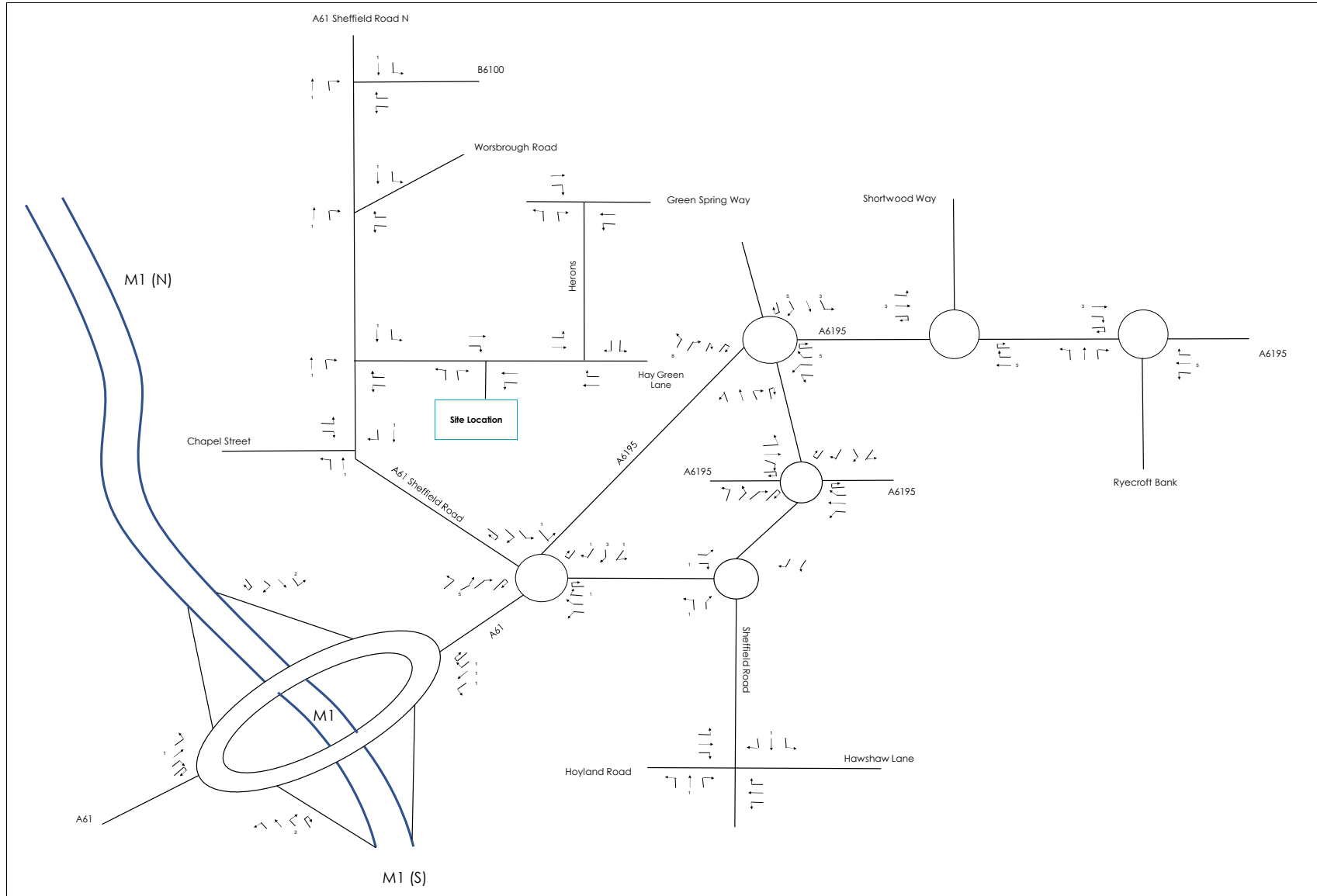
Project: Hay Green Lane, Barnsley

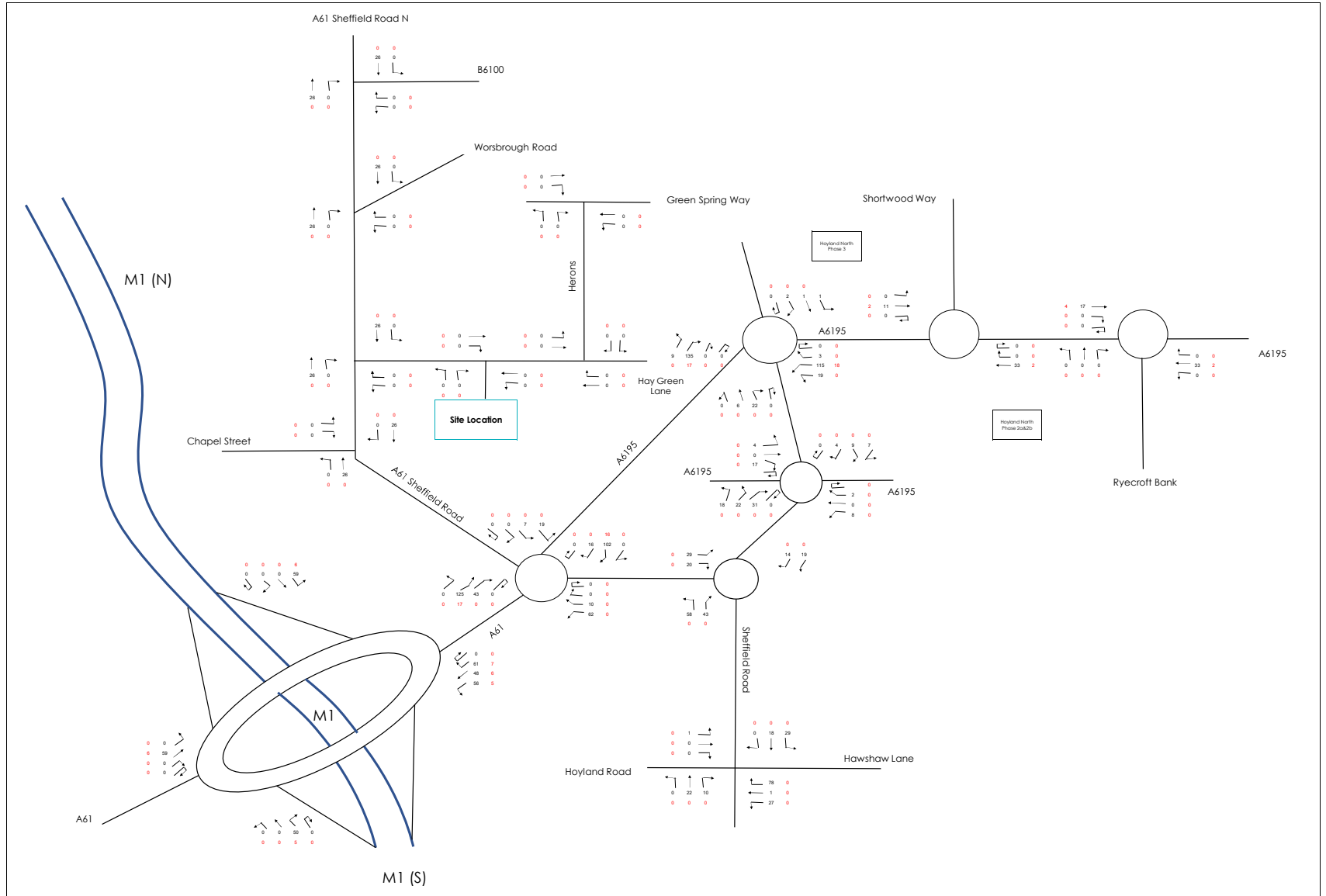
Trip Generation AM Peak (08:30-09:30)

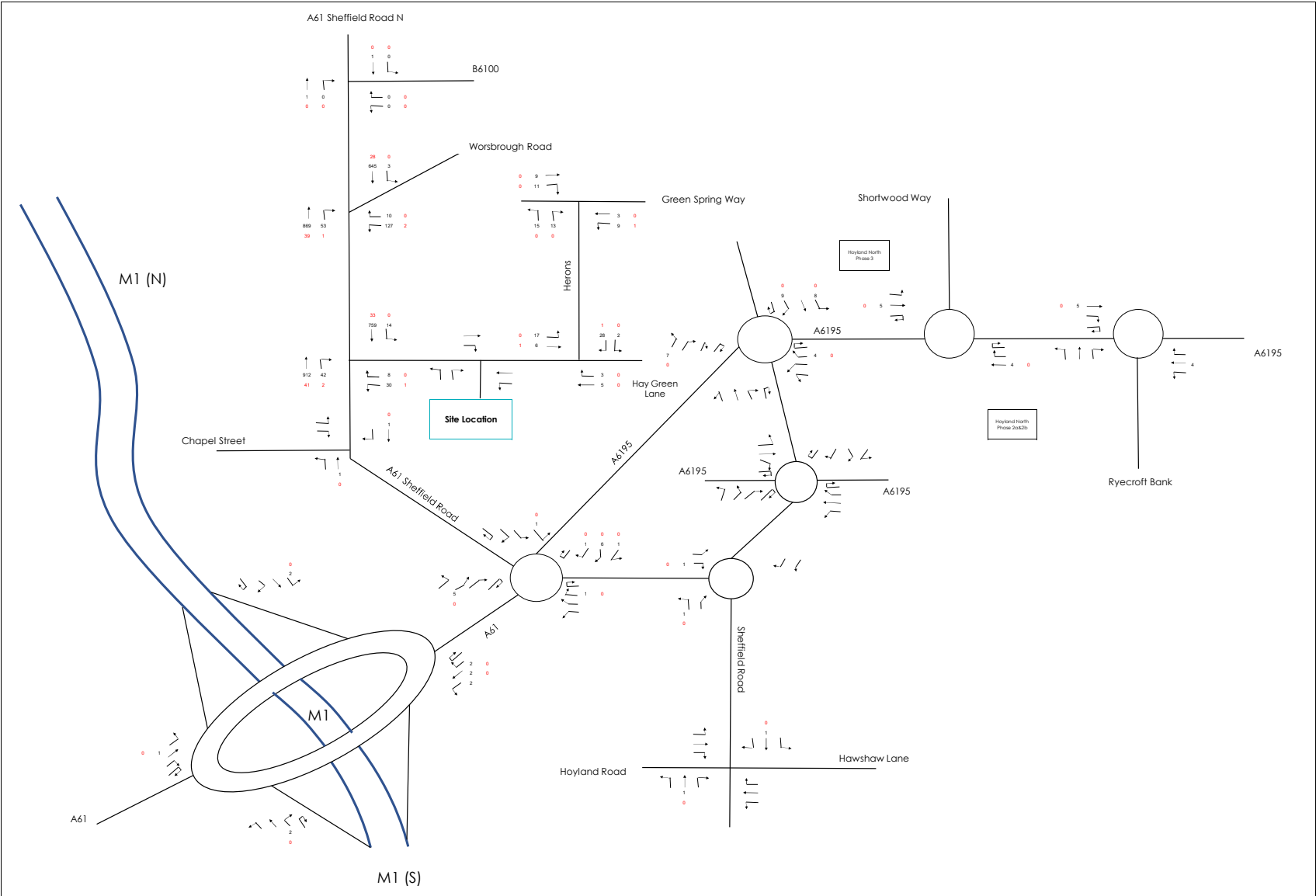
Figure 13



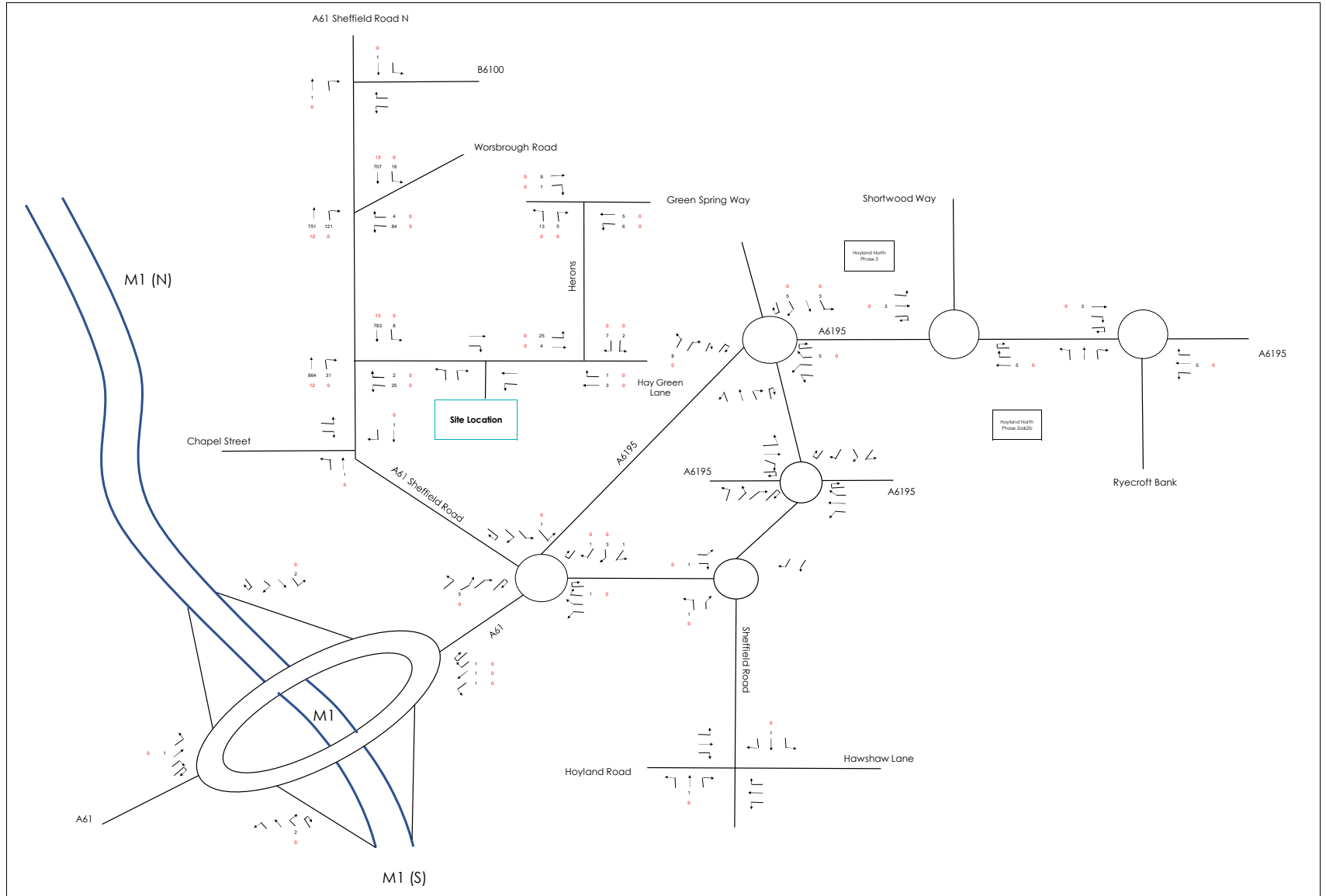




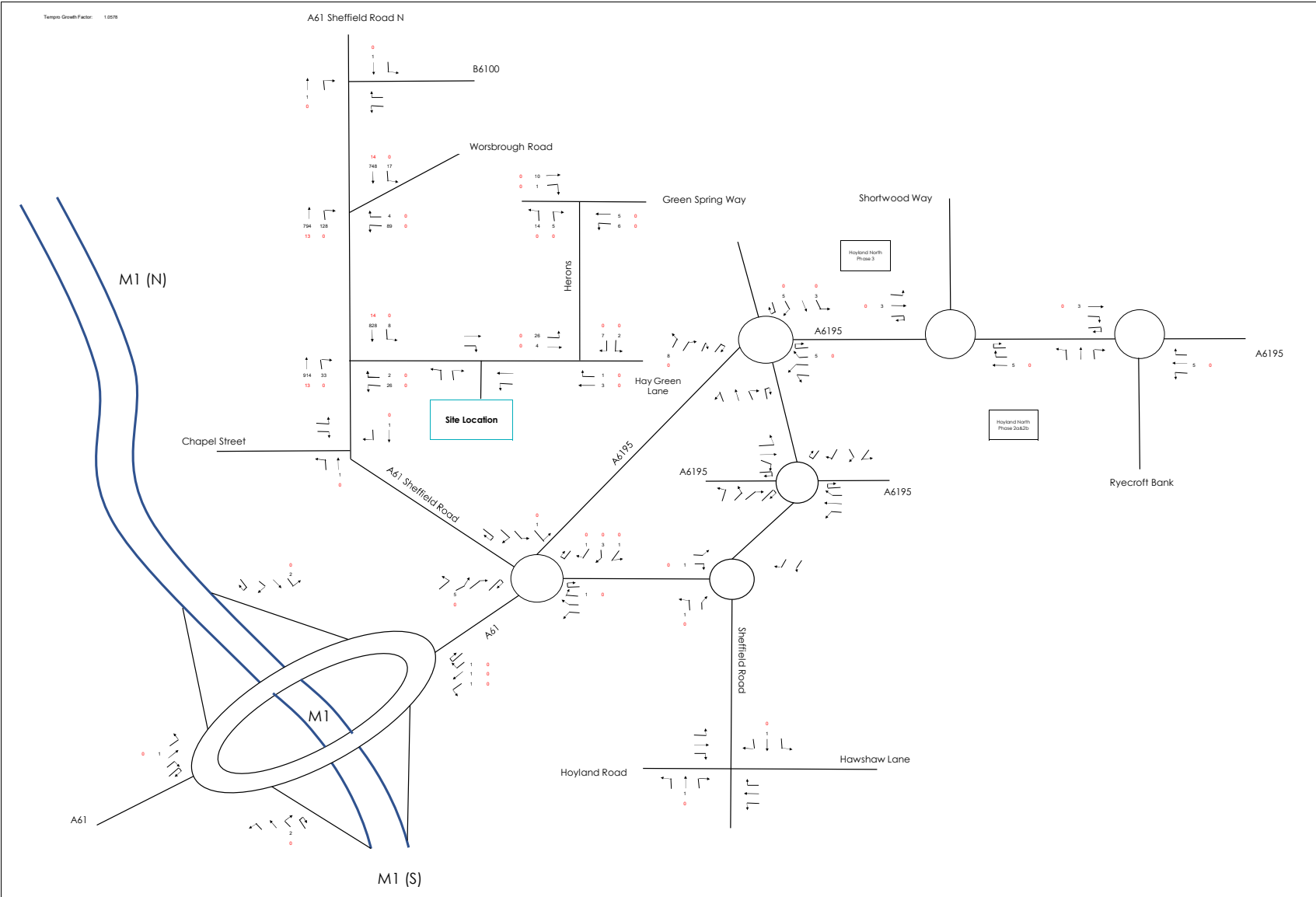


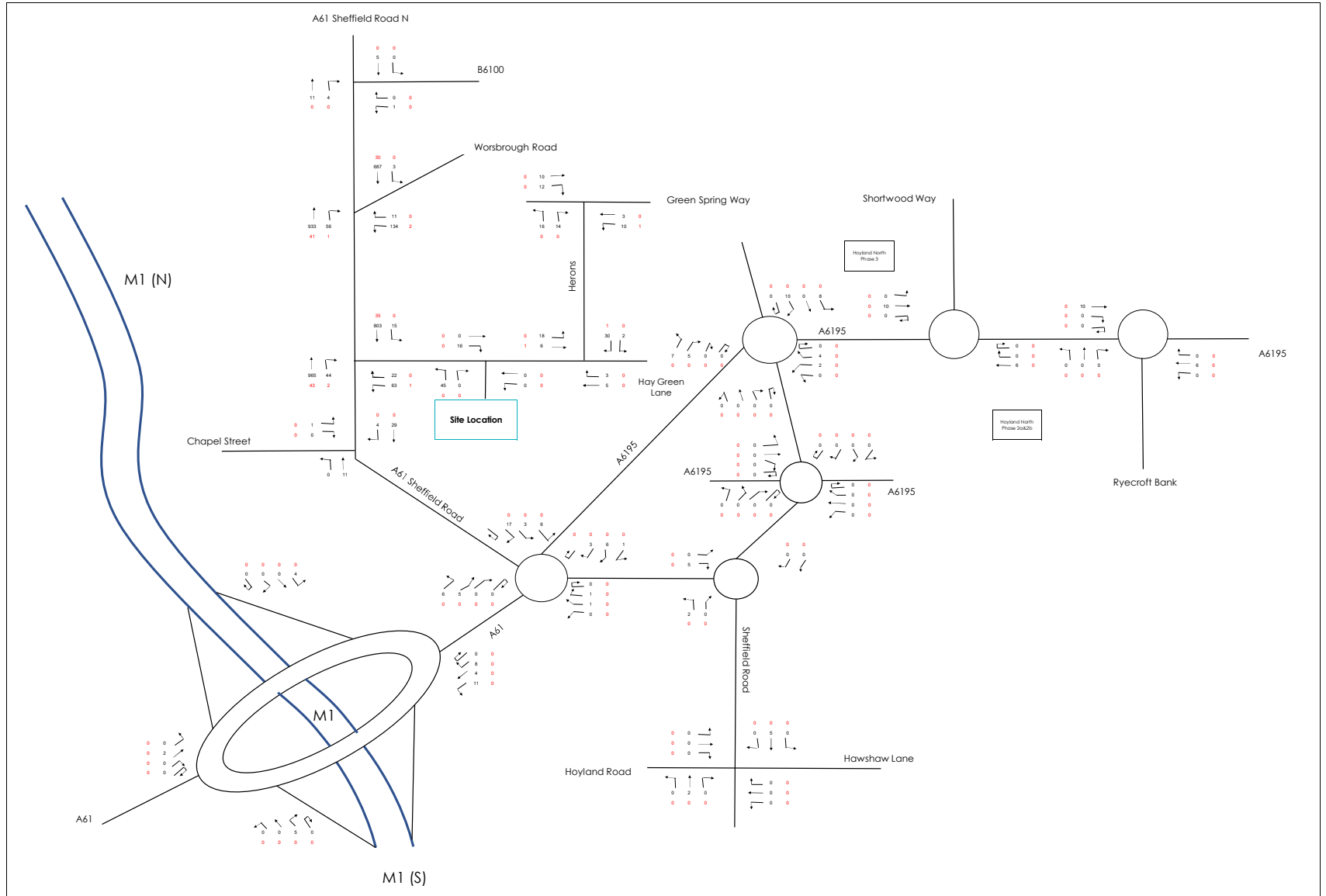


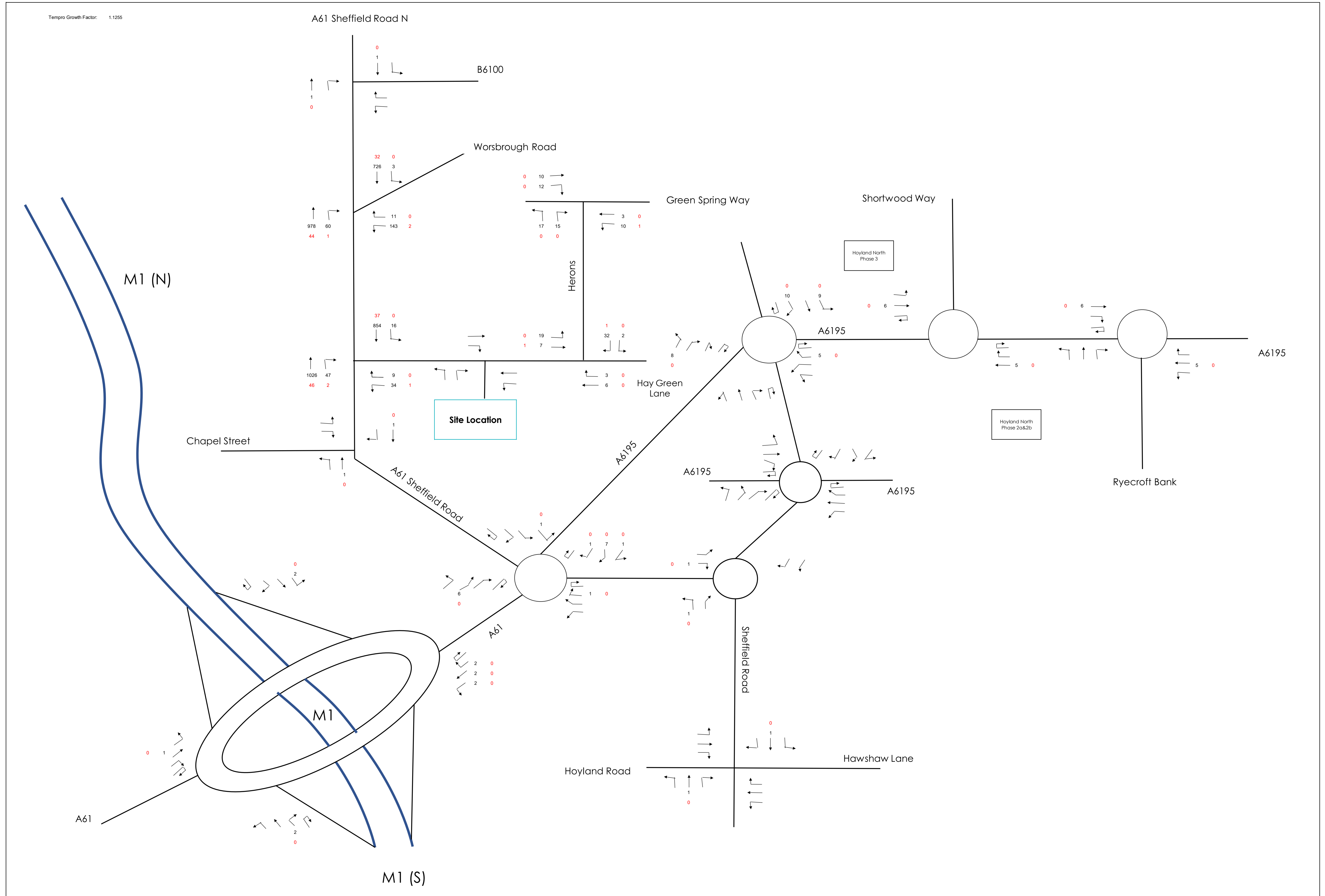
© Hay Green Lane, Barnsley - 2019/20/21/22/23/24/25/26/27/28/29/30/31/32/33/34/35/36/37/38/39/40/41/42/43/44/45/46/47/48/49/50/51/52/53/54/55/56/57/58/59/60/61/62/63/64/65/66/67/68/69/70/71/72/73/74/75/76/77/78/79/80/81/82/83/84/85/86/87/88/89/90/91/92/93/94/95/96/97/98/99/100

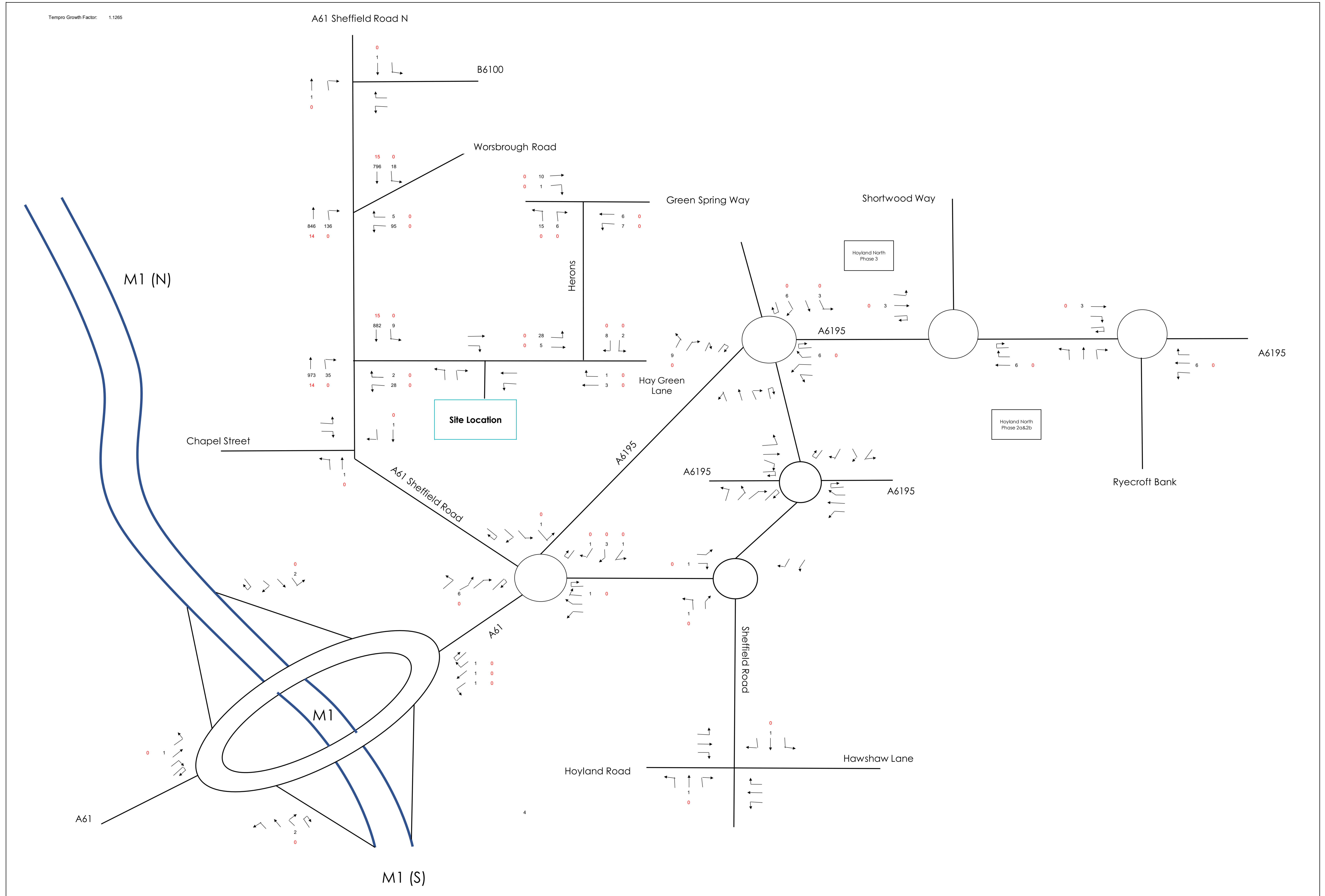


© Hay Green Lane, Barnsley - 2019 Base PM Flows (Copy of 20201115).docx

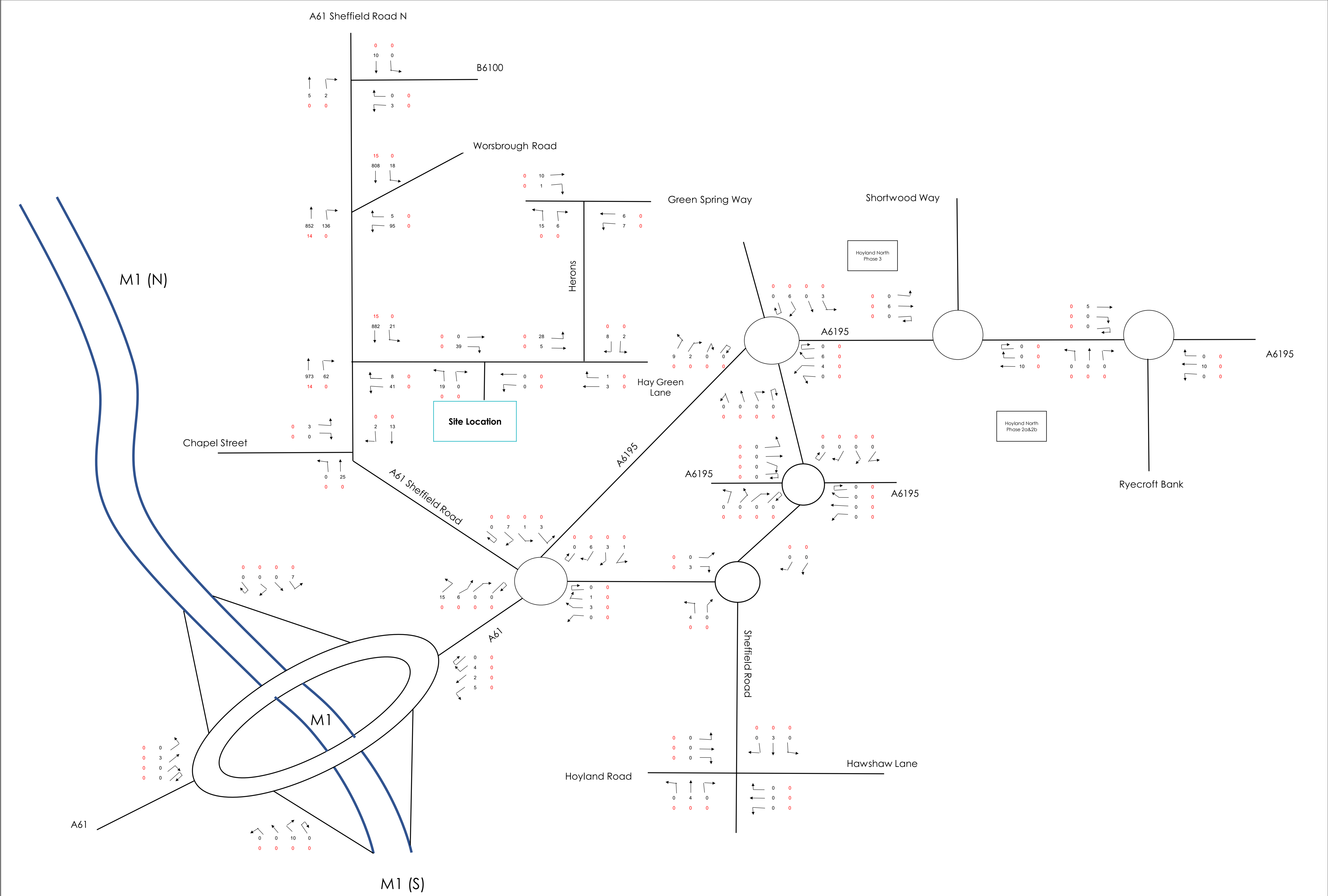


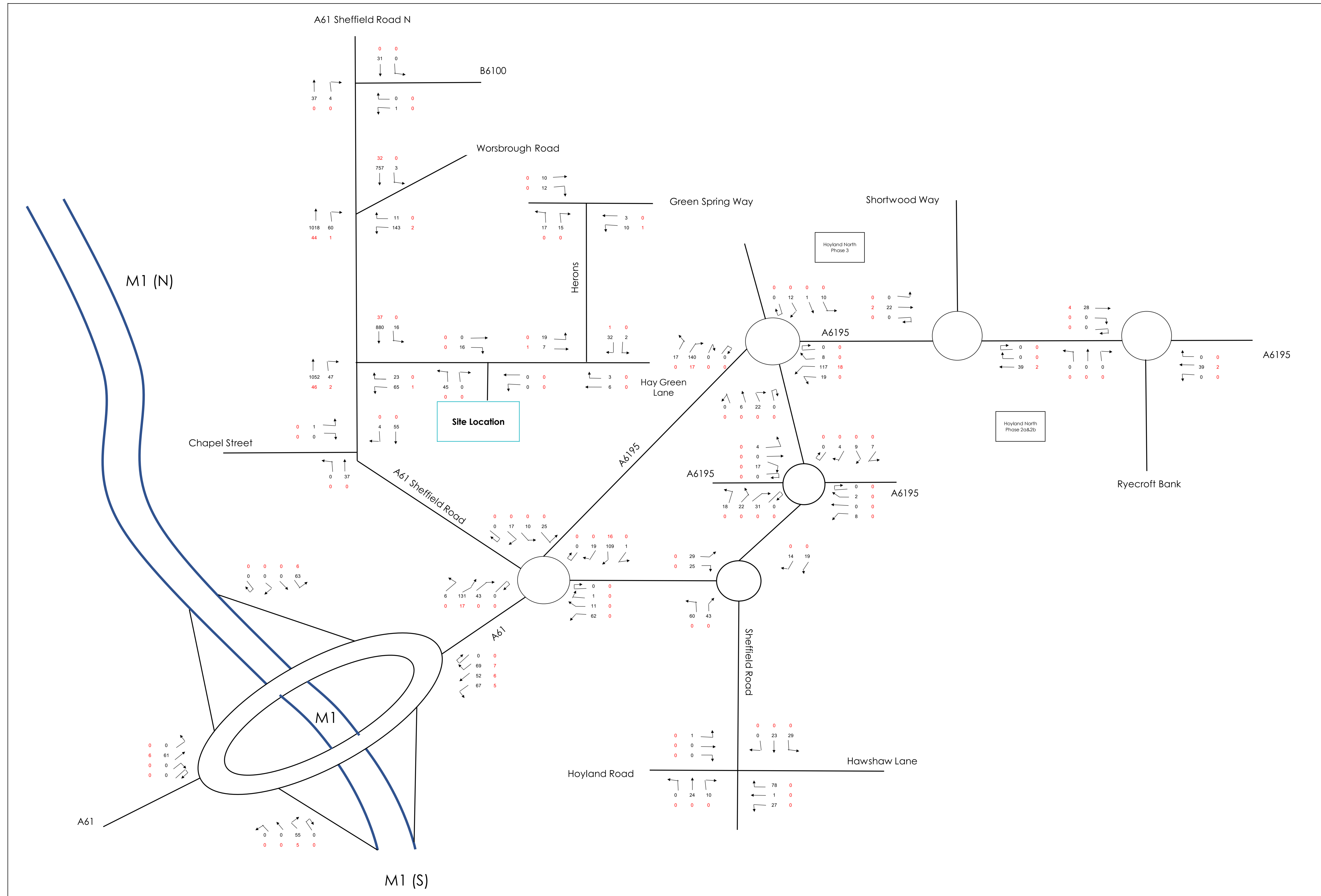


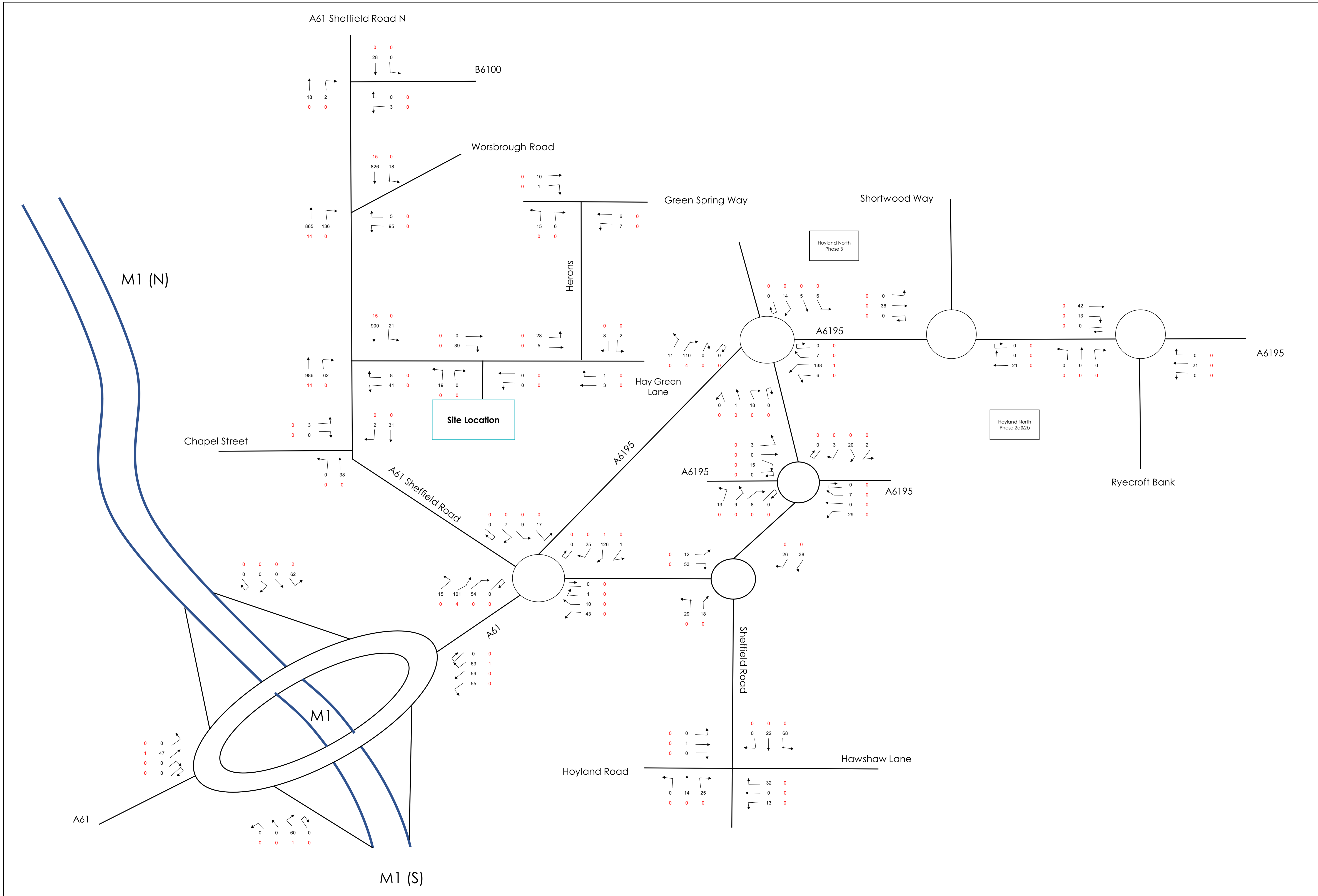




L:\Hay Green Lane, Barnsley - EROW ANALYSIS\SPREADSHEETS\202009 Traffic Flows







L:\Hay Green Lane, Bamsley - EROW ANALYSIS\SPREADSHEETS\202009 Traffic Flow

Appendices

Appendix A Masterplan

Appendix B Surveys

Site 1: Green Spring Avenue/Herons Way A: Green Spring Avenue (East)
 Day: Wednesday B: Herons Way
 Date: 27 November 2019 C: Green Spring Avenue (West)
 Weather: Dull & Rain AM/Dull & Showers PM

A - B									A - C								
Time	Car	LGV	OGV1	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGV1	OGV2	P/C	M/C	PSV	Total	
06:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
06:45	1	0	0	0	0	0	0	1	3	0	0	0	0	0	0	3	
07:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
07:15	1	0	0	0	0	0	0	1	2	0	0	0	0	0	0	2	
07:30	2	0	0	0	0	0	0	2	1	0	0	0	0	0	0	1	
07:45	2	0	0	0	0	0	0	2	3	1	0	0	0	0	0	4	
08:00	2	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	
08:15	0	0	0	0	0	0	0	0	1	1	0	0	0	0	0	2	
08:30	3	2	0	0	0	0	0	5	3	1	0	0	0	0	0	4	
08:45	4	0	0	0	0	0	0	4	2	1	0	0	0	0	0	3	
09:00	1	0	0	0	0	0	0	1	2	0	0	0	0	0	0	2	
09:15	2	0	0	0	0	0	0	2	0	1	0	0	0	0	0	1	
Total	18	2	0	0	0	0	0	20	17	5	0	0	0	0	0	22	

16:30	2	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0
16:45	4	0	0	0	0	0	0	4	1	0	0	0	0	0	0	1
17:00	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1
17:15	0	0	0	0	0	0	0	0	3	0	0	0	0	0	0	3
17:30	0	0	0	0	0	0	0	0	4	0	0	0	0	0	0	4
17:45	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	2
18:00	0	1	0	0	0	0	0	1	1	0	0	0	0	0	0	1
18:15	0	1	0	0	0	0	0	1	0	0	0	0	0	0	0	0
18:30	3	0	0	0	0	0	0	3	2	0	0	0	0	0	0	2
18:45	1	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0
19:00	1	0	0	0	0	0	0	1	2	0	0	0	0	0	0	2
19:15	2	0	0	0	0	0	0	2	2	0	0	0	0	0	0	2
Total	13	2	0	0	0	0	0	15	17	1	0	0	0	0	0	18

B - A									B - C								
Time	Car	LGV	OGV1	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGV1	OGV2	P/C	M/C	PSV	Total	
06:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
06:45	2	0	0	0	0	0	0	2	1	0	0	0	0	0	0	1	
07:00	0	1	0	0	0	0	0	1	0	0	0	0	0	0	0	0	
07:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
07:30	1	0	0	0	0	0	0	1	1	0	0	0	0	0	0	1	
07:45	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	2	
08:00	2	0	0	0	0	0	0	2	1	0	0	0	0	0	0	1	
08:15	2	3	0	0	0	0	0	5	2	0	0	0	0	0	0	2	
08:30	4	0	0	0	0	0	0	4	3	0	0	0	0	0	0	3	
08:45	3	1	0	0	0	0	0	4	7	0	0	0	0	0	0	7	
09:00	7	0	0	0	0	0	0	7	12	1	0	0	0	0	0	13	
09:15	1	0	0	0	0	0	0	1	3	0	0	0	0	0	0	3	
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16:30	3	1	0	0	0	0	0	4	9	0	0	0	0	0	0	9
16:45	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	2
17:00	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
17:15	0	1	0	0	0	0	0	1	1	0	0	0	0	0	0	1
17:30	3	0	0	0	0	0	0	3	2	0	0	0	0	0	0	2
17:45	2	0	0	0	0	0	0	2	3	0	0	0	0	0	0	3
18:00	1	0	0	0	0	0	0	1	2	0	0	0	0	0	0	2
18:15	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
18:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:45	1	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0
19:00	4	0	0	0	0	0	0	4	2	0	0	0	0	0	0	2
19:15	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
Total	14	2	0	0	0	0	0	16	24	0	0	0	0	0	0	24

C - A									C - B								
Time	Car	LGV	OGV1	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGV1	OGV2	P/C	M/C	PSV	Total	
06:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
06:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
07:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
07:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
07:30	1	1	0	0	0	0	0	2	1	0	0	0	0	0	0	1	
07:45	1	0	0	0	0	0	0	1	2	0	0	0	0	0	0	2	
08:00	0	0	0	0	0	0	0	0	3	1	0	0	0	0	0	4	
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08:45	0	0	0	0	0	0	0	0	4	0	0	0	0	0	0	4	
09:00	1	0	0	0	0	0	0	1	2	1	0	0	0	0	0	3	
09:15	1	1	0	0	0	0	0	2	1	0	0	0	0	0	0	1	
Total	7	2	0	0	0	0	0	9	13	2	1	0	0	0	0	16	

16:30	3	1	0	0	0	0	0	4	1	0	0	0	0	0	0	1
16:45	2	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0
17:00	2	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0
17:15	1	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0
17:30	1	0	0	0	0	0	0	1	1	0	0	0	0	0	0	1
17:45	0	1	0	0	0	0	0	1	2	0	0	0	0	0	0	2
18:00	2	1	0	0	0	0	0	3	2	0	0	0	0	0	0	2
18:15	2	0	0	0	0	0	0	2	1	1	0	0	0	0	0	2
18:30	1	0	0	0	0	0	0	1	1	0	0	0	0	0	0	1
18:45	1	0	0	0	0	0	0	1	4	0	0	0	0	0	0	4
19:00	0	1	0	0	0	0	0	1	1	0	0	0	0	0	0	1
19:15	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
Total	15	4	0	0	0	0	0	19	14	1	0	0	0	0	0	15

A - B										A - C									
Time	Car	LGV	OGV1	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGV1	OGV2	P/C	M/C	PSV	Total			
06:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
06:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
07:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
07:15	3	0	0	0	0	0	0	3	1	0	0	0	0	0	0	1			
07:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
07:45	1	0	0	0	0	0	0	1	1	0	0	0	0	0	0	1			
08:00	3	0	0	0	0	0	0	3	0	1	0	0	0	0	0	1			
08:15	3	1	0	0	0	0	0	4	0	2	0	0	0	0	0	2			
08:30	1	0	0	0	0	0	0	1	0	0	1	0	0	0	0	1			
08:45	8	1	0	0	0	0	0	9	1	1	0	0	0	0	0	2			
09:00	14	1	0	0	0	0	0	15	2	0	0	0	0	0	0	2			
09:15	4	0	0	0	0	0	0	4	1	1	0	0	0	0	0	2			
Total	37	3	0	0	0	0	0	40	6	5	1	0	0	0	0	12			

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16:45	4	2	0	0	0	0	0	6	0	0	0	0	0	0	0	0
17:00	2	0	0	0	0	0	0	2	1	0	0	0	0	0	0	1
17:15	2	1	0	0	0	0	0	3	0	0	0	0	0	0	0	0
17:30	8	0	0	0	0	0	0	8	0	0	0	0	0	0	0	0
17:45	7	0	0	0	0	0	0	7	1	0	0	0	0	0	0	1
18:00	4	0	0	0	0	0	0	4	0	0	0	0	0	0	0	0
18:15	1	0	0	0	0	0	0	1	1	0	0	0	0	0	0	1
18:30	3	0	0	0	0	0	0	3	2	0	0	0	0	0	0	2
18:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
19:00	2	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0
19:15	3	0	0	0	0	0	0	3	2	0	0	0	0	0	0	2
Total	47	6	0	0	0	0	0	53	9	1	0	0	0	0	0	10

B - A										B - C									
Time	Car	LGV	OGV1	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGV1	OGV2	P/C	M/C	PSV	Total			
06:30	2	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0			
06:45	2	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0			
07:00	3	0	0	0	0	0	0	3	0	0	0	0	0	0	0	0			
07:15	4	0	0	0	0	0	0	4	0	0	0	0	0	0	0	0			
07:30	5	0	0	0	0	0	0	5	0	0	0	0	0	0	0	0			
07:45	8	1	0	0	0	0	0	9	0	1	0	0	0	0	0	1			
08:00	7	3	0	0	0	0	0	10	1	0	0	0	0	0	0	1			
08:15	1	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0			
08:30	6	2	1	0	0	0	0	9	0	1	0	0	0	0	0	1			
08:45	8	0	0	0	0	0	0	8	0	0	0	0	0	0	0	0			
09:00	3	0	0	0	0	0	0	3	1	0	0	0	0	0	0	1			
09:15	2	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0			
Total	51	6	1	0	0	0	0	58	2	2	0	0	0	0	0	4			

16:30	2	1	0	0	0	0	0	3	1	0	0	0	0	0	0	1
16:45	1	1	0	0	0	0	0	2	1	0	0	0	0	0	0	1
17:00	1	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0
17:15	1	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0
17:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45	3	0	0	0	0	0	0	3	0	0	0	0	0	0	0	0
18:00	1	1	0	0	0	0	0	2	0	0	0	0	0	0	0	0
18:15	1	1	0	0	0	0	0	2	0	0	0	0	0	0	0	0
18:30	4	1	0	0	0	0	0	5	1	0	0	0	0	0	0	1
18:45	3	0	0	0	0	0	0	3	4	0	0	0	0	0	0	4
19:00	2	0	0	0	0	0	0	2	1	0	0	0	0	0	0	1
19:15	2	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0
Total	21	5	0	0	0	0	0	26	8	0	0	0	0	0	0	8

C - A										C - B									
Time	Car	LGV	OGV1	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGV1	OGV2	P/C	M/C	PSV	Total			
06:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
06:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
07:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
07:15	1	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0			
07:30	1	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0			
07:45	1	1	0	0	0	0	0	2	1	0	0	0	0	0	0	1			
08:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
08:15	2	1	0	0	0	0	0	3	0	1	0	0	0	0	0	1			
08:30	1	1	0	0	0	0	0	2	0	0	0	0	0	0	0	0			
08:45	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	2			
09:00	1	0	0	0	0	0	0	1	2	0	0	0	0	0	0	2			
09:15	2	1	0	0	0	0	0	3	0	0	0	0	0	0	0	0			
Total	9	4	0	0	0	0	0	13	5	1	0	0	0	0	0	6			

16:30	1	1	0	0	0	0	0	2	1	0	0	0	0	0	0	1
16:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:00	1	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0
17:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30	2	0	0	0	0	0	0	2	1	0	0	0	0	0	0	1
17:45	2	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0
18:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:15	1	0	0	0	0	0	0	1	1	0	0	0	0	0	0	1
18:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:45	1	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0
19:00	1	0	0	0	0	0	0	1	1	0	0	0	0	0	0	1
19:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	9	1	0	0	0	0	0	10	4	0	0	0	0	0	0	4

Site 3: A61 Sheffield Road/Hay Green Lane A: A61 Barnsley
 Day: Tuesday B: Hay Green Lane
 Date: 26 November 2019 C: A61 Sheffield
 Weather: Dull & Rain AM/Fine & Dull PM

A - B										A - C							
Time	Car	LGV	OGV1	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGV1	OGV2	P/C	M/C	PSV	Total	
06:30	0	0	0	0	0	0	0	0	119	41	4	2	0	0	2	168	
06:45	0	0	0	0	0	0	0	0	117	44	2	3	0	1	2	169	
07:00	1	0	0	0	0	0	0	1	147	54	5	3	3	3	6	221	
07:15	0	0	0	0	0	0	0	0	122	34	4	2	0	0	4	166	
07:30	0	1	0	0	0	0	0	1	132	40	7	5	1	0	5	190	
07:45	1	1	0	0	0	0	0	2	126	30	2	2	0	1	2	163	
08:00	2	0	0	0	0	0	0	2	132	41	13	3	0	0	2	191	
08:15	0	0	0	0	0	0	0	0	182	32	5	3	0	0	3	225	
08:30	3	0	0	0	0	0	0	3	124	25	2	3	0	2	4	160	
08:45	9	0	0	0	0	0	0	9	143	33	1	3	0	1	1	182	
09:00	4	0	0	0	0	0	0	4	108	20	6	3	0	0	1	138	
09:15	2	0	0	0	0	0	0	2	126	16	5	1	0	1	5	154	
Total	22	2	0	0	0	0	0	24	1578	410	56	33	4	9	37	2127	

16:30	3	1	0	0	0	0	0	4	171	27	3	3	0	1	4	209
16:45	1	0	0	0	0	0	0	1	170	15	3	2	2	0	2	194
17:00	1	0	0	0	0	0	0	1	159	31	1	0	0	0	3	194
17:15	2	0	0	0	0	0	0	2	163	16	1	0	0	2	3	185
17:30	3	0	0	0	0	0	0	3	161	14	2	0	0	3	5	185
17:45	0	0	0	0	0	0	0	0	142	13	1	0	1	0	1	158
18:00	1	0	0	0	0	0	0	1	152	9	1	0	0	0	3	165
18:15	0	0	0	0	0	0	0	0	147	18	2	1	0	0	2	170
18:30	1	0	0	0	0	0	0	1	136	16	1	3	0	1	4	161
18:45	2	0	0	0	0	0	0	2	125	7	1	1	0	0	1	135
19:00	2	0	0	0	0	0	0	2	121	12	0	0	0	0	1	134
19:15	1	0	0	0	0	0	0	1	132	17	1	3	0	0	1	154
Total	17	1	0	0	0	0	0	18	1779	195	17	13	3	7	30	2044

B - A									B - C							
Time	Car	LGV	OGV1	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGV1	OGV2	P/C	M/C	PSV	Total
06:30	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
06:45	2	0	0	0	0	0	0	2	1	0	0	0	0	0	0	1
07:00	1	1	0	0	0	0	0	2	2	0	0	0	0	0	0	2
07:15	0	0	0	0	0	0	0	0	7	0	0	0	0	0	0	7
07:30	0	0	0	0	0	0	0	0	8	0	0	0	0	0	0	8
07:45	2	0	0	0	0	0	0	2	11	4	0	0	0	0	0	15
08:00	2	4	0	0	0	0	0	6	11	1	0	0	0	0	0	12
08:15	0	0	0	0	0	0	0	0	4	0	0	0	0	0	0	4
08:30	0	1	0	0	0	0	0	1	4	1	1	0	0	0	0	6
08:45	1	0	0	0	0	0	0	1	7	1	0	0	0	0	0	8
09:00	2	0	0	0	0	0	0	2	10	1	0	0	0	0	0	11
09:15	4	0	0	0	0	0	0	4	4	0	1	0	0	0	0	5
Total	14	6	0	0	0	0	0	20	70	8	2	0	0	0	0	80

16:30	1	0	0	0	0	0	0	1	6	2	0	0	0	0	0	8
16:45	1	0	0	0	0	0	0	1	3	3	0	0	0	0	0	6
17:00	0	0	0	0	0	0	0	0	4	2	0	0	0	0	0	6
17:15	0	0	0	0	0	0	0	0	5	0	0	0	0	0	0	5
17:30	2	0	0	0	0	0	0	2	7	1	0	0	0	0	0	8
17:45	1	1	0	0	0	0	0	2	6	1	0	0	0	0	0	7
18:00	0	1	0	0	0	0	0	1	4	2	0	0	0	0	0	6
18:15	2	0	0	0	0	0	0	2	4	0	0	0	0	0	0	4
18:30	1	0	0	0	0	0	0	1	9	1	0	0	0	0	0	10
18:45	3	0	0	0	0	0	0	3	7	1	0	0	0	0	0	8
19:00	1	0	0	0	0	0	0	1	2	1	0	0	0	0	0	3
19:15	0	0	0	0	0	0	0	0	6	0	0	0	0	0	0	6
Total	12	2	0	0	0	0	0	14	63	14	0	0	0	0	0	77

C - A									C - B							
Time	Car	LGV	OGV1	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGV1	OGV2	P/C	M/C	PSV	Total
06:30	49	10	2	4	0	0	3	68	0	0	0	0	0	0	0	0
06:45	42	17	2	1	0	0	2	64	0	0	0	0	0	0	0	0
07:00	63	14	2	2	1	1	3	86	1	1	0	0	0	0	0	2
07:15	103	27	2	1	0	0	0	133	4	2	0	0	0	0	0	6
07:30	141	27	5	7	0	0	6	186	3	0	0	0	0	0	0	3
07:45	178	30	6	5	0	0	2	221	2	0	0	0	0	0	0	2
08:00	169	28	5	7	1	0	0	210	10	2	0	0	0	0	0	12
08:15	204	30	6	2	0	0	3	245	4	0	1	0	0	0	0	5
08:30	202	23	6	4	0	2	6	243	14	0	0	0	0	0	0	14
08:45	176	22	8	3	0	1	3	213	8	2	1	0	0	0	0	11
09:00	150	29	4	3	1	0	2	189	4	0	0	0	0	0	0	4
09:15	126	24	2	2	0	0	8	162	5	1	0	0	0	0	0	6
Total	1603	281	50	41	3	4	38	2020	55	8	2	0	0	0	0	65

16:30	152	47	2	0	0	1	3	205	7	0	0	0	0	0	0	7
16:45	170	44	2	2	0	1	2	221	7	1	0	0	0	0	0	8
17:00	183	38	1	1	1	3	2	229	6	1	0	0	0	0	0	7
17:15	166	32	1	3	1	2	3	208	8	1	0	0	0	0	0	9
17:30	196	28	2	1	0	0	2	229	10	0	0	0	0	0	0	10
17:45	202	30	0	2	0	0	3	237	6	1	0	0	0	0	0	7
18:00	198	30	1	1	0	0	5	235	8	1	0	0	0	0	0	9
18:15	156	17	2	3	1	0	3	182	5	0	0	0	0	0	0	5
18:30	141	27	0	0	1	1	5	175	4	0	0	0	0	0	0	4
18:45	115	11	1	0	0	0	1	128	4	1	0	0	0	0	0	5
19:00	95	10	0	1	0	0	2	108	4	1	0	0	0	0	0	5
19:15	103	14	1	0	0	0	1	119	4	0	0	0	0	0	0	4
Total	1877	328	13	14	4	8	32	2276	73	7	0	0	0	0	0	80

Time	A - B							A - C							A - D									
	Car	LGV	OGV1	OGV2	PIC	M/C	PSV	Total	Car	LGV	OGV1	OGV2	PIC	M/C	PSV	Total	Car	LGV	OGV1	OGV2	PIC	M/C	PSV	Total
06:30	0	2	0	0	0	0	0	2	118	37	3	3	0	0	1	162	0	0	0	0	0	0	0	0
06:45	0	0	0	0	0	0	0	0	97	39	1	2	0	2	1	142	0	0	0	0	0	0	0	0
07:00	0	0	0	0	0	0	0	0	130	53	5	4	0	1	5	198	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0	0	112	38	5	1	0	0	5	161	0	0	0	0	0	0	0	0
07:30	1	0	0	0	0	0	0	1	116	27	6	6	0	0	2	157	0	0	0	0	0	0	0	0
07:45	0	0	1	0	0	0	0	1	110	28	3	2	0	1	1	145	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0	126	38	11	2	0	0	2	179	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	0	0	0	146	29	4	3	0	0	2	184	0	0	0	0	0	0	0	0
08:30	2	1	0	0	0	0	0	3	110	20	1	3	0	2	2	138	0	0	0	0	0	0	0	0
08:45	0	0	0	0	0	0	0	0	108	29	1	3	0	1	1	143	0	0	0	0	0	0	0	0
09:00	0	0	0	0	0	0	0	0	99	17	5	3	0	0	0	124	0	0	0	0	0	0	0	0
09:15	1	0	0	0	0	0	0	1	105	24	6	1	0	1	3	140	0	0	0	0	0	0	0	0
Total	4	3	1	0	0	0	0	8	1377	379	51	33	0	8	25	1873	0	0	0	0	0	0	0	0

Time	A - B							A - C							A - D									
	Car	LGV	OGV1	OGV2	PIC	M/C	PSV	Total	Car	LGV	OGV1	OGV2	PIC	M/C	PSV	Total	Car	LGV	OGV1	OGV2	PIC	M/C	PSV	Total
16:30	1	1	0	0	0	0	0	2	149	25	3	3	0	1	2	183	0	0	0	0	0	0	0	0
16:45	5	1	0	0	0	0	0	6	153	14	3	2	2	0	2	176	0	0	0	0	0	0	0	0
17:00	2	1	0	0	0	0	0	3	148	28	1	0	0	0	1	178	1	0	0	0	0	0	0	1
17:15	4	1	0	0	0	0	0	5	150	14	1	0	0	2	2	169	0	0	0	0	0	0	0	0
17:30	2	0	0	0	0	0	0	2	148	13	2	0	0	1	3	167	1	0	0	0	0	0	0	1
17:45	3	2	0	0	0	0	0	5	129	9	1	0	1	0	1	141	0	0	0	0	0	0	0	0
18:00	2	0	0	0	0	0	0	2	134	7	1	0	0	0	1	143	0	0	0	0	0	0	0	0
18:15	3	1	0	0	0	0	0	4	129	16	1	1	0	0	2	149	1	0	0	0	0	0	0	1
18:30	5	0	0	0	0	0	0	5	107	14	1	3	0	0	2	127	0	0	0	0	0	0	0	0
18:45	1	0	0	0	0	0	0	1	108	7	0	1	0	0	1	117	0	0	0	0	0	0	0	0
19:00	3	0	0	0	0	0	0	3	115	12	0	0	0	0	0	127	0	0	0	0	0	0	0	0
19:15	2	0	0	0	0	0	0	2	116	16	1	3	0	0	0	136	0	0	0	0	0	0	0	0
Total	33	7	0	0	0	0	0	40	1586	175	15	13	3	4	17	1813	3	0	0	0	0	0	0	3

Time	B - A							B - C							B - D									
	Car	LGV	OGV1	OGV2	PIC	M/C	PSV	Total	Car	LGV	OGV1	OGV2	PIC	M/C	PSV	Total	Car	LGV	OGV1	OGV2	PIC	M/C	PSV	Total
06:30	1	0	0	0	0	0	0	1	8	3	0	0	0	0	1	12	0	0	0	0	0	0	0	0
06:45	1	0	0	0	0	0	0	1	7	3	1	0	0	0	3	14	0	0	0	0	0	0	0	0
07:00	3	0	0	0	0	0	0	3	12	1	0	0	1	0	0	14	0	0	0	0	0	0	0	0
07:15	3	0	0	0	0	0	0	3	17	5	1	0	0	0	1	24	0	0	0	0	0	0	0	0
07:30	2	0	0	0	0	0	0	2	19	4	0	0	0	0	1	24	0	0	0	0	0	0	0	0
07:45	2	0	0	0	0	0	0	2	18	2	0	0	0	1	1	22	0	0	0	0	0	0	0	0
08:00	4	0	0	0	0	0	0	4	25	3	2	0	0	0	1	31	0	0	0	0	0	0	0	0
08:15	2	0	0	0	0	0	0	2	19	3	0	0	0	0	0	22	0	0	0	0	0	0	0	0
08:30	2	1	0	0	0	0	0	3	28	4	0	0	0	2	34	0	0	0	0	0	0	0	0	0
08:45	1	0	0	0	0	0	0	1	35	5	0	0	0	0	0	40	0	0	0	0	0	0	0	0
09:00	0	1	0	0	0	0	0	1	14	1	0	0	0	0	1	16	0	0	0	0	0	0	0	0
09:15	0	0	0	0	0	0	0	0	21	1	1	0	0	0	2	25	0	0	0	0	0	0	0	0
Total	21	2	0	0	0	0	0	23	223	35	5	0	1	1	13	278	0	0	0	0	0	0	0	0

Time	C - A							C - B							C - D									
	Car	LGV	OGV1	OGV2	PIC	M/C	PSV	Total	Car	LGV	OGV1	OGV2	PIC	M/C	PSV	Total	Car	LGV	OGV1	OGV2	PIC	M/C	PSV	Total
16:30	0	0	0	0	0	0	0	0	22	3	0	0	0	0	1	26	0	0	0	0	0	0	0	0
16:45	2	0	0	0	0	0	0	2	18	1	0	0	0	0	0	19	1	0	0	0	0	0	0	1
17:00	0	0	0	0	0	0	0	0	12	3	0	0	0	0	2	17	0	0	0	0	0	0	0	0
17:15	2	0	0	0	0	0	0	2	19	2	0	0	0	0	1	22	0	0	0	0	0	0	0	0
17:30	0	0	0	0	0	0	0	0	17	1	0	0	0	2	2	22	0	0	0	0	0	0	0	0
17:45	3	0	0	0	0	0	0	3	13	4	0	0	0	0	0	17	0	0	0	0	0	0	0	0
18:00	2	0	0	0	0	0	0	2	18	2	0	0	0	0	2	22	0	0	0	0	0	0	0	0
18:15	1	0	0	0	0	0	0	1	19	1	1	0	0	1	0	22	0	0	0	0	0	0	0	0
18:30	2	0	0	0	0	0	0	2	14	2	0	0	0	0	2	18	0	0	0	0	0	0	0	0
18:45	2	0	0	0	0	0	0	2	20	1	1	0	0	0	0	22	0	0	0	0	0	0	0	0
19:00	0	0	0	0	0	0	0	0	15	0	0	0	0	0	1	16	0	0	0	0	0	0	0	0
19:15	1	0	0	0	0	0	0	1	11	1	0	0	0	0	1	13	0	0	0	0	0	0	0	0
Total	15	0	0	0	0	0	0	15	198	21	2	0	0	3	12	236	1	0	0	0	0	0	0	1

Time	D - A							D - B							D - C									
	Car	LGV	OGV1	OGV2	PIC	M/C	PSV	Total	Car	LGV	OGV1	OGV2	PIC	M/C	PSV	Total	Car	LGV	OGV1	OGV2	PIC	M/C	PSV	Total
06:30	48	7	3	4	0	0	1	63	3	0	0	0	0	0	2	5	0	0	0	0	0	0	0	0
06:45	41	16	2	1	0	0	1	61	1	0	0	0	0	1	2	0	0	0	0	0	0	0	0	0
07:00	65	16	2	2	1	1	1	88	2	0	0	0	0	0	2	4	0	0	0	0	0	0	0	0
07:15	102	23	3	1	0	0	0	129	6	0	0	0	0	0	6	0	0	0	0	0	0	0	0	0
07:30	134	30	5	7	1	0	5	182	2	1	0	0	0	1	4	0	0	0	0	0	0	0	0	0
07:45	183	30	6	5	0	0	2	226	1	2	0	0	0	0	3	0	0	0	0	0	0	0	0	0
08:00	165	28	5	6	1	0	0	205	8	1	1	0	0	0	10	1	0	0	0	0	0	0	1	0
08:15	199	30	6	2	0	0	1	238	7	2	0	0	0	1	10	0	0	0	0	0	0	0	0	0
08:30	192	23	5	4	0	2	5	231	12	1	0	0	0	2	15	1	0	0	0	0	0	0	1	0
08:45	159	21	7	4	0	1	2	194	14	3	0	0	0	1	18	0	0	0	0	0	0	0	0	0
09:00	136	28	2	3	1	0	2	172	18	2	1	0	0	0	21	0	0	0	0	0	0	0	0	0
09:15	117	18	2	2	0	0	7	146	6	4	0	0	0	0	1	11	0	0	0	0	0	0	0	0
Total	1541	270	48	41	4	4	27	1935	80	16	2	0	0	0	11	109	2	0	0	0	0	0	0	2

Time	D - A							D - B							D - C								
	Car	LGV	OGV1	OGV2	PIC	M/C	PSV	Total	Car	LGV	OGV1	OGV2	PIC	M/C	PSV	Total	Car	LGV	OGV1	OGV2	PIC	M/C	PSV
16:30																							

Site 1:	Green Spring Avenue/Herons Way	A:	Green Spring Avenue (East)
Day:	Wednesday	B:	Herons Way
Date:	23 September 2020	C:	Green Spring Avenue/ (West)
Weather:	Overcast & Rain		

Time	From A								From B							
	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total
14:30	3	0	1	0	0	0	0	4	2	1	0	0	0	0	0	3
14:45	2	2	0	0	0	0	0	4	2	1	0	0	0	0	0	3
15:00	3	1	0	0	0	0	0	4	6	2	0	0	0	0	0	8
15:15	2	0	0	0	0	0	0	2	8	1	0	0	0	0	0	9
15:30	3	1	0	0	0	0	0	4	15	0	0	0	0	0	0	15
15:45	2	0	0	0	0	0	0	2	4	0	0	0	0	0	0	4
16:00	2	1	0	0	0	0	0	3	3	0	0	0	0	0	0	3
16:15	7	0	0	0	0	0	0	7	3	1	0	0	1	0	0	5
16:30	4	1	0	0	0	0	0	5	3	1	0	0	0	0	0	4
16:45	3	1	0	0	0	0	0	4	5	0	0	0	0	0	0	5
17:00	0	2	0	0	0	0	0	2	4	0	0	0	0	0	0	4
17:15	0	1	0	0	0	0	0	1	4	1	0	0	0	0	0	5
17:30	1	1	0	0	0	0	0	2	0	1	0	0	0	0	0	1
17:45	0	0	0	0	0	0	0	0	3	1	0	0	0	1	0	5
18:00	3	1	0	0	0	0	0	4	2	0	0	0	0	0	0	2
18:15	2	0	0	0	0	0	0	2	4	0	0	0	0	0	0	4
Total	37	12	1	0	0	0	0	50	68	10	0	0	1	1	0	80

Time	From C								To A							
	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total
14:30	3	0	1	0	0	0	0	4	1	0	1	0	0	0	0	2
14:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:00	5	0	0	0	0	0	0	5	4	1	0	0	0	0	0	5
15:15	6	1	0	0	0	0	0	7	4	2	0	0	0	0	0	6
15:30	3	1	0	0	0	0	0	4	9	1	0	0	0	0	0	10
15:45	7	0	0	0	0	0	0	7	5	0	0	0	0	0	0	5
16:00	3	1	0	0	0	0	0	4	2	1	0	0	0	0	0	3
16:15	3	1	0	0	0	0	0	4	3	1	0	0	1	0	0	5
16:30	2	1	0	0	0	0	0	3	3	2	0	0	0	0	0	5
16:45	4	0	0	0	0	0	0	4	3	0	0	0	0	0	0	3
17:00	2	0	0	0	0	0	0	2	1	0	0	0	0	0	0	1
17:15	3	0	0	0	0	1	0	4	2	1	0	0	0	0	0	3
17:30	2	0	0	0	0	0	0	2	1	0	0	0	0	0	0	1
17:45	4	1	0	0	0	0	0	5	2	1	0	0	0	0	0	3
18:00	4	0	0	0	1	0	0	5	4	0	0	0	0	0	0	4
18:15	4	0	0	0	0	0	0	4	4	0	0	0	0	0	0	4
Total	55	6	1	0	1	1	0	64	48	10	1	0	1	0	0	60

Time	To B								To C							
	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total
14:30	3	0	0	0	0	0	0	3	4	1	1	0	0	0	0	6
14:45	2	1	0	0	0	0	0	3	2	2	0	0	0	0	0	4
15:00	6	1	0	0	0	0	0	7	4	1	0	0	0	0	0	5
15:15	5	0	0	0	0	0	0	5	7	0	0	0	0	0	0	7
15:30	3	0	0	0	0	0	0	3	9	1	0	0	0	0	0	10
15:45	5	0	0	0	0	0	0	5	3	0	0	0	0	0	0	3
16:00	3	0	0	0	0	0	0	3	3	1	0	0	0	0	0	4
16:15	6	0	0	0	0	0	0	6	4	1	0	0	0	0	0	5
16:30	1	0	0	0	0	0	0	1	5	1	0	0	0	0	0	6
16:45	4	1	0	0	0	0	0	5	5	0	0	0	0	0	0	5
17:00	2	1	0	0	0	0	0	3	3	1	0	0	0	0	0	4
17:15	3	0	0	0	0	1	0	4	2	1	0	0	0	0	0	3
17:30	1	1	0	0	0	0	0	2	1	1	0	0	0	0	0	2
17:45	3	0	0	0	0	0	0	3	2	1	0	0	0	1	0	4
18:00	2	1	0	0	1	0	0	4	3	0	0	0	0	0	0	3
18:15	1	0	0	0	0	0	0	1	5	0	0	0	0	0	0	5
Total	50	6	0	0	1	1	0	58	62	12	1	0	0	1	0	76

Site 2:	Hay Green Lane/Herons Way	A:	Hay Green Lane (West)
Day:	Wednesday	B:	Herons Way
Date:	23 September 2020	C:	Hay Green Lane (East)
Weather:	Overcast & Rain		

Time	From A								From B							
	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total
14:30	3	1	0	0	0	0	0	4	2	0	0	0	0	0	0	2
14:45	3	1	0	0	0	0	0	4	1	2	0	0	0	0	0	3
15:00	7	2	0	0	0	0	0	9	3	2	0	0	0	0	0	5
15:15	9	2	0	0	0	0	0	11	3	0	0	0	0	0	0	3
15:30	15	0	0	0	0	0	0	15	2	0	0	0	0	0	0	2
15:45	6	0	0	0	0	0	0	6	4	1	0	0	0	0	0	5
16:00	5	1	0	0	0	0	0	6	3	0	0	0	0	0	0	3
16:15	4	0	0	0	1	0	0	5	4	0	0	0	0	0	0	4
16:30	5	2	0	0	0	0	0	7	1	0	0	0	0	0	0	1
16:45	3	0	0	0	0	0	0	3	3	0	0	0	0	0	0	3
17:00	6	1	0	0	1	0	0	8	6	0	0	0	0	0	0	6
17:15	5	0	0	0	0	0	0	5	4	1	0	0	0	1	0	6
17:30	1	1	0	0	0	0	0	2	3	0	0	0	0	0	0	3
17:45	5	1	0	0	0	1	0	7	2	0	0	0	0	1	0	3
18:00	5	1	1	0	0	0	0	7	1	1	0	0	1	0	0	3
18:15	3	0	0	0	1	1	0	5	2	1	0	0	0	0	0	3
Total	85	13	1	0	3	2	0	104	44	8	0	0	1	2	0	55

Time	From C								To A							
	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total
14:30	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	2
14:45	1	0	0	0	0	0	0	1	2	2	0	0	0	0	0	4
15:00	0	0	0	0	0	0	0	0	3	1	0	0	0	0	0	4
15:15	1	0	0	0	0	0	0	1	4	0	0	0	0	0	0	4
15:30	1	0	0	0	0	0	0	1	3	0	0	0	0	0	0	3
15:45	0	1	0	0	0	0	0	1	3	1	0	0	0	0	0	4
16:00	2	0	0	0	0	0	0	2	4	0	0	0	0	0	0	4
16:15	1	0	0	0	0	0	0	1	4	0	0	0	0	0	0	4
16:30	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
16:45	3	0	0	0	0	0	0	3	4	0	0	0	0	0	0	4
17:00	1	0	0	0	0	0	0	1	6	0	0	0	0	0	0	6
17:15	0	0	0	0	0	0	0	0	3	1	0	0	0	1	0	5
17:30	1	0	0	0	0	0	0	1	3	0	0	0	0	0	0	3
17:45	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	2
18:00	3	0	0	0	0	0	0	3	4	1	0	0	1	0	0	6
18:15	1	0	0	0	0	0	0	1	2	1	0	0	0	0	0	3
Total	15	1	0	0	0	0	0	16	49	7	0	0	1	2	0	59

Time	To B								To C							
	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total
14:30	3	1	0	0	0	0	0	4	0	0	0	0	0	0	0	0
14:45	2	1	0	0	0	0	0	3	1	0	0	0	0	0	0	1
15:00	6	2	0	0	0	0	0	8	1	1	0	0	0	0	0	2
15:15	7	2	0	0	0	0	0	9	2	0	0	0	0	0	0	2
15:30	14	0	0	0	0	0	0	14	1	0	0	0	0	0	0	1
15:45	5	1	0	0	0	0	0	6	2	0	0	0	0	0	0	2
16:00	4	1	0	0	0	0	0	5	2	0	0	0	0	0	0	2
16:15	4	0	0	0	1	0	0	5	1	0	0	0	0	0	0	1
16:30	3	2	0	0	0	0	0	5	2	0	0	0	0	0	0	2
16:45	4	0	0	0	0	0	0	4	1	0	0	0	0	0	0	1
17:00	4	1	0	0	1	0	0	6	3	0	0	0	0	0	0	3
17:15	5	0	0	0	0	0	0	5	1	0	0	0	0	0	0	1
17:30	0	1	0	0	0	0	0	1	2	0	0	0	0	0	0	2
17:45	4	1	0	0	0	1	0	6	2	0	0	0	0	0	0	2
18:00	3	0	0	0	0	0	0	3	2	1	1	0	0	0	0	4
18:15	2	0	0	0	0	1	0	3	2	0	0	0	1	0	0	3
Total	70	13	0	0	2	2	0	87	25	2	1	0	1	0	0	29

Site 3:	A61 Sheffield Rd/Hay Green Ln/Birdwell Ven.	A:	A61 Barnsley
Day:	Tuesday	B:	Hay Green Lane
Date:	22 September 2020	C:	A61 Sheffield
Weather:	Fine & Sunny until 16:40 then Fine & Cloudy	D:	Birdwell Venue

From A

From B

Time	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total
14:30	109	27	3	4	0	1	2	146	5	0	0	0	0	0	0	5
14:45	122	17	5	4	0	0	2	150	5	1	0	0	0	1	0	7
15:00	117	13	4	3	0	0	2	139	7	0	0	0	0	0	0	7
15:15	146	24	2	1	1	0	4	178	5	0	1	0	0	0	0	6
15:30	136	23	3	2	1	0	2	167	9	1	0	0	0	0	0	10
15:45	116	30	4	2	0	0	1	153	3	3	0	0	0	0	0	6
16:00	135	33	2	1	3	0	3	177	3	1	0	0	1	0	0	5
16:15	127	21	5	3	1	2	4	163	10	1	0	0	0	0	0	11
16:30	129	19	2	1	1	1	2	155	3	1	0	0	0	0	0	4
16:45	153	22	1	1	0	3	2	182	2	0	0	0	0	0	0	2
17:00	126	29	2	2	0	2	3	164	7	0	0	0	0	0	0	7
17:15	176	21	3	0	1	1	3	205	5	0	0	0	0	0	0	5
17:30	130	15	0	0	2	3	2	152	5	1	0	0	0	0	0	6
17:45	116	17	0	2	0	1	2	138	8	1	0	0	0	0	0	9
18:00	115	9	1	1	0	0	1	127	12	1	0	0	0	0	0	13
18:15	106	14	1	0	0	0	2	123	5	2	0	0	0	0	0	7
Total	2059	334	38	27	10	14	37	2519	94	13	1	0	1	1	0	110

From C

From D

Time	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total
14:30	117	27	4	4	0	2	1	155	0	0	0	0	0	0	0	0
14:45	140	29	6	3	1	3	2	184	1	0	0	0	0	0	0	1
15:00	142	33	5	1	0	0	2	183	1	0	0	0	0	0	0	1
15:15	152	30	4	1	0	4	3	194	1	0	0	0	0	0	0	1
15:30	149	28	6	4	2	1	3	193	4	0	0	0	0	0	0	4
15:45	159	40	2	3	1	2	3	210	0	0	0	0	0	0	0	0
16:00	167	40	7	3	1	1	2	221	1	0	0	0	0	0	0	1
16:15	187	49	2	3	0	2	2	245	1	0	0	0	0	0	0	1
16:30	173	53	2	0	3	1	2	234	0	0	0	0	0	0	0	0
16:45	169	43	4	1	1	1	3	222	0	0	0	0	0	0	0	0
17:00	202	37	4	2	1	2	1	249	0	0	0	0	0	0	0	0
17:15	201	37	2	1	0	2	3	246	1	0	0	0	0	0	0	1
17:30	176	37	2	1	1	1	1	219	0	0	0	0	0	0	0	0
17:45	190	32	0	1	0	2	3	228	0	0	0	0	0	0	0	0
18:00	134	21	2	0	0	1	1	159	0	0	0	0	0	0	0	0
18:15	149	22	0	2	0	1	3	177	2	0	0	0	0	0	0	2
Total	2607	558	52	30	11	26	35	3319	12	0	0	0	0	0	0	12

To A

To B

Time	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total
14:30	112	27	4	4	0	2	1	150	7	1	0	0	0	0	0	8
14:45	132	27	6	3	1	4	2	175	12	3	0	0	0	0	0	15
15:00	129	32	5	1	0	0	2	169	13	0	0	0	0	0	0	13
15:15	147	30	4	1	0	4	3	189	8	0	0	0	0	0	0	8
15:30	149	28	6	4	2	1	3	193	5	0	0	0	0	0	0	5
15:45	150	41	2	3	1	2	3	202	11	1	0	0	0	0	0	12
16:00	161	39	7	3	1	0	2	213	10	1	1	0	0	1	0	13
16:15	181	47	2	3	0	2	2	237	7	2	0	0	0	0	0	9
16:30	161	52	2	0	3	1	2	221	12	0	0	0	0	0	0	12
16:45	165	43	4	1	1	1	3	218	6	0	0	0	0	0	0	6
17:00	199	36	4	2	1	2	1	245	5	2	0	0	0	0	0	7
17:15	192	36	2	1	0	2	3	236	13	2	0	0	1	0	0	16
17:30	176	38	1	1	0	1	1	218	2	0	1	0	1	0	0	4
17:45	184	31	0	1	0	2	3	221	11	2	0	0	0	0	0	13
18:00	135	21	2	0	0	1	1	160	4	0	0	0	0	0	0	4
18:15	144	23	0	2	0	1	3	173	7	0	0	0	0	0	0	7
Total	2517	551	51	30	10	26	35	3220	133	14	2	0	2	1	0	152

To C

To D

Time	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total
14:30	111	26	3	4	0	1	2	147	1	0	0	0	0	0	0	1
14:45	122	17	5	4	0	0	2	150	2	0	0	0	0	0	0	2
15:00	116	13	3	3	0	0	2	137	9	1	1	0	0	0	0	11
15:15	143	24	3	1	1	0	4	176	6	0	0	0	0	0	0	6
15:30	143	22	3	2	1	0	2	173	1	2	0	0	0	0	0	3
15:45	113	31	4	2	0	0	1	151	4	0	0	0	0	0	0	4
16:00	135	34	1	1	4	0	2	177	0	0	0	0	0	0	1	1
16:15	134	21	5	3	1	2	4	170	3	1	0	0	0	0	0	4
16:30	130	20	2	1	1	1	2	157	2	1	0	0	0	0	0	3
16:45	153	22	1	1	0	3	2	182	0	0	0	0	0	0	0	0
17:00	130	28	2	2	0	2	3	167	1	0	0	0	0	0	0	1
17:15	178	20	3	0	0	1	3	205	0	0	0	0	0	0	0	0
17:30	133	15	0	0	2	3	2	155	0	0	0	0	0	0	0	0
17:45	119	17	0	2	0	1	2	141	0	0	0	0	0	0	0	0
18:00	122	10	1	1	0	0	1	135	0	0	0	0	0	0	0	0
18:15	110	15	1	0	0	0	2	128	1	0	0	0	0	0	0	1
Total	2092	335	37	27	10	14	36	2551	30	5	1	0	0	0	1	37

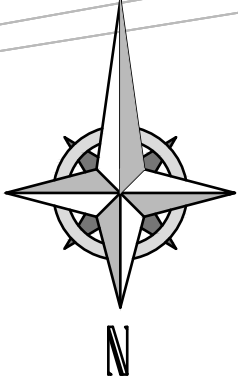
Site 4:	A61 Sheffield Road/Worsbrough Road	A:	A61 Barnsley
Day:	Tuesday	B:	Worsbrough Road
Date:	22 September 2020	C:	A61 Sheffield
Weather:	Fine & Sunny until 16:40 then Fine & Cloudy	D:	Un-named Access Road

Time	From A								From B							
	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total
14:30	101	26	2	5	0	1	1	136	18	1	1	0	0	0	1	21
14:45	116	15	5	3	0	0	2	141	22	3	2	0	0	0	0	27
15:00	109	11	4	3	0	0	1	128	18	1	0	0	0	0	1	20
15:15	121	21	2	1	1	0	3	149	21	4	0	0	0	0	2	27
15:30	122	25	2	1	1	0	1	152	18	2	1	1	0	0	0	22
15:45	97	24	4	2	0	0	1	128	12	3	0	0	0	0	0	15
16:00	127	28	1	1	1	0	2	160	18	5	0	0	2	0	1	26
16:15	109	20	5	3	0	2	1	140	20	4	0	0	1	0	3	28
16:30	109	17	3	1	1	1	2	134	19	0	0	0	0	0	0	19
16:45	114	23	0	1	0	2	1	141	14	0	0	0	0	1	2	17
17:00	134	23	4	2	1	0	2	166	15	5	0	0	0	2	0	22
17:15	149	18	1	0	0	2	1	171	16	1	0	0	0	0	2	19
17:30	103	12	0	0	1	2	2	120	21	2	0	0	1	0	1	25
17:45	106	16	0	2	0	1	1	126	7	4	0	0	0	0	0	11
18:00	101	8	1	1	0	0	1	112	26	0	0	0	0	0	0	26
18:15	86	12	1	0	0	0	1	100	18	5	0	0	0	0	2	25
Total	1804	299	35	26	6	11	23	2204	283	40	4	1	4	3	15	350

Time	From C								From D							
	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total
14:30	110	23	4	4	0	2	1	144	0	1	0	0	0	0	0	1
14:45	128	26	6	3	2	4	2	171	0	0	0	0	0	0	0	0
15:00	114	31	4	1	0	0	1	151	0	0	0	0	0	0	0	0
15:15	147	34	6	1	0	4	4	196	0	0	0	0	0	0	0	0
15:30	159	28	5	4	2	1	3	202	1	0	0	0	0	0	0	1
15:45	144	44	2	3	1	2	3	199	0	0	0	0	0	0	0	0
16:00	156	41	7	3	3	0	1	211	1	0	0	0	0	0	0	1
16:15	182	51	2	3	4	2	3	247	2	0	0	0	0	0	0	2
16:30	154	54	2	0	1	1	2	214	0	0	0	0	0	0	0	0
16:45	162	41	4	1	1	1	3	213	1	0	0	0	0	0	0	1
17:00	199	34	4	2	1	2	1	243	1	0	0	0	0	0	0	1
17:15	190	33	2	1	0	2	3	231	2	0	0	0	0	0	0	2
17:30	180	37	1	1	0	1	1	221	2	0	0	0	0	0	0	2
17:45	192	30	0	1	0	2	3	228	0	0	0	0	0	0	0	0
18:00	141	22	2	0	0	1	1	167	1	0	0	0	0	0	0	1
18:15	144	23	0	2	0	1	3	173	1	0	0	0	0	0	0	1
Total	2502	552	51	30	15	26	35	3211	12	1	0	0	0	0	0	13

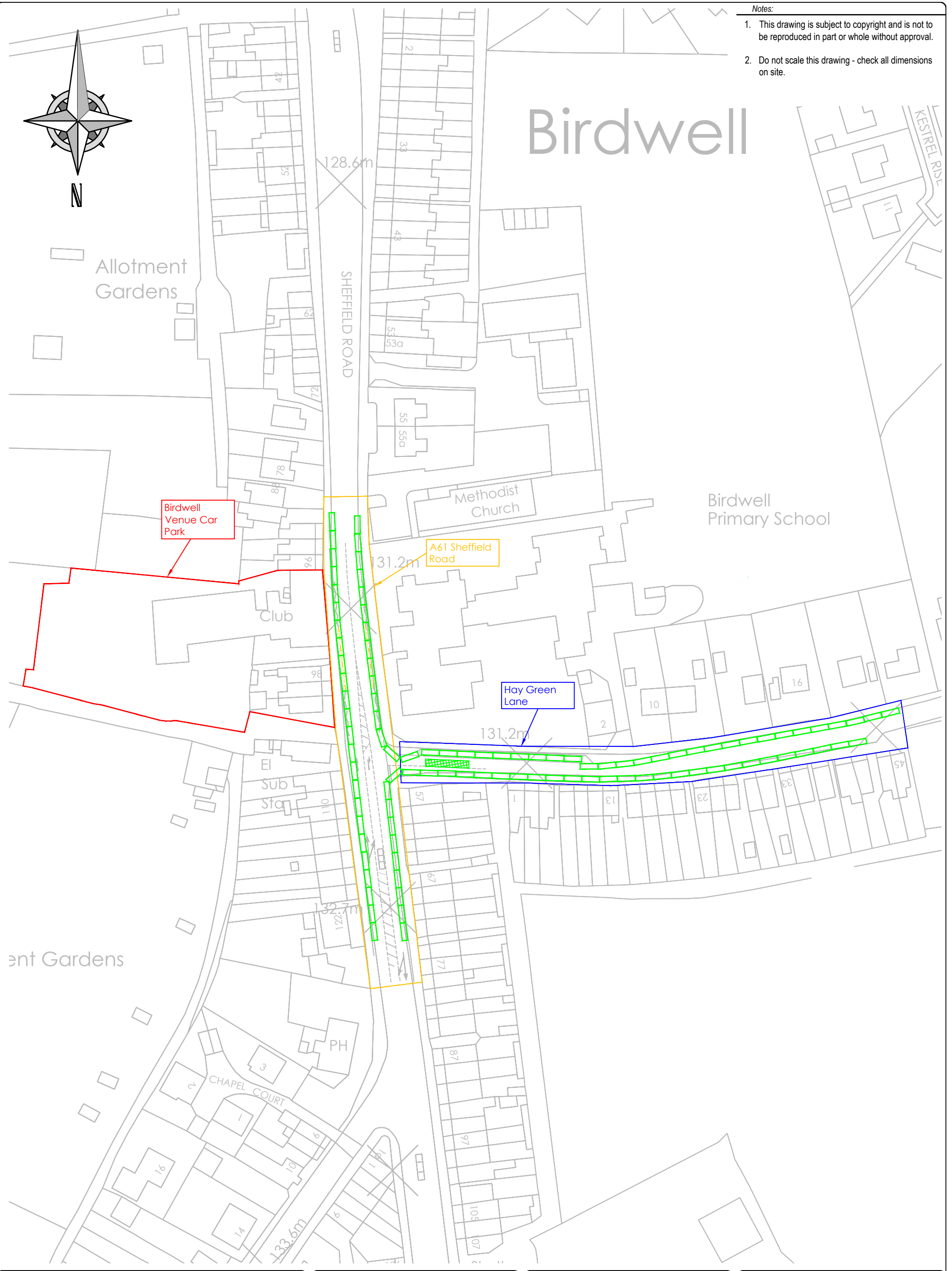
Time	To A								To B							
	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total
14:30	94	21	2	4	0	2	1	124	17	4	2	0	0	0	0	23
14:45	105	25	6	3	2	3	2	146	23	4	1	0	0	1	0	29
15:00	105	26	4	1	0	0	1	137	13	5	1	0	0	0	0	19
15:15	120	33	6	1	0	3	2	165	34	1	0	0	0	1	2	38
15:30	126	25	5	4	2	1	2	165	36	3	0	0	0	0	1	40
15:45	126	41	2	3	1	2	3	178	18	3	0	0	0	0	0	21
16:00	134	37	7	3	3	0	0	184	25	5	0	0	0	0	1	31
16:15	153	47	2	3	4	2	2	213	33	4	0	0	0	0	1	38
16:30	137	49	2	0	0	1	1	190	22	5	0	0	1	0	1	29
16:45	145	39	3	1	1	1	2	192	22	2	1	0	0	0	1	26
17:00	177	31	4	2	0	2	1	217	23	3	0	0	1	0	0	27
17:15	160	30	2	1	0	2	2	197	34	3	0	0	0	0	1	38
17:30	154	32	1	1	0	1	0	189	28	5	0	0	0	0	1	34
17:45	169	30	0	1	0	1	3	204	23	3	0	0	0	1	0	27
18:00	122	20	2	0	0	1	1	146	24	2	0	0	0	0	0	26
18:15	130	19	0	2	0	1	2	154	19	4	0	0	0	0	1	24
Total	2157	505	48	30	13	23	25	2801	394	56	5	0	2	3	10	470

Time	To C								To D							
	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total	Car	LGV	OGVI	OGV2	P/C	M/C	PSV	Total
14:30	117	26	3	5	0	1	2	154	1	0	0	0	0	0	0	1
14:45	138	15	6	3	0	0	2	164	0	0	0	0	0	0	0	0
15:00	123	12	3	3	0	0	2	143	0	0	0	0	0	0	0	0
15:15	135	25	2	1	1	0	5	169	0	0	0	0	0	0	0	0
15:30	138	27	3	2	1	0	1	172	0	0	0	0	0	0	0	0
15:45	109	27	4	2	0	0	1	143	0	0	0	0	0	0	0	0
16:00	142	32	1	1	3	0	3	182	1	0	0	0	0	0	0	1
16:15	127	24	5	3	1	2	4	166	0	0	0	0	0	0	0	0
16:30	123	17	3	1	1	1	2	148	0	0	0	0	0	0	0	0
16:45	122	23	0	1	0	3	3	152	2	0	0	0	0	0	0	2
17:00	146	28	4	2	1	2	2	185	3	0	0	0	0	0	0	3
17:15	163	19	1	0	0	2	3	188	0	0	0	0	0	0	0	0
17:30	124	14	0	0	2	2	3	145	0	0	0	0	0	0	0	0
17:45	112	17	0	2	0	1	1	133	1	0	0	0	0	0	0	1
18:00	123	8	1	1	0	0	1	134	0	0	0	0	0	0	0	0
18:15	100	17	1	0	0	0	3	121	0	0	0	0	0	0	0	0
Total	2042	331	37	27	10	14	38	2499	8	0	0	0	0	0	0	8



- Notes:
1. This drawing is subject to copyright and is not to be reproduced in part or whole without approval.
 2. Do not scale this drawing - check all dimensions on site.

Birdwell



Rev:	09.09.20	INITIAL ISSUE	KG
Date:		Status/Amendments:	By:

mosodi
mobility solutions through design and innovation

Manchester Leeds
0161 413 5168 0113 323 0854

Client:
HARWORTH

Project:
**HAY GREEN LANE,
BIRDWELL**

Drawing Title: PARKING SURVEY		
Drawn By: KG	Checked By: -	Approved By: -
Scale: 1:1000	Paper Size: A3	Date Created: 09.09.20
Drawing Number: 18079.IN.10		Drawing Revision: -

BIRDWELL VENUE CAR PARK SURVEY - WEDNESDAY 16 SEPTEMBER 2020 (TOTAL)

Arm A: A61 North Arm C A61 South
 Arm B: Hay Green Lane Arm D Birdwell Venue

Time	A - D	B - D	C - D	Total	D - A	D - B	D - C	Total	Acc.
07:30	0	0	0	0	0	0	0	0	1
07:35	0	0	0	0	0	0	0	0	1
07:40	0	0	0	0	0	0	0	0	1
07:45	0	0	0	0	0	0	0	0	1
07:50	0	0	0	0	0	0	0	0	1
07:55	0	0	0	0	0	0	0	0	1
08:00	0	0	0	0	0	0	0	0	1
08:05	0	0	0	0	0	0	0	0	1
08:10	0	0	1	1	0	0	0	0	1
08:15	0	0	0	0	0	0	0	0	2
08:20	0	0	0	0	0	0	0	0	2
08:25	0	0	0	0	0	0	0	0	2
08:30	0	0	0	0	0	0	0	0	2
08:35	3	1	7	11	0	0	0	0	2
08:40	1	0	4	5	0	0	0	0	13
08:45	0	0	4	4	2	0	1	3	18
08:50	1	1	2	4	3	0	4	7	19
08:55	1	0	1	2	3	0	2	5	16
09:00	0	0	1	1	0	0	2	2	13
09:05	0	0	0	0	1	0	1	2	12
09:10	0	0	0	0	1	0	2	3	10
09:15	1	0	2	3	0	0	1	1	7
09:20	1	0	3	4	1	0	0	1	9
09:25	0	0	2	2	0	0	0	0	12
Total	8	2	27	37	11	0	13	24	14

BIRDWELL VENUE CAR PARK SURVEY - WEDNESDAY 16 SEPTEMBER 2020 (TOTAL)

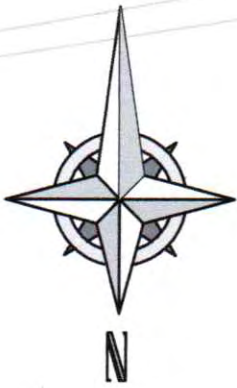
Time	A - D	B - D	C - D	Total	D - A	D - B	D - C	Total	Acc.
14:30	0	0	0	0	0	0	0	0	4
14:35	0	0	0	0	0	0	0	0	4
14:40	0	0	0	0	0	0	0	0	4
14:45	0	0	0	0	0	0	0	0	4
14:50	1	0	1	2	1	0	0	1	4
14:55	1	0	0	1	0	0	0	0	5
15:00	1	0	1	2	0	0	0	0	6
15:05	2	0	4	6	0	0	1	1	8
15:10	2	0	2	4	0	0	0	0	13
15:15	1	0	3	4	0	0	1	1	17
15:20	0	0	0	0	0	0	1	1	20
15:25	0	0	1	1	2	0	4	6	19
15:30	0	0	0	0	2	0	2	4	14
15:35	0	0	0	0	1	0	1	2	10
15:40	0	0	1	1	3	0	2	5	8
15:45	0	0	1	1	1	0	0	1	4
15:50	1	1	0	2	0	0	0	0	4
15:55	0	0	0	0	0	0	2	2	6
16:00	0	0	0	0	0	0	0	0	4
16:05	0	0	0	0	0	0	0	0	4
16:10	0	0	0	0	0	0	0	0	4
16:15	0	0	0	0	0	0	0	0	4
16:20	0	0	0	0	0	0	0	0	4
16:25	0	0	1	1	1	0	0	1	4
16:30	1	0	2	3	1	0	0	1	4
16:35	0	0	0	0	0	0	0	0	6
16:40	0	0	0	0	0	0	0	0	6
16:45	0	0	0	0	0	0	0	0	6
16:50	0	0	0	0	0	0	0	0	6
16:55	0	0	0	0	0	0	0	0	6
Total	10	1	17	28	12	0	14	26	6

BIRDWELL VENUE CAR PARK SURVEY - WEDNESDAY 16 SEPTEMBER 2020 (NORTH)

Time	A - D	B - D	C - D	Total	D - A	D - B	D - C	Total
07:30	0	0	0	0	0	0	0	0
07:35	0	0	0	0	0	0	0	0
07:40	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0
07:50	0	0	0	0	0	0	0	0
07:55	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0
08:05	0	0	0	0	0	0	0	0
08:10	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	0	0	0
08:20	0	0	0	0	0	0	0	0
08:25	0	0	0	0	0	0	0	0
08:30	0	0	0	0	0	0	0	0
08:35	0	0	0	0	0	0	0	0
08:40	0	0	0	0	0	0	0	0
08:45	0	0	0	0	2	0	1	3
08:50	0	0	0	0	2	0	2	4
08:55	0	0	0	0	2	0	2	4
09:00	0	0	0	0	0	0	1	1
09:05	0	0	0	0	1	0	0	1
09:10	0	0	0	0	0	0	0	0
09:15	1	0	1	2	0	0	1	1
09:20	0	0	0	0	1	0	0	1
09:25	0	0	0	0	0	0	0	0
Total	1	0	1	2	8	0	7	15

BIRDWELL VENUE CAR PARK SURVEY - WEDNESDAY 16 SEPTEMBER 2020 (NORTH)

Time	A - D	B - D	C - D	Total	D - A	D - B	D - C	Total
14:30	0	0	0	0	0	0	0	0
14:35	0	0	0	0	0	0	0	0
14:40	0	0	0	0	0	0	0	0
14:45	0	0	0	0	0	0	0	0
14:50	0	0	0	0	1	0	0	1
14:55	0	0	0	0	0	0	0	0
15:00	0	0	0	0	0	0	0	0
15:05	0	0	0	0	0	0	1	1
15:10	0	0	0	0	0	0	0	0
15:15	0	0	0	0	0	0	1	1
15:20	0	0	0	0	0	0	1	1
15:25	0	0	0	0	2	0	1	3
15:30	0	0	0	0	1	0	1	2
15:35	0	0	0	0	1	0	1	2
15:40	0	0	0	0	3	0	2	5
15:45	0	0	0	0	0	0	0	0
15:50	0	0	0	0	0	0	0	0
15:55	0	0	0	0	0	0	0	0
16:00	0	0	0	0	0	0	0	0
16:05	0	0	0	0	0	0	0	0
16:10	0	0	0	0	0	0	0	0
16:15	0	0	0	0	0	0	0	0
16:20	0	0	0	0	0	0	0	0
16:25	0	0	0	0	1	0	0	1
16:30	0	0	0	0	1	0	0	1
16:35	0	0	0	0	0	0	0	0
16:40	0	0	0	0	0	0	0	0
16:45	0	0	0	0	0	0	0	0
16:50	0	0	0	0	0	0	0	0
16:55	0	0	0	0	0	0	0	0
Total	0	0	0	0	10	0	8	18



Birdw

Allotment Gardens

128.6m

SHEFFIELD ROAD

Birdwell Venue Car Park

Methodist Church

A61 Sheffield Road

Hay Green Lane

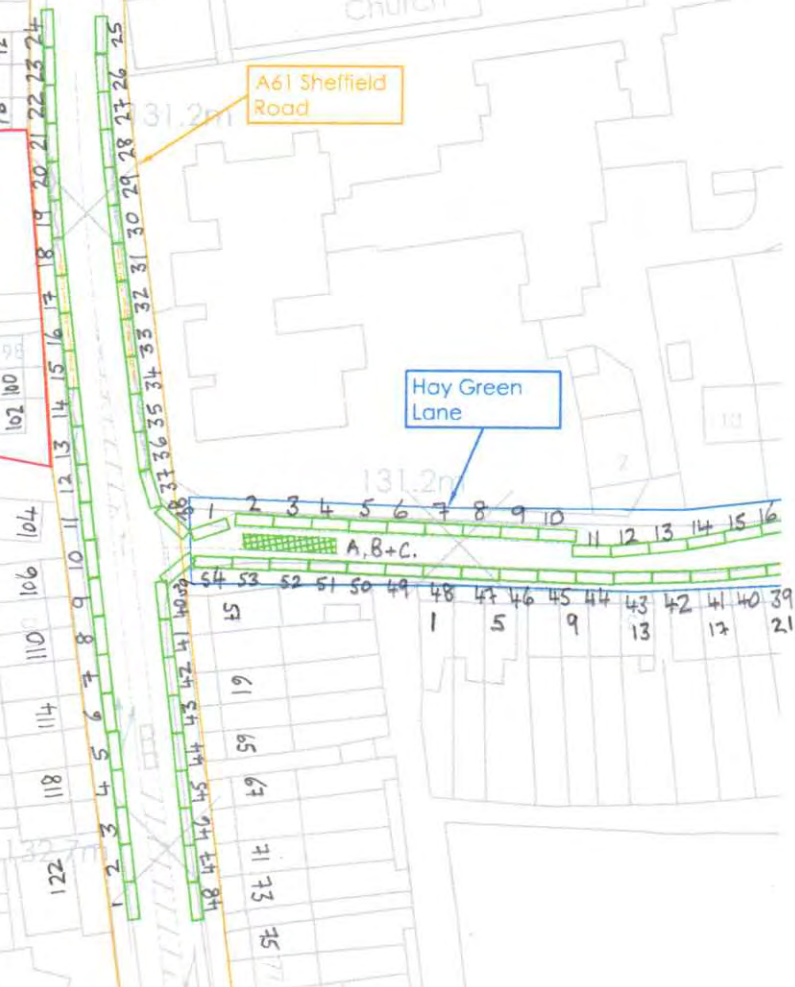
Club

SUB
sta

131.2m

A, B + C.

ent Gardens



A61 SHEFFIELD ROAD, BIRDWELL - SCHOOL PATROL CROSSING (TOTAL)

WEDNESDAY 16 SEPTEMBER 2020

AM Peak

Time	Pedestrians W/B			Pedestrians E/B			Pedestrians Total			Queues	
	Adults	Children	Total	Adults	Children	Total	Adults	Children	Total	N/B	S/B
08:39	0	0	0	1	1	2	1	1	2	-	3
08:40	0	0	0	8	13	21	8	13	21	15	14
08:43	0	0	0	3	4	7	3	4	7	8	11
08:44	0	0	0	2	3	5	2	3	5	4	8
08:45	0	0	0	3	6	9	3	6	9	3	13
08:46	2	0	2	2	2	4	4	2	6	10	14
08:47	1	0	1	1	2	3	2	2	4	9	-
08:47	1	0	1	2	3	5	3	3	6	15	7
08:48	3	1	4	2	3	5	5	4	9	11	20
08:49	0	0	0	0	1	1	0	1	1	8	19
08:50	0	0	0	1	1	2	1	1	2	9	17
08:50	1	0	1	0	0	0	1	0	1	10	16
08:50	4	0	4	0	0	0	4	0	4	1	15
08:55	1	0	1	0	0	0	1	0	1	7	8
08:56	1	0	1	1	2	3	2	2	4	9	12

PM Peak

Time	Pedestrians W/B			Pedestrians E/B			Pedestrians Total			Queues	
	Adults	Children	Total	Adults	Children	Total	Adults	Children	Total	N/B	S/B
15:09	0	0	0	1	0	1	1	0	1	7	1
15:12	0	0	0	1	0	1	1	0	1	9	4
15:12	0	0	0	2	1	3	2	1	3	9	6
15:13	0	0	0	3	0	3	3	0	3	12	9
15:14	0	0	0	2	0	2	2	0	2	6	6
15:15	0	0	0	1	0	1	1	0	1	10	7
15:15	0	0	0	1	0	1	1	0	1	9	4
15:16	0	0	0	1	0	1	1	0	1	4	4
15:17	1	1	2	1	0	1	2	1	3	3	-
15:17	0	0	0	1	1	2	1	1	2	6	2
15:18	1	1	2	1	0	1	2	1	3	10	11
15:18	0	0	0	1	0	1	1	0	1	10	8
15:19	1	1	2	0	0	0	1	1	2	5	9
15:19	1	1	2	4	0	4	5	1	6	9	12
15:20	1	2	3	2	0	2	3	2	5	7	9
15:20	1	1	2	1	0	1	2	1	3	3	9
15:21	1	1	2	0	0	0	1	1	2	3	1
15:22	2	3	5	0	0	0	2	3	5	3	4
15:22	2	5	7	0	0	0	2	5	7	10	4
15:23	2	3	5	0	0	0	2	3	5	8	6
15:24	5	6	11	0	0	0	5	6	11	5	12
15:27	2	6	8	0	0	0	2	6	8	9	5
15:30	3	4	7	0	0	0	3	4	7	1	6
15:31	1	2	3	0	0	0	1	2	3	11	3
15:33	3	4	7	0	0	0	3	4	7	7	3
15:34	1	0	1	0	0	0	1	0	1	-	5
15:35	1	1	2	0	0	0	1	1	2	7	2
15:37	1	2	3	0	0	0	1	2	3	10	12
15:37	1	2	3	0	0	0	1	2	3	11	15
15:38	1	2	3	0	0	0	1	2	3	6	5

**A61 SHEFFIELD ROAD, BIRDWELL - SCHOOL PATROL CROSSING - THOSE ACCESSING
VEHICLES PARKED IN BIRDWELL VENUE CAR PARK**

WEDNESDAY 16 SEPTEMBER 2020

AM Peak

Time	Pedestrians W/B			Pedestrians E/B			Pedestrians Total		
	Adults	Children	Total	Adults	Children	Total	Adults	Children	Total
08:39	0	0	0	1	1	2	1	1	2
08:40	0	0	0	6	9	15	6	9	15
08:43	0	0	0	2	3	5	2	3	5
08:44	0	0	0	2	3	5	2	3	5
08:45	0	0	0	2	5	7	2	5	7
08:46	2	0	2	2	2	4	4	2	6
08:47	1	0	1	1	2	3	2	2	4
08:47	1	0	1	2	3	5	3	3	6
08:48	2	1	3	2	3	5	4	4	8
08:49	0	0	0	0	1	1	0	1	1
08:50	0	0	0	1	1	2	1	1	2
08:50	1	0	1	0	0	0	1	0	1
08:50	3	0	3	0	0	0	3	0	3
08:55	1	0	1	0	0	0	1	0	1
08:56	1	0	1	1	2	3	2	2	4

PM Peak

Time	Pedestrians W/B			Pedestrians E/B			Pedestrians Total		
	Adults	Children	Total	Adults	Children	Total	Adults	Children	Total
15:09	0	0	0	1	0	1	1	0	1
15:12	0	0	0	1	0	1	1	0	1
15:12	0	0	0	1	1	2	1	1	2
15:13	0	0	0	2	0	2	2	0	2
15:14	0	0	0	2	0	2	2	0	2
15:15	0	0	0	1	0	1	1	0	1
15:15	0	0	0	1	0	1	1	0	1
15:16	0	0	0	1	0	1	1	0	1
15:17	1	1	2	1	0	1	2	1	3
15:17	0	0	0	0	0	0	0	0	0
15:18	1	1	2	1	0	1	2	1	3
15:18	0	0	0	1	0	1	1	0	1
15:19	1	1	2	0	0	0	1	1	2
15:19	1	1	2	2	0	2	3	1	4
15:20	1	2	3	0	0	0	1	2	3
15:20	1	1	2	1	0	1	2	1	3
15:21	1	1	2	0	0	0	1	1	2
15:22	1	1	2	0	0	0	1	1	2
15:22	2	4	6	0	0	0	2	4	6
15:23	2	3	5	0	0	0	2	3	5
15:24	3	4	7	0	0	0	3	4	7
15:27	0	0	0	0	0	0	0	0	0
15:30	0	0	0	0	0	0	0	0	0
15:31	1	2	3	0	0	0	1	2	3
15:33	1	1	2	0	0	0	1	1	2
15:34	1	0	1	0	0	0	1	0	1
15:35	0	0	0	0	0	0	0	0	0
15:37	0	0	0	0	0	0	0	0	0
15:37	1	2	3	0	0	0	1	2	3
15:38	1	2	3	0	0	0	1	2	3

A61 SHEFFIELD ROAD, BIRDWELL - ZEBRA CROSSING SURVEY
WEDNESDAY 16 SEPTEMBER 2020

AM Peak

Time	Pedestrians W/B			Pedestrians E/B			Pedestrians Total			Queues	
	Adults	Children	Total	Adults	Children	Total	Adults	Children	Total	N/B	S/B
07:38	0	0	0	0	2	2	0	2	2	-	-
07:46	0	0	0	1	0	1	1	0	1	3	2
08:13	1	0	1	0	0	0	1	0	1	-	1
08:14	0	0	0	1	0	1	1	0	1	2	3
08:18	1	0	1	0	0	0	1	0	1	-	-
08:19	1	0	1	0	0	0	1	0	1	2	2
08:31	0	0	0	2	1	3	2	1	3	3	2
08:41	0	0	0	1	2	3	1	2	3	2	2
08:51	0	0	0	1	1	2	1	1	2	4	4
08:52	1	0	1	0	0	0	1	0	1	5	6
08:56	0	0	0	1	0	1	1	0	1	-	-
08:58	0	0	0	1	0	1	1	0	1	3	2
08:59	0	0	0	1	1	2	1	1	2	3	-
09:01	1	0	1	0	0	0	1	0	1	1	2
09:02	1	0	1	0	0	0	1	0	1	-	-
09:03	0	0	0	1	0	1	1	0	1	-	1
09:05	0	0	0	1	0	1	1	0	1	1	6
09:06	1	0	1	0	0	0	1	0	1	1	5
09:12	1	0	1	0	0	0	1	0	1	-	1
09:15	0	0	0	1	1	2	1	1	2	2	-
09:24	2	0	2	0	0	0	2	0	2	1	2
09:25	0	0	0	1	0	1	1	0	1	-	1
09:26	1	0	1	0	0	0	1	0	1	2	4

PM Peak

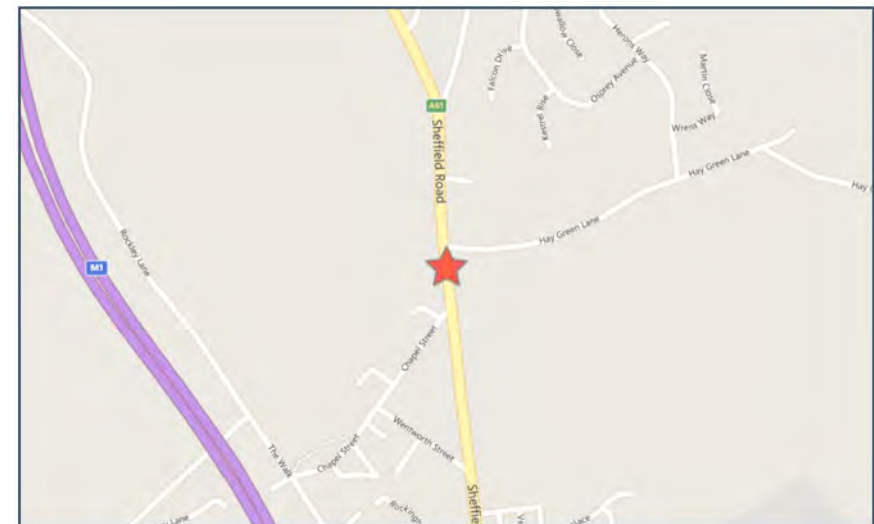
14:35	1	0	1	0	0	0	1	0	1	3	-
14:36	0	0	0	1	1	2	1	1	2	4	1
14:37	0	0	0	1	0	1	1	0	1	2	-
14:38	0	0	0	1	0	1	1	0	1	4	-
14:39	1	0	1	0	0	0	1	0	1	-	-
14:44	1	0	1	0	0	0	1	0	1	-	-
14:45	0	0	0	1	0	1	1	0	1	4	1
14:48	1	0	1	0	0	0	1	0	1	2	2
14:48	0	0	0	1	0	1	1	0	1	3	1
14:50	0	0	0	1	1	2	1	1	2	4	3
14:51	1	0	1	0	0	0	1	0	1	5	-
14:54	0	0	0	1	1	2	1	1	2	1	-
14:55	1	0	1	0	0	0	1	0	1	-	-
15:04	0	0	0	1	0	1	1	0	1	3	3
15:09	0	0	0	1	0	1	1	0	1	5	2
15:10	0	0	0	1	0	1	1	0	1	3	1
15:11	0	0	0	1	0	1	1	0	1	4	-
15:15	0	0	0	1	0	1	1	0	1	4	2
15:19	0	0	0	1	0	1	1	0	1	6	-
15:20	0	1	1	0	0	0	0	1	1	-	-
15:22	2	1	3	0	0	0	2	1	3	3	4
15:24	0	0	0	1	0	1	1	0	1	-	2
15:26	0	0	0	1	1	2	1	1	2	4	3
15:27	1	1	2	0	0	0	1	1	2	2	2
15:28	1	0	1	0	0	0	1	0	1	1	-
15:30	2	0	2	0	0	0	2	0	2	3	-
15:31	0	0	0	1	0	1	1	0	1	2	2
15:33	1	2	3	0	0	0	1	2	3	8	4
15:36	0	0	0	1	0	1	1	0	1	7	2
15:37	0	2	2	0	0	0	0	2	2	7	1
15:40	1	2	3	0	0	0	1	2	3	9	5
15:41	1	7	8	0	0	0	1	7	8	12	6
15:44	0	0	0	1	0	1	1	0	1	-	6
15:48	1	0	1	0	0	0	1	0	1	2	1
15:49	0	0	0	1	0	1	1	0	1	2	1
15:50	1	0	1	0	0	0	1	0	1	2	1
15:51	1	0	1	1	0	1	2	0	2	12	-
15:53	1	1	2	0	0	0	1	1	2	10	1
15:55	0	0	0	1	0	1	1	0	1	8	2
15:56	2	0	2	0	0	0	2	0	2	8	-
16:03	0	0	0	1	1	2	1	1	2	10	-
16:04	1	1	2	0	0	0	1	1	2	11	-
16:05	0	0	0	1	0	1	1	0	1	2	2
16:07	0	0	0	1	0	1	1	0	1	2	3
16:08	0	0	0	0	1	1	0	1	1	1	-
16:09	0	0	0	1	0	1	1	0	1	1	3
16:10	1	0	1	0	0	0	1	0	1	-	-
16:11	0	0	0	1	1	2	1	1	2	5	1
16:12	0	0	0	1	0	1	1	0	1	3	-
16:17	1	0	1	0	0	0	1	0	1	3	-
16:26	0	0	0	2	0	2	2	0	2	7	1
16:32	0	0	0	1	0	1	1	0	1	9	2
16:34	0	0	0	1	2	3	1	2	3	-	2
16:39	0	0	0	0	2	2	0	2	2	8	2
16:40	0	0	0	1	0	1	1	0	1	2	1
16:43	1	0	1	0	0	0	1	0	1	4	5
16:47	1	0	1	0	0	0	1	0	1	2	2
16:55	2	0	2	0	0	0	2	0	2	2	-

Appendix C Accident Data



Crash Date: Saturday, July 07, 2018 **Time of Crash:** 2:28:00 PM **Crash Reference:** 2018140311489

Highest Injury Severity:	Serious	Road Number:	A61	Number of Casualties:	1
Highway Authority:	Barnsley			Number of Vehicles:	3
Local Authority:	Barnsley Metropolitan Borough			OS Grid Reference:	434555 401363
Weather Description:	Fine without high winds				
Road Surface Description:	Dry				
Speed Limit:	30				
Light Conditions:	Daylight: regardless of presence of streetlights				
Carriageway Hazards:	None				
Junction Detail:	T or staggered junction				
Junction Pedestrian Crossing:	No physical crossing facility within 50 metres				
Road Type:	Single carriageway				
Junction Control:	Give way or uncontrolled				

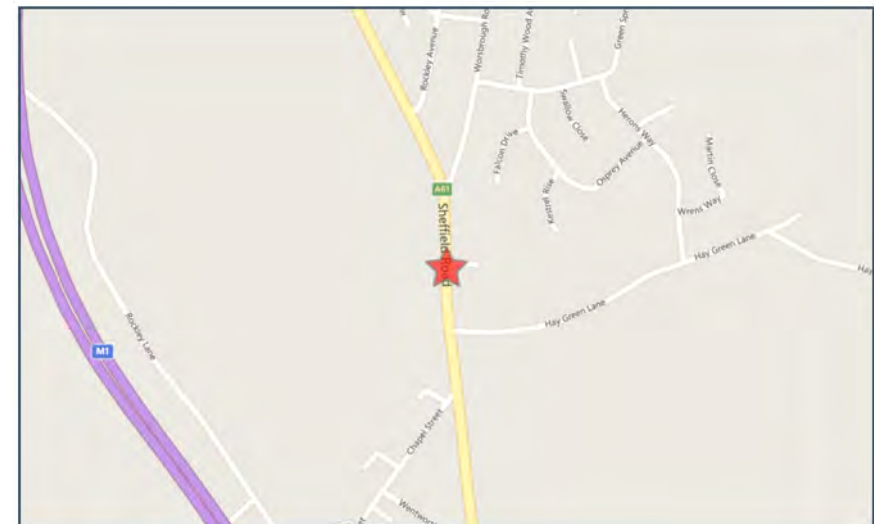


For more information about the data please visit: www.crashmap.co.uk/home/Faq
To subscribe to unlimited reports using CrashMap Pro visit www.crashmap.co.uk/Home/Premium_Services



Crash Date: Thursday, November 26, 2015 **Time of Crash:** 7:30:00 PM **Crash Reference:** 201514B108615

Highest Injury Severity:	Slight	Road Number:	A61	Number of Casualties:	1
Highway Authority:	Barnsley			Number of Vehicles:	2
Local Authority:	Barnsley			OS Grid Reference:	434546 401488
Weather Description:	Fine without high winds				
Road Surface Description:	Dry				
Speed Limit:	30				
Light Conditions:	Darkness: street lights present and lit				
Carriageway Hazards:	None				
Junction Detail:	Not at or within 20 metres of junction				
Junction Pedestrian Crossing:	No physical crossing facility within 50 metres				
Road Type:	Single carriageway				
Junction Control:	Not Applicable				

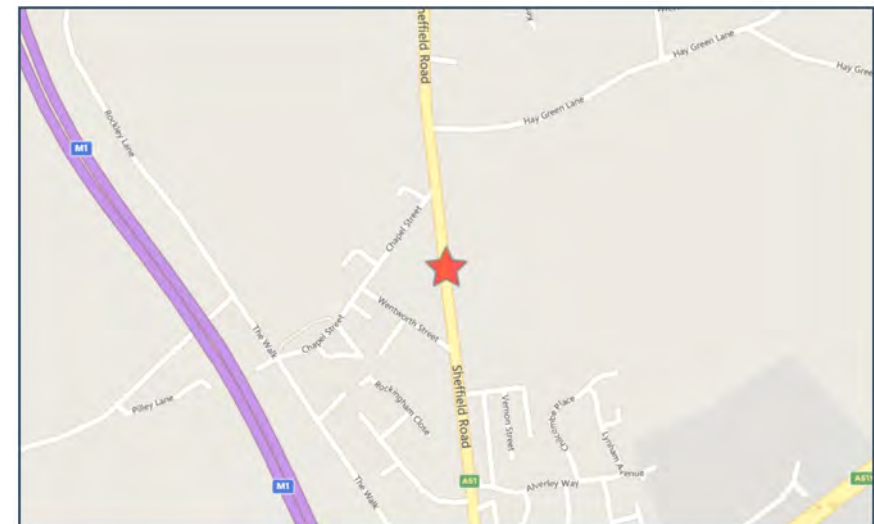


For more information about the data please visit: www.crashmap.co.uk/home/Faq
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Crash Date: Monday, November 16, 2015 **Time of Crash:** 12:10:00 PM **Crash Reference:** 201514B105615

Highest Injury Severity:	Slight	Road Number:	A61	Number of Casualties:	1
Highway Authority:	Barnsley	Number of Vehicles:	2	OS Grid Reference:	434580 401174
Local Authority:	Barnsley				
Weather Description:	Fine without high winds				
Road Surface Description:	Wet or Damp				
Speed Limit:	30				
Light Conditions:	Daylight: regardless of presence of streetlights				
Carriageway Hazards:	Dislodged vehicle load in carriageway				
Junction Detail:	Not at or within 20 metres of junction				
Junction Pedestrian Crossing:	Pelican, puffin, toucan or similar non-junction pedestrian light crossing				
Road Type:	Single carriageway				
Junction Control:	Not Applicable				



For more information about the data please visit: www.crashmap.co.uk/home/Faq
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AccsMap - Accident Analysis System

Accidents between dates 01/01/2015 and 03/11/2020 (70) months

Selection:

Notes:

Selected using Pre-defined Query : District - (Barnsley) collis

18311489 07/07/2018 Saturday Time: 1428 Vehicles 3 Casualties 1 Serious
 Easting: 434,555 Northing: 401,363
 Fine without high winds Road Surface: Dry Daylight
 Road Type: Single carriageway Speed Limit: 30

Location: SHEFFIELD ROAD (A61) BARNSELY AT OR NR JN WITH HAY GREEN LANE
 Description: V01 WAITING TO TURN RIGHT, SEEN OFF ROAD BIKE V02 IN MIRROR PULLING WHEELIES COMING DOWN THE MIDDLE OF THE ROAD, V02 LANDS ON REAR OF V01 CAUSING DAMAGE. RIDER FALLS FROM BIKE CAUSING INJURY TO RIDER C01 V02 THEN SKIDS INTO V03 CAUSING DAMAGE.

Vehicle Reference: 1 Car Waiting to turn right
 First point of impact: Back
 Vehicle direction: S to E Journey: Other
 Age of Driver : 33 Breath test: Driver not contacted

Contributory Factors : 602 405

Vehicle Reference: 2 Motorcycle over 500cc Going ahead
 First point of impact: Front
 Vehicle direction: S to N Journey: Not known
 Age of Driver : 27 Breath test: Not requested

Contributory Factors : 602 405

Vehicle Reference: 3 Van or Goods <= 3.5 tonnes Parked
 First point of impact: Back
 Vehicle direction: Parked to Parked Journey: Not known
 Age of Driver : 47 Breath test: Not requested

Contributory Factors : 602 405

Accidents between dates 01/01/2015 and 03/11/2020 (70) months

Selection: Notes:

Selected using Pre-defined Query : District - (Barnsley) collis

20959655 01/06/2020 Monday Time: 2154 Vehicles 2 Casualties 1 Serious
 Easting: 434,555 Northing: 401,384
 Fine without high winds Road Surface: Dry Darkness: street lights present and lit
 Road Type: Single carriageway Speed Limit: 30

Location: SHEFFIELD ROAD (A61) AT JUNCTION WITH HAY GREEN LANE
 Description: TWO VEHICLE INJURY RTC, WITH V1 TURNING RIGHT DIRECTLY INTO PATH OF V2. INJURY CAUSED TO RIDER FROM V2.

Vehicle Reference: 1 Car Turning right
 First point of impact: Front
 Vehicle direction: S to E Journey: Journey as part of work
 Age of Driver : 52 Breath test: Negative
 Contributory Factors : 405

Vehicle Reference: 2 Motorcycle 50cc and under Going ahead
 First point of impact: Front
 Vehicle direction: N to S Journey: Not known
 Age of Driver : 62 Breath test: Negative
 Contributory Factors : 405

Accidents involving:

	Fatal	Serious	Slight	Total
Motor vehicles only excluding 2-wheels	0	0	0	0
2-wheeled motor vehicles	0	2	0	2
Pedal cycles	0	0	0	0
Horses & other	0	0	0	0
Total	0	2	0	2

Casualties:

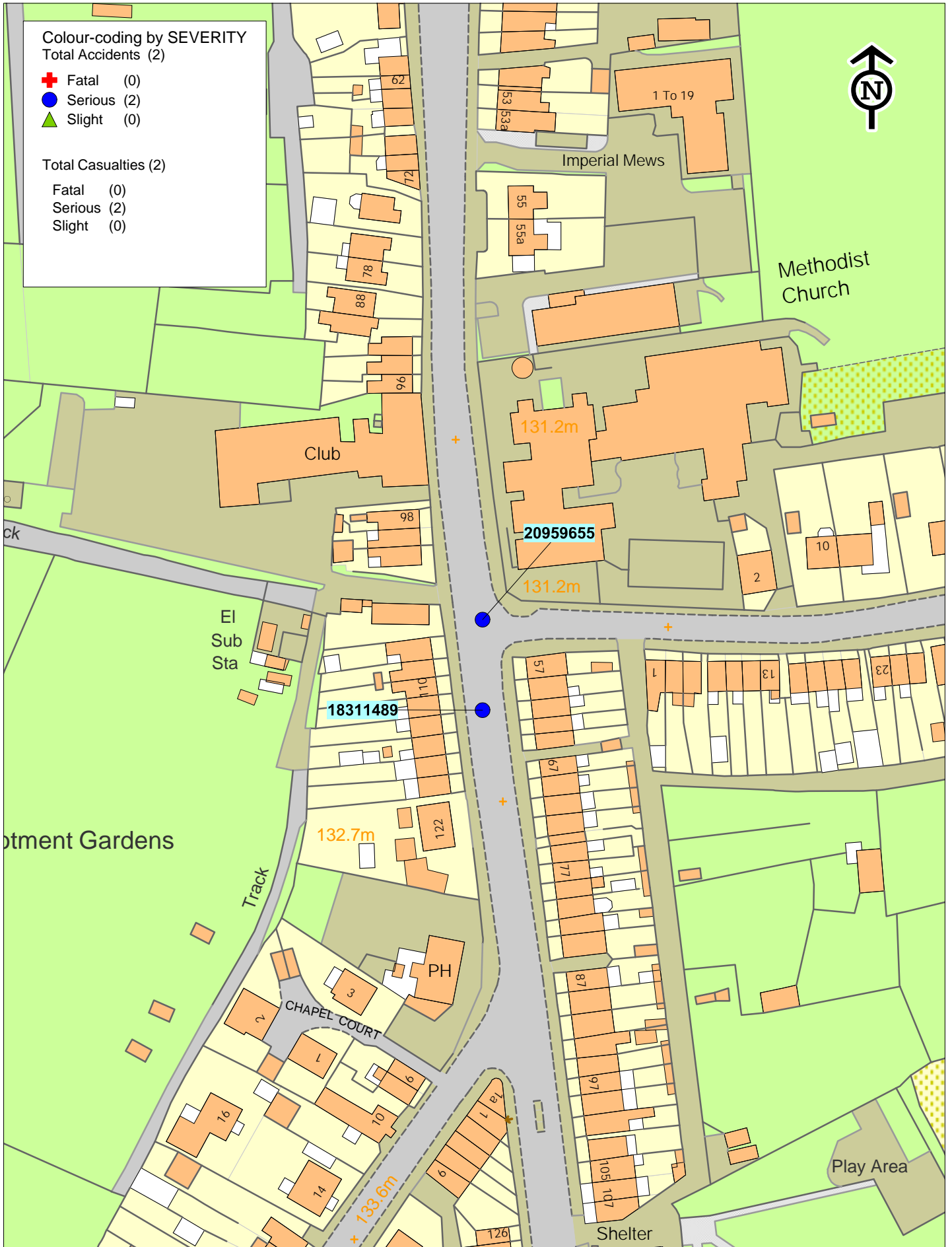
	Fatal	Serious	Slight	Total
Vehicle driver	0	0	0	0
Passenger	0	0	0	0
Motorcycle rider	0	0	0	0
Cyclist	0	0	0	0
Pedestrian	0	0	0	0
Other	0	0	0	0
Total	0	0	0	0

Colour-coding by SEVERITY
Total Accidents (2)

- + Fatal (0)
- Serious (2)
- ▲ Slight (0)

Total Casualties (2)

- Fatal (0)
- Serious (2)
- Slight (0)



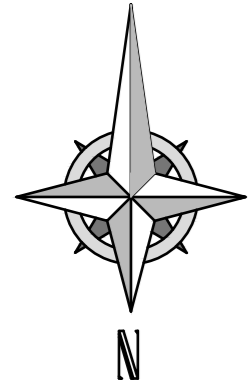
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ROADS SAFER**

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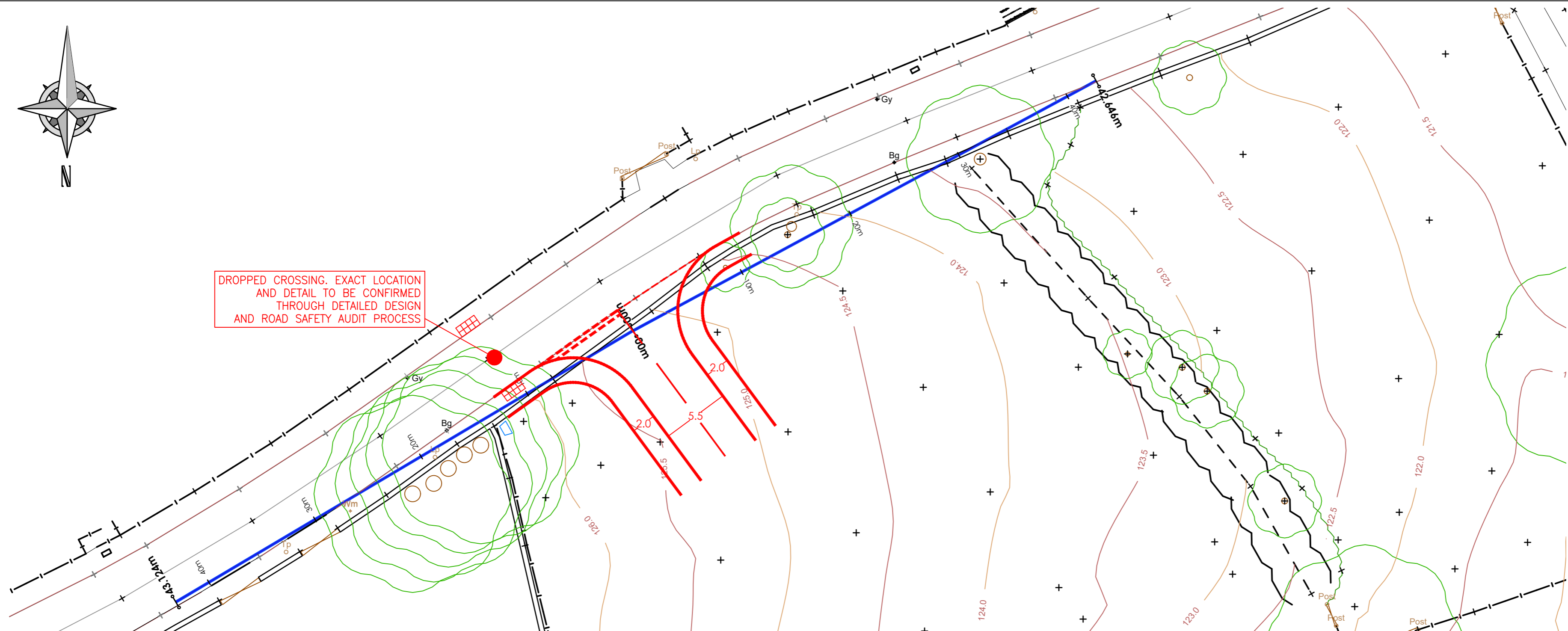
**Hay Green Lane
01.01.2015 to Present**

SCALE	NTS
DATE	16/11/2020
DRWG No.	
DRN BY	

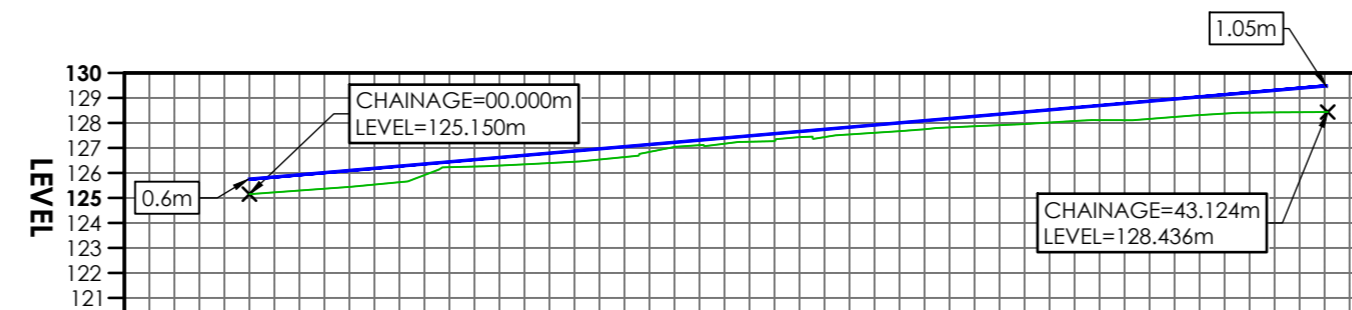
Appendix D Visibility Splay at Access



DROPPED CROSSING. EXACT LOCATION AND DETAIL TO BE CONFIRMED THROUGH DETAILED DESIGN AND ROAD SAFETY AUDIT PROCESS

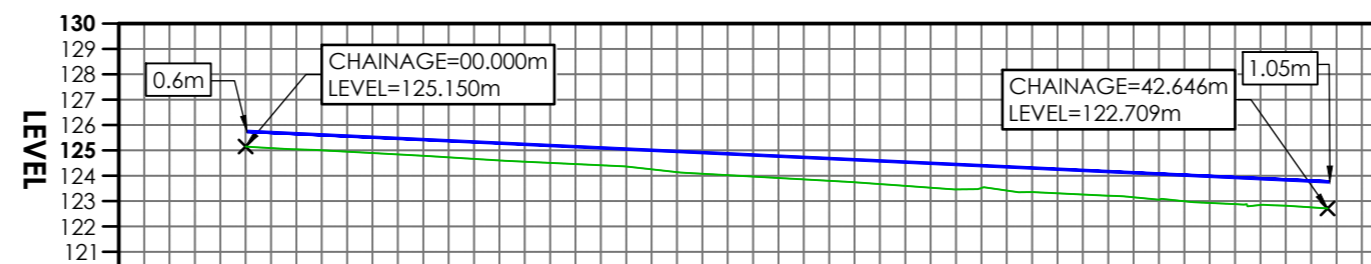


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- KEY
- Proposed Junction Arrangement
 - Vertical Visibility



CHAINAGE	EXISTING LEVELS
-5,000	
-4,000	
-3,000	
-2,000	
-1,000	
0,000	125.150
1,000	125.223
2,000	125.296
3,000	125.369
4,000	125.441
5,000	125.513
6,000	125.587
7,000	125.661
8,000	125.734
9,000	125.807
10,000	125.880
11,000	125.953
12,000	126.026
13,000	126.099
14,000	126.172
15,000	126.245
16,000	126.318
17,000	126.391
18,000	126.464
19,000	126.537
20,000	126.610
21,000	126.683
22,000	126.756
23,000	126.829
24,000	126.902
25,000	126.975
26,000	127.048
27,000	127.121
28,000	127.194
29,000	127.267
30,000	127.340
31,000	127.413
32,000	127.486
33,000	127.559
34,000	127.632
35,000	127.705
36,000	127.778
37,000	127.851
38,000	127.924
39,000	127.997
40,000	128.070
41,000	128.143
42,000	128.216
43,000	128.289
44,000	128.362
45,000	128.435

5568-VIS-LHS - LONGSECTION
SCALE 1:250



CHAINAGE	EXISTING LEVELS
-5,000	
-4,000	
-3,000	
-2,000	
-1,000	
0,000	125.150
1,000	125.089
2,000	125.029
3,000	124.968
4,000	124.904
5,000	124.840
6,000	124.786
7,000	124.725
8,000	124.663
9,000	124.602
10,000	124.543
11,000	124.489
12,000	124.433
13,000	124.374
14,000	124.313
15,000	124.253
16,000	124.192
17,000	124.132
18,000	124.071
19,000	124.011
20,000	123.956
21,000	123.901
22,000	123.855
23,000	123.800
24,000	123.743
25,000	123.671
26,000	123.600
27,000	123.527
28,000	123.460
29,000	123.391
30,000	123.325
31,000	123.255
32,000	123.187
33,000	123.112
34,000	123.037
35,000	122.954
36,000	122.873
37,000	122.797
38,000	122.727
39,000	122.659
40,000	122.584
41,000	122.512
42,000	122.435
43,000	122.353
44,000	122.267
45,000	122.177

5568-VIS-RHS - LONGSECTION
SCALE 1:250

A	19.01.21	TITLE BLOCK UPDATED	KG
-	10.11.20	INITIAL ISSUE	KG
Rev.	Date	Status/Amendments	By



Manchester Leeds
0161 413 5168 0113 323 0854

Client:
HARWORTH

Project:
HAY GREEN LANE, BIRDWELL

Drawing Title:
PROPOSED ACCESS ARRANGEMENT INC VERTICAL VISIBILITY ENVELOPE

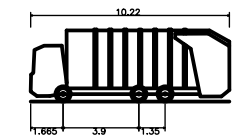
Drawn By:	Checked By:	Approved By:
KG	RM	RM
Scale:	Paper Size:	Date Created:
1:250	A2	10.11.20
Drawing Number:	Drawing Revision:	
18039.IN.13	-	

Appendix E Swept Paths



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Phoenix 2-17N (with Elite 2 6x2 RS chassis)
 Overall Length 10.220m
 Overall Width 2.250m
 Overall Body Height 3.707m
 Min Body Ground Clearance 0.250m
 Track Width 2.250m
 Lock to lock time 4.00s
 Kerb to Kerb Turning Radius 7.900m

Rev:	20.05.20	INITIAL ISSUE	RAM
Date:		Status/Amendments:	By:

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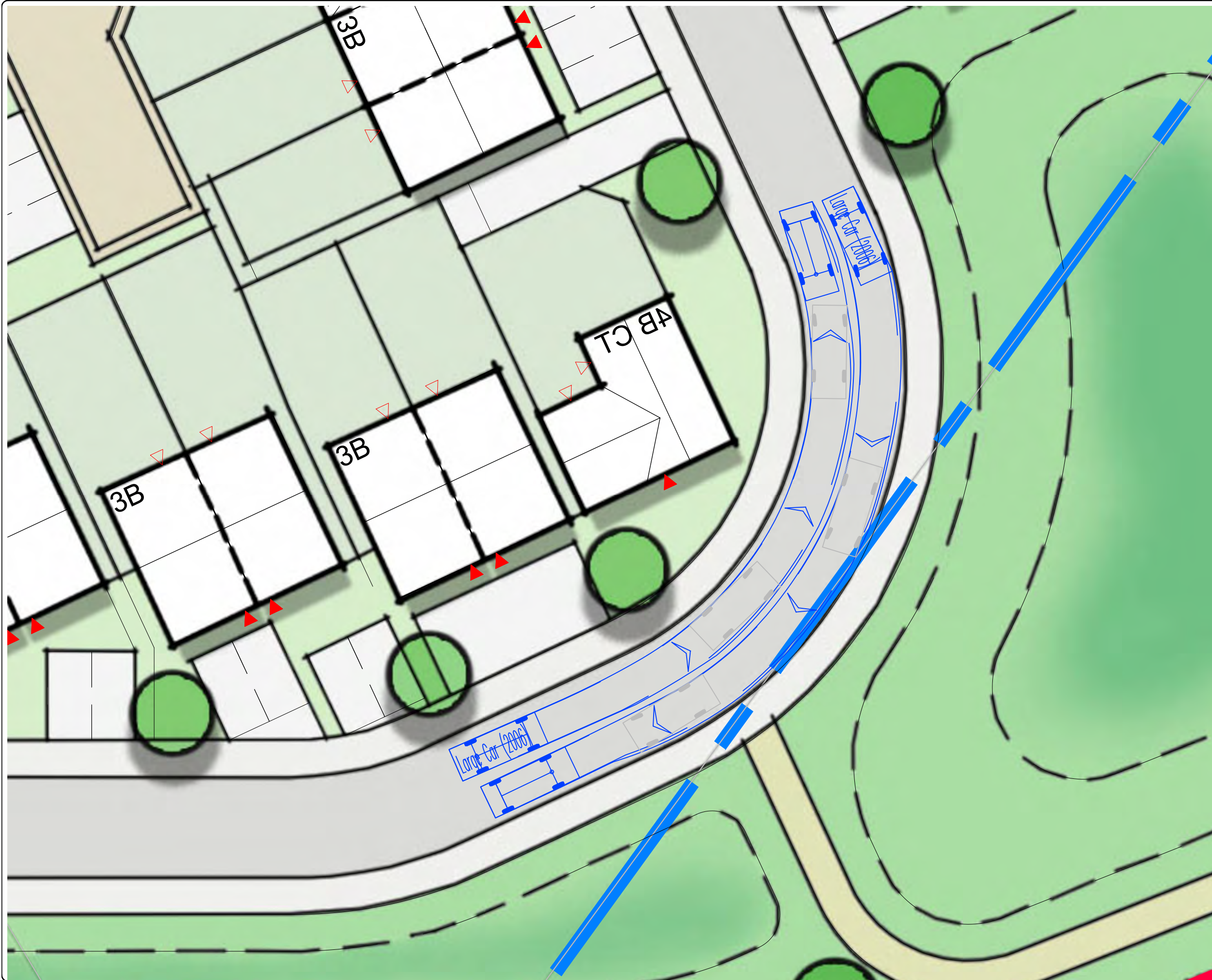
Manchester 0161 413 5168
 Leeds 0113 323 0854

Client:
 HARWORTH

Project:
 HAY GREEN LANE, BIRDWELL

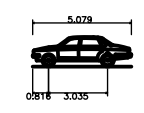
Drawing Title:
 SWEEP PATH ANALYSIS

Drawn By: KG	Checked By: RAM	Approved By: RAM
Scale: 1:500	Paper Size: A3	Date Created: 20.05.2020
Drawing Number: 18039.ATR.01	Drawing Revision: -	



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Large Car (2006)	
Overall Length	5.079m
Overall Width	1.872m
Overall Body Height	1.525m
Min Body Ground Clearance	0.310m
Max Track Width	1.831m
Lock to lock time	4.00s
Kerb to Kerb Turning Radius	5.900m

Rev:	20.05.20	INITIAL ISSUE	RAM
Date:		Status/Amendments:	By:

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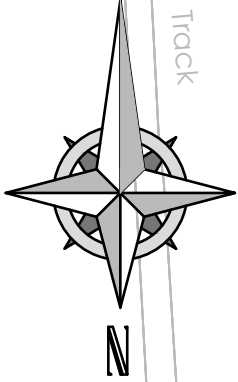
Client:
 HARWORTH

Project:
 HAY GREEN LANE, BIRDWELL

Drawing Title:
 SWEPT PATH ANALYSIS
 LARGE CAR

Drawn By: KG	Checked By: RAM	Approved By: RAM
Scale: 1:200	Paper Size: A3	Date Created: 20.05.2020
Drawing Number: 18039.ATR.02	Drawing Revision: -	

Appendix F Mitigation Proposals



Track

Allotment Gardens

SHEFFIELD ROAD

Methodist Church

Birdwell Primary School

Club

31.2m

128.6m

DOUBLE YELLOW LINES TO BE PROVIDED AROUND JUNCTIONS RADII. EXACT LENGTH TBC.

DOUBLE YELLOW LINES TO BE PROVIDED AROUND JUNCTIONS RADII

POTENTIAL FOR BOLLARDS TO PREVENT PAVEMENT PARKING

POTENTIAL FOR BOLLARDS TO PREVENT PAVEMENT PARKING.

Allotment Gardens

CHapel COURT

Parker's Terrace

Shelter

133.6m

- Notes:
1. This drawing is subject to copyright and is not to be reproduced in part or whole without approval.
 2. Do not scale this drawing - check all dimensions on site.
- KEY
- 2.4m x 43.0m Visibility Splay

Rev:	Date:	Status/Amendments:	By:
E	17.03.21	TRO AMNDED	RAM
D	09.11.20	BIRDWELL VENUE PARKING REMOVED	RAM
C	11.10.20	VISI SPLAY ADDED. CROSSING REMOVED	RAM
B	18.05.20	AMENDED PARKING	RAM
A	04.05.20	AMENDED PARKING	RAM
-	24.03.20	INITIAL ISSUE	RAM



Manchester Leeds
0161 413 5168 0113 323 0854

Client:
HARWORTH

Project:
HAY GREEN LANE,
BIRDWELL

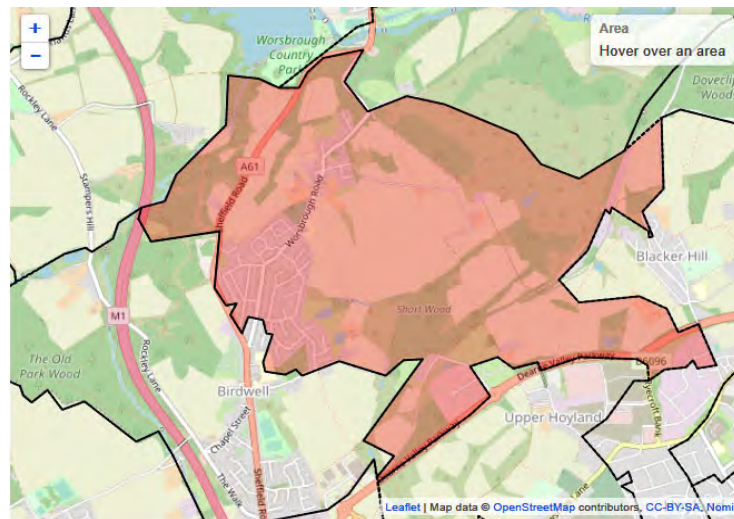
Drawing Title:
PROPOSED TRAFFIC REGULATION ORDER

Drawn By: RAM	Checked By: KG	Approved By: SJP
Scale: 1:1000	Paper Size: A3	Date Created: 24.03.2020
Drawing Number: 18039.IN.08		Drawing Revision: E

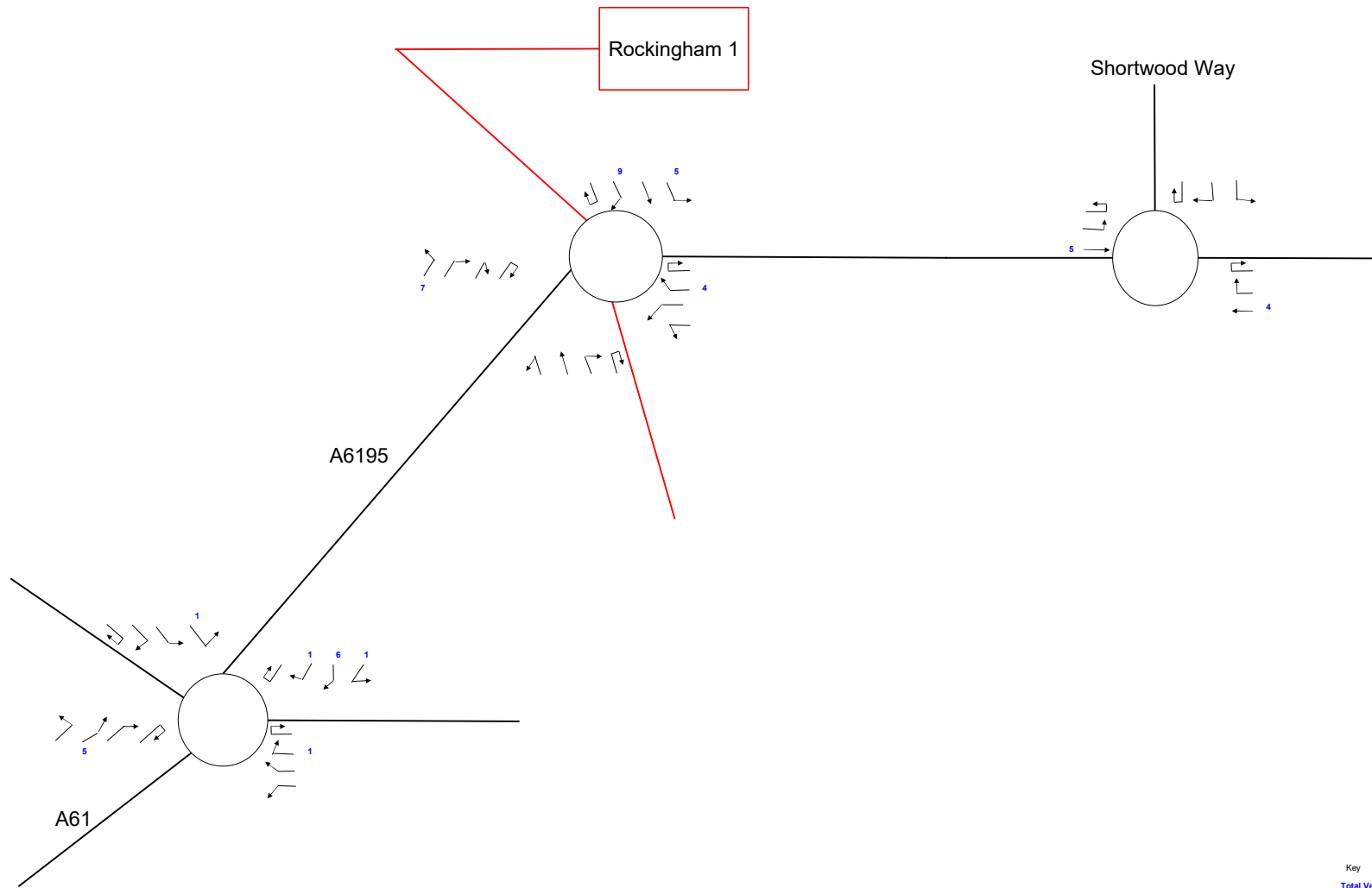
Appendix G Census

E01011800 : Wakefield 029F	1	0.19%	0.19%		
E01011828 : Wakefield 021F	1	0.19%	0.19%		
E01011855 : Wakefield 041B	1	0.19%		0.19%	
E01011860 : Wakefield 044A	1	0.19%		0.19%	
E01011883 : Wakefield 009D	1	0.19%	0.19%		
E01011885 : Wakefield 014A	1	0.19%	0.19%		
E01011898 : Wakefield 030A	1	0.19%	0.19%		
E01011907 : Wakefield 028D	1	0.19%	0.19%		
E01011912 : Wakefield 017B	1	0.19%	0.19%		
E01011916 : Wakefield 017D	1	0.19%	0.19%		
E01011937 : Wakefield 036D	1	0.19%	0.19%		
E01011940 : Wakefield 037C	1	0.19%	0.19%		
E01011941 : Wakefield 038B	1	0.19%			0.19%
E01012061 : Middlesbrough 002D	1	0.19%	0.19%		
E01012882 : Kingston upon Hull 033A	1	0.19%	0.19%		
E01013302 : North Lincolnshire 007C	1	0.19%	0.19%		
E01013647 : Leicester 040B	1	0.19%	0.19%		
E01019200 : Carlisle 002A	1	0.19%	0.19%		
E01019239 : Carlisle 007C	1	0.19%	0.19%		
E01019453 : Amber Valley 017D	1	0.19%	0.19%		
E01019614 : Derbyshire Dales 001B	1	0.19%		0.19%	
E01022099 : Uttlesford 004D	1	0.19%	0.19%		
E01022175 : Cotswold 005A	1	0.19%	0.19%		
E01025303 : Preston 004E	1	0.19%	0.19%		
E01027202 : Northampton 028D	1	0.19%	0.19%		
E01027665 : Harrogate 008C	1	0.19%	0.19%		
E01027923 : Selby 010C	1	0.19%			0.19%
E01028062 : Bassetlaw 013C	1	0.19%	0.19%		
E01028063 : Bassetlaw 014D	1	0.19%	0.19%		
E01028533 : Oxford 013C	1	0.19%	0.19%		
E01032925 : Barnsley 026G	1	0.19%			0.19%
E01033008 : Leeds 111A	1	0.19%	0.19%		
E01033011 : Leeds 111C	1	0.19%	0.19%		
E01033016 : Leeds 111E	1	0.19%	0.19%		
E01033032 : Leeds 082F	1	0.19%	0.19%		
E01033261 : Sheffield 073A	1	0.19%	0.19%		
E01033262 : Sheffield 073B	1	0.19%	0.19%		
E01033265 : Sheffield 074C	1	0.19%	0.19%		
E01033271 : Sheffield 040E	1	0.19%	0.19%		
W01001931 : Wrexham 012G	1	0.19%		0.19%	
E01000375 : Bexley 019B	2	0.38%	0.38%		
E01007317 : Barnsley 018A	2	0.38%			0.38%
E01007318 : Barnsley 018B	2	0.38%			0.38%
E01007328 : Barnsley 006A	2	0.38%			0.38%
E01007334 : Barnsley 009A	2	0.38%			0.38%
E01007370 : Barnsley 022B	2	0.38%			0.38%
E01007373 : Barnsley 014B	2	0.38%			0.38%
E01007381 : Barnsley 012C	2	0.38%	0.38%		
E01007387 : Barnsley 012D	2	0.38%		0.38%	
E01007421 : Barnsley 027A	2	0.38%	0.19%	0.19%	
E01007445 : Barnsley 013G	2	0.38%	0.38%		
E01007448 : Barnsley 023B	2	0.38%			0.38%
E01007457 : Barnsley 029E	2	0.38%			0.38%
E01007527 : Doncaster 027B	2	0.38%	0.38%		
E01007534 : Doncaster 028D	2	0.38%	0.38%		
E01007677 : Rotherham 017A	2	0.38%			0.38%
E01007728 : Rotherham 009A	2	0.38%	0.19%		0.19%
E01007784 : Rotherham 008D	2	0.38%	0.19%		0.19%
E01007816 : Rotherham 002A	2	0.38%			0.38%
E01007854 : Sheffield 014E	2	0.38%	0.38%		
E01007855 : Sheffield 014F	2	0.38%	0.38%		
E01007885 : Sheffield 075E	2	0.38%	0.38%		
E01007897 : Sheffield 005A	2	0.38%		0.38%	
E01007902 : Sheffield 027B	2	0.38%	0.38%		
E01007903 : Sheffield 018A	2	0.38%	0.38%		
E01007904 : Sheffield 018B	2	0.38%	0.38%		
E01007929 : Sheffield 055C	2	0.38%	0.19%	0.19%	
E01008096 : Sheffield 043B	2	0.38%	0.38%		
E01008099 : Sheffield 051E	2	0.38%	0.38%		
E01008104 : Sheffield 036E	2	0.38%	0.19%	0.19%	
E01008124 : Sheffield 012F	2	0.38%	0.38%		
E01008138 : Sheffield 006D	2	0.38%		0.19%	0.19%
E01008143 : Sheffield 002B	2	0.38%	0.38%		
E01011257 : Kirklees 024A	2	0.38%	0.38%		
E01011363 : Leeds 071E	2	0.38%	0.38%		
E01011677 : Leeds 064D	2	0.38%	0.38%		
E01011749 : Wakefield 005B	2	0.38%	0.38%		
E01011758 : Wakefield 013A	2	0.38%	0.38%		
E01011819 : Wakefield 016E	2	0.38%	0.38%		
E01011871 : Wakefield 043D	2	0.38%		0.38%	
E01011935 : Wakefield 038A	2	0.38%			0.38%
E01013790 : Rutland 001C	2	0.38%	0.38%		
E01027904 : Selby 002E	2	0.38%	0.38%		
E01032550 : Barnsley 010G	2	0.38%			0.38%
E01033010 : Leeds 111B	2	0.38%	0.38%		
E01033013 : Leeds 082E	2	0.38%	0.38%		
E01007345 : Barnsley 008B	3	0.57%			0.57%
E01007376 : Barnsley 022D	3	0.57%			0.57%
E01007384 : Barnsley 019C	3	0.57%		0.57%	
E01007395 : Barnsley 030E	3	0.57%		0.57%	
E01007405 : Barnsley 011D	3	0.57%			0.57%
E01007430 : Barnsley 024D	3	0.57%		0.57%	
E01007443 : Barnsley 013F	3	0.57%			0.57%
E01007715 : Rotherham 016A	3	0.57%	0.57%		

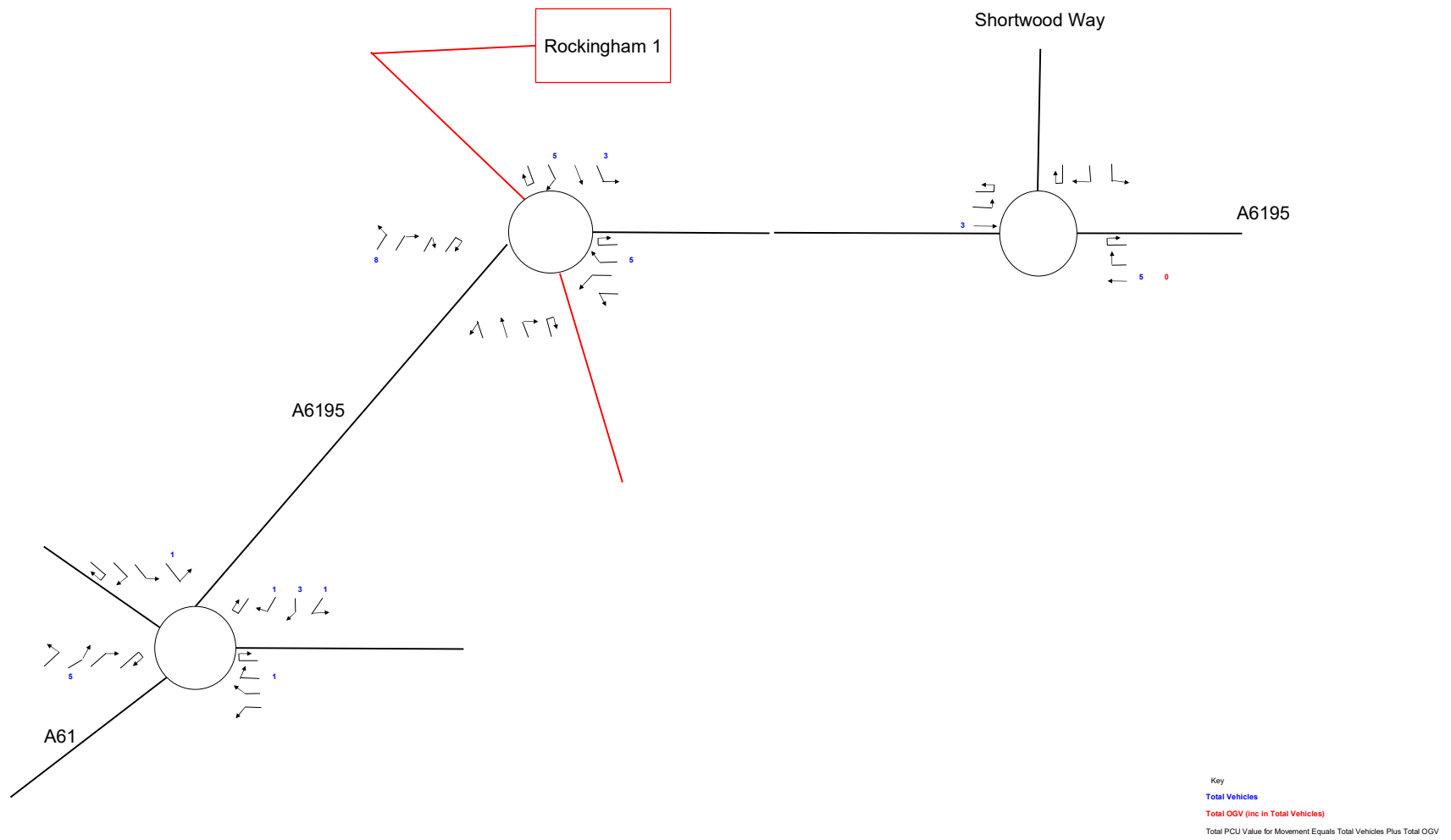
E01007852 : Sheffield 014C	3	0.57%	0.57%										
E01008093 : Sheffield 017D	3	0.57%	0.57%										
E01032924 : Barnsley 026F	3	0.57%				0.57%							
E01007327 : Barnsley 007E	4	0.76%								0.76%			
E01007382 : Barnsley 019A	4	0.76%			0.76%								
E01007396 : Barnsley 028A	4	0.76%						0.76%					
E01007463 : Barnsley 021F	4	0.76%										0.76%	
E01007866 : Sheffield 030A	4	0.76%			0.76%								
E01007913 : Sheffield 018C	4	0.76%			0.76%								
E01007914 : Sheffield 018D	4	0.76%			0.76%								
E01028828 : Shropshire 034A	4	0.76%			0.76%								
E01007319 : Barnsley 015A	5	0.95%										0.95%	
E01007449 : Barnsley 023C	5	0.95%										0.95%	
E01007676 : Rotherham 025A	5	0.95%	0.95%										
E01007694 : Rotherham 001B	5	0.95%				0.95%							
E01007322 : Barnsley 015C	6	1.14%										1.14%	
E01007391 : Barnsley 030C	6	1.14%						1.14%					
E01007714 : Rotherham 017B	6	1.14%	1.14%										
E01008135 : Sheffield 005C	6	1.14%						1.14%					
E01007380 : Barnsley 012B	7	1.33%										1.33%	
E01007397 : Barnsley 028B	7	1.33%						1.33%					
E01007437 : Barnsley 002C	7	1.33%										1.33%	
E01033264 : Sheffield 073D	7	1.33%	1.33%										
E01007820 : Rotherham 002D	8	1.52%				1.52%							
E01007894 : Sheffield 004E	8	1.52%						1.52%					
E01033269 : Sheffield 022G	8	1.52%	1.52%										
E01007460 : Barnsley 021C	9	1.70%										1.70%	
E01007446 : Barnsley 012G	10	1.89%										1.89%	
E01007889 : Sheffield 004B	10	1.89%						1.89%					
E01007337 : Barnsley 015D	11	2.08%										2.08%	
E01007398 : Barnsley 028C	13	2.46%										2.46%	
E01007336 : Barnsley 013A	19	3.60%										3.60%	
E01007424 : Barnsley 028E	28	5.30%				5.30%							
E01007340 : Barnsley 013B	35	6.63%										6.63%	
E01007399 : Barnsley 028D	10	1.89%										1.89%	
TOTAL	528	100.00%	13.07%	20.27%	4.07%	8.43%	11.55%	7.77%	0.00%	0.00%	22.54%	7.95%	4.36%



Appendix H Committed Development Traffic



Key
 Total Vehicles
 Total OGV (inc in Total Vehicles)
 Total PCU Value for Movement Equals Total Vehicles Plus Total OGV



Appendix I Turning Movements & Modelling Outputs

<h1>Junctions 9</h1>
<h2>PICADY 9 - Priority Intersection Module</h2>
Version: 9.5.1.7462 © Copyright TRL Limited, 2019
For sales and distribution information, program advice and maintenance, contact TRL: +44 (0)1344 379777 software@trl.co.uk www.trlsoftware.co.uk
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Filename: 201028 Hay Green Lane, Sheffield Road.j9
Path: L:\Hay Green Lane, Birdwell - 18039\ANALYSIS\CAPACITY\Priority Junctions
Report generation date: 20/01/2021 14:49:17

- »2019 Count, AM
- »2019 Count, PM
- »2019 Base Scenario, AM
- »2019 Base Scenario, PM
- »2025 Base Scenario, AM
- »2025 Base Scenario, PM
- »2025 Design Scenario, AM
- »2025 Design Scenario, PM
- »2025 Design Sensitivity Scenario, AM
- »2025 Design Sensitivity Scenario, PM
- »2033 Base Scenario, AM
- »2033 Base Scenario, PM
- »2033 Design Scenario, AM
- »2033 Design Scenario, PM
- »2033 Design Sensitivity Scenario , AM
- »2033 Design Sensitivity Scenario , PM

Summary of junction performance

	AM					PM				
	Set ID	Queue (PCU)	Delay (s)	RFC	LOS	Set ID	Queue (PCU)	Delay (s)	RFC	LOS
2019 Count										
Stream B-AC	D1	0.2	13.38	0.13	B	D2	0.1	10.60	0.08	B
Stream C-AB		0.1	9.10	0.10	A		0.1	8.93	0.08	A
2019 Base Scenario										
Stream B-AC	D3	0.2	13.40	0.13	B	D4	0.1	10.61	0.08	B
Stream C-AB		0.1	9.11	0.10	A		0.1	8.94	0.08	A
2025 Base Scenario										
Stream B-AC	D5	0.2	14.29	0.15	B	D6	0.1	11.07	0.09	B
Stream C-AB		0.1	9.43	0.11	A		0.1	9.24	0.09	A
2025 Design Scenario										
Stream B-AC	D7	0.5	20.33	0.35	C	D8	0.2	14.35	0.17	B
Stream C-AB		0.2	9.77	0.14	A		0.2	10.09	0.16	B
2025 Design Sensitivity Scenario										
Stream B-AC	D9	0.6	21.68	0.36	C	D10	0.2	14.74	0.18	B
Stream C-AB		0.2	9.94	0.14	A		0.2	10.22	0.16	B
2033 Base Scenario										
Stream B-AC	D11	0.2	16.25	0.18	C	D12	0.1	11.70	0.10	B
Stream C-AB		0.1	9.84	0.12	A		0.1	9.63	0.09	A
2033 Design Scenario										
Stream B-AC	D13	0.6	24.27	0.40	C	D14	0.2	15.78	0.19	C
Stream C-AB		0.2	10.22	0.15	B		0.2	10.56	0.17	B
2033 Design Sensitivity Scenario										
Stream B-AC	D15	0.7	26.39	0.42	D	D16	0.2	16.28	0.20	C
Stream C-AB		0.2	10.41	0.16	B		0.2	10.70	0.17	B

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.

Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle.

File summary

File Description

Title	Hay Green Lane / Sheffield Road Junction
Location	
Site number	
Date	22/04/2020
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	ZERUM\kathryn.griffiths
Description	

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Average delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perHour	s	-Min	perMin

Analysis Options

Vehicle length (m)	Calculate Queue Percentiles	Calculate detailed queueing delay	Calculate residual capacity	RFC Threshold	Average Delay threshold (s)	Queue threshold (PCU)
5.75				0.85	36.00	20.00

Demand Set Summary

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)	Run automatically
D1	2019 Count	AM	ONE HOUR	08:30	10:00	15	✓
D2	2019 Count	PM	ONE HOUR	16:30	18:00	15	✓
D3	2019 Base Scenario	AM	ONE HOUR	08:30	10:00	15	✓
D4	2019 Base Scenario	PM	ONE HOUR	16:30	18:00	15	✓
D5	2025 Base Scenario	AM	ONE HOUR	08:30	10:00	15	✓
D6	2025 Base Scenario	PM	ONE HOUR	16:30	18:00	15	✓
D7	2025 Design Scenario	AM	ONE HOUR	08:30	10:00	15	✓
D8	2025 Design Scenario	PM	ONE HOUR	16:30	18:00	15	✓
D9	2025 Design Sensitivity Scenario	AM	ONE HOUR	08:30	10:00	15	✓
D10	2025 Design Sensitivity Scenario	PM	ONE HOUR	16:30	18:00	15	✓
D11	2033 Base Scenario	AM	ONE HOUR	08:30	10:00	15	✓
D12	2033 Base Scenario	PM	ONE HOUR	16:30	18:00	15	✓
D13	2033 Design Scenario	AM	ONE HOUR	08:30	10:00	15	✓
D14	2033 Design Scenario	PM	ONE HOUR	16:30	18:00	15	✓
D15	2033 Design Sensitivity Scenario	AM	ONE HOUR	08:30	10:00	15	✓
D16	2033 Design Sensitivity Scenario	PM	ONE HOUR	16:30	18:00	15	✓

Analysis Set Details

ID	Include in report	Network flow scaling factor (%)	Network capacity scaling factor (%)
A1	✓	100.000	100.000

2019 Count, AM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.51	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Name	Description	Arm type
A	A61 Sheffuekd Road (N)		Major
B	Hay Green Lane		Minor
C	Sheffield Road (S)		Major

Major Arm Geometry

Arm	Width of carriageway (m)	Has kerbed central reserve	Has right turn bay	Width for right turn (m)	Visibility for right turn (m)	Blocks?	Blocking queue (PCU)
C	7.90		✓	2.30	90.0	✓	6.20

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor arm type	Lane width (m)	Visibility to left (m)	Visibility to right (m)
B	One lane	2.34	18	14

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Stream	Intercept (PCU/hr)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
B-A	458	0.077	0.193	0.122	0.276
B-C	591	0.083	0.210	-	-
C-B	633	0.225	0.225	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)	Run automatically
D1	2019 Count	AM	ONE HOUR	08:30	10:00	15	✓

Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)
✓	✓	HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A		ONE HOUR	✓	772	100.000
B		ONE HOUR	✓	38	100.000
C		ONE HOUR	✓	953	100.000

Origin-Destination Data

Demand (PCU/hr)

	To			
	A	B	C	
From	A	0	14	758
	B	8	0	30
	C	911	42	0

Vehicle Mix

Heavy Vehicle Percentages

	To			
	A	B	C	
From	A	0	0	0
	B	0	0	0
	C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS	Average Demand (PCU/hr)	Total Junction Arrivals (PCU)
B-AC	0.13	13.38	0.2	B	35	52
C-AB	0.10	9.10	0.1	A	39	58
C-A					836	1254
A-B					13	19
A-C					696	1043

Main Results for each time segment

08:30 - 08:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	29	7	399	0.072	28	0.0	0.1	9.699	A
C-AB	32	8	502	0.063	31	0.0	0.1	7.643	A
C-A	686	171			686				
A-B	11	3			11				
A-C	571	143			571				

08:45 - 09:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	34	9	364	0.094	34	0.1	0.1	10.898	B
C-AB	38	9	477	0.079	38	0.1	0.1	8.198	A
C-A	819	205			819				
A-B	13	3			13				
A-C	681	170			681				

09:00 - 09:15

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	42	10	311	0.135	42	0.1	0.2	13.360	B
C-AB	46	12	442	0.105	46	0.1	0.1	9.097	A
C-A	1003	251			1003				
A-B	15	4			15				
A-C	835	209			835				

09:15 - 09:30

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	42	10	311	0.135	42	0.2	0.2	13.379	B
C-AB	46	12	442	0.105	46	0.1	0.1	9.103	A
C-A	1003	251			1003				
A-B	15	4			15				
A-C	835	209			835				

09:30 - 09:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	34	9	364	0.094	34	0.2	0.1	10.921	B
C-AB	38	9	477	0.079	38	0.1	0.1	8.205	A
C-A	819	205			819				
A-B	13	3			13				
A-C	681	170			681				

09:45 - 10:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	29	7	399	0.072	29	0.1	0.1	9.721	A
C-AB	32	8	502	0.063	32	0.1	0.1	7.652	A
C-A	686	171			686				
A-B	11	3			11				
A-C	571	143			571				

2019 Count, PM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.33	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)	Run automatically
D2	2019 Count	PM	ONE HOUR	16:30	18:00	15	✓

Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)
✓	✓	HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A		ONE HOUR	✓	790	100.000
B		ONE HOUR	✓	27	100.000
C		ONE HOUR	✓	894	100.000

Origin-Destination Data

Demand (PCU/hr)

		To		
		A	B	C
From	A	0	8	782
	B	2	0	25
	C	863	31	0

Vehicle Mix

Heavy Vehicle Percentages

		To		
		A	B	C
From	A	0	0	0
	B	0	0	0
	C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS	Average Demand (PCU/hr)	Total Junction Arrivals (PCU)
B-AC	0.08	10.60	0.1	B	25	37
C-AB	0.08	8.93	0.1	A	28	43
C-A					792	1188
A-B					7	11
A-C					718	1076

Main Results for each time segment

16:30 - 16:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	20	5	441	0.046	20	0.0	0.0	8.559	A
C-AB	23	6	499	0.047	23	0.0	0.0	7.559	A
C-A	650	162			650				
A-B	6	2			6				
A-C	589	147			589				

16:45 - 17:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	24	6	412	0.059	24	0.0	0.1	9.288	A
C-AB	28	7	473	0.059	28	0.0	0.1	8.083	A
C-A	776	194			776				
A-B	7	2			7				
A-C	703	176			703				

17:00 - 17:15

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	30	7	369	0.081	30	0.1	0.1	10.596	B
C-AB	34	9	437	0.078	34	0.1	0.1	8.926	A
C-A	950	238			950				
A-B	9	2			9				
A-C	861	215			861				

17:15 - 17:30

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	30	7	369	0.081	30	0.1	0.1	10.602	B
C-AB	34	9	437	0.078	34	0.1	0.1	8.930	A
C-A	950	238			950				
A-B	9	2			9				
A-C	861	215			861				

17:30 - 17:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	24	6	412	0.059	24	0.1	0.1	9.295	A
C-AB	28	7	473	0.059	28	0.1	0.1	8.089	A
C-A	776	194			776				
A-B	7	2			7				
A-C	703	176			703				

17:45 - 18:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	20	5	441	0.046	20	0.1	0.0	8.569	A
C-AB	23	6	499	0.047	23	0.1	0.0	7.567	A
C-A	650	162			650				
A-B	6	2			6				
A-C	589	147			589				

2019 Base Scenario, AM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.51	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)	Run automatically
D3	2019 Base Scenario	AM	ONE HOUR	08:30	10:00	15	✓

Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)
✓	✓	HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A		ONE HOUR	✓	773	100.000
B		ONE HOUR	✓	38	100.000
C		ONE HOUR	✓	954	100.000

Origin-Destination Data

Demand (PCU/hr)

		To		
		A	B	C
From	A	0	14	759
	B	8	0	30
	C	912	42	0

Vehicle Mix

Heavy Vehicle Percentages

		To		
		A	B	C
From	A	0	0	0
	B	0	0	0
	C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS	Average Demand (PCU/hr)	Total Junction Arrivals (PCU)
B-AC	0.13	13.40	0.2	B	35	52
C-AB	0.10	9.11	0.1	A	39	58
C-A					837	1255
A-B					13	19
A-C					696	1045

Main Results for each time segment

08:30 - 08:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	29	7	399	0.072	28	0.0	0.1	9.704	A
C-AB	32	8	502	0.063	31	0.0	0.1	7.646	A
C-A	687	172			687				
A-B	11	3			11				
A-C	571	143			571				

08:45 - 09:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	34	9	364	0.094	34	0.1	0.1	10.907	B
C-AB	38	9	477	0.079	38	0.1	0.1	8.201	A
C-A	820	205			820				
A-B	13	3			13				
A-C	682	171			682				

09:00 - 09:15

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	42	10	311	0.135	42	0.1	0.2	13.378	B
C-AB	46	12	441	0.105	46	0.1	0.1	9.103	A
C-A	1004	251			1004				
A-B	15	4			15				
A-C	836	209			836				

09:15 - 09:30

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	42	10	311	0.135	42	0.2	0.2	13.398	B
C-AB	46	12	441	0.105	46	0.1	0.1	9.108	A
C-A	1004	251			1004				
A-B	15	4			15				
A-C	836	209			836				

09:30 - 09:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	34	9	364	0.094	34	0.2	0.1	10.930	B
C-AB	38	9	477	0.079	38	0.1	0.1	8.209	A
C-A	820	205			820				
A-B	13	3			13				
A-C	682	171			682				

09:45 - 10:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	29	7	399	0.072	29	0.1	0.1	9.728	A
C-AB	32	8	502	0.063	32	0.1	0.1	7.655	A
C-A	687	172			687				
A-B	11	3			11				
A-C	571	143			571				

2019 Base Scenario, PM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.33	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)	Run automatically
D4	2019 Base Scenario	PM	ONE HOUR	16:30	18:00	15	✓

Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)
✓	✓	HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A		ONE HOUR	✓	791	100.000
B		ONE HOUR	✓	27	100.000
C		ONE HOUR	✓	895	100.000

Origin-Destination Data

Demand (PCU/hr)

		To		
		A	B	C
From	A	0	8	783
	B	2	0	25
	C	864	31	0

Vehicle Mix

Heavy Vehicle Percentages

		To		
		A	B	C
From	A	0	0	0
	B	0	0	0
	C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS	Average Demand (PCU/hr)	Total Junction Arrivals (PCU)
B-AC	0.08	10.61	0.1	B	25	37
C-AB	0.08	8.94	0.1	A	28	43
C-A					793	1189
A-B					7	11
A-C					718	1078

Main Results for each time segment

16:30 - 16:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	20	5	440	0.046	20	0.0	0.0	8.563	A
C-AB	23	6	499	0.047	23	0.0	0.0	7.562	A
C-A	650	163			650				
A-B	6	2			6				
A-C	589	147			589				

16:45 - 17:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	24	6	412	0.059	24	0.0	0.1	9.294	A
C-AB	28	7	473	0.059	28	0.0	0.1	8.086	A
C-A	777	194			777				
A-B	7	2			7				
A-C	704	176			704				

17:00 - 17:15

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	30	7	369	0.081	30	0.1	0.1	10.605	B
C-AB	34	9	437	0.078	34	0.1	0.1	8.932	A
C-A	951	238			951				
A-B	9	2			9				
A-C	862	216			862				

17:15 - 17:30

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	30	7	369	0.081	30	0.1	0.1	10.612	B
C-AB	34	9	437	0.078	34	0.1	0.1	8.935	A
C-A	951	238			951				
A-B	9	2			9				
A-C	862	216			862				

17:30 - 17:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	24	6	412	0.059	24	0.1	0.1	9.300	A
C-AB	28	7	473	0.059	28	0.1	0.1	8.092	A
C-A	777	194			777				
A-B	7	2			7				
A-C	704	176			704				

17:45 - 18:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	20	5	440	0.046	20	0.1	0.0	8.574	A
C-AB	23	6	499	0.047	23	0.1	0.0	7.573	A
C-A	650	163			650				
A-B	6	2			6				
A-C	589	147			589				

2025 Base Scenario, AM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.53	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)	Run automatically
D5	2025 Base Scenario	AM	ONE HOUR	08:30	10:00	15	✓

Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)
✓	✓	HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A		ONE HOUR	✓	818	100.000
B		ONE HOUR	✓	40	100.000
C		ONE HOUR	✓	1009	100.000

Origin-Destination Data

Demand (PCU/hr)

		To		
		A	B	C
From	A	0	15	803
	B	8	0	32
	C	965	44	0

Vehicle Mix

Heavy Vehicle Percentages

		To		
		A	B	C
From	A	0	0	0
	B	0	0	0
	C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS	Average Demand (PCU/hr)	Total Junction Arrivals (PCU)
B-AC	0.15	14.29	0.2	B	37	55
C-AB	0.11	9.43	0.1	A	40	61
C-A					886	1328
A-B					14	21
A-C					737	1105

Main Results for each time segment

08:30 - 08:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	30	8	392	0.077	30	0.0	0.1	9.931	A
C-AB	33	8	494	0.067	33	0.0	0.1	7.795	A
C-A	727	182			727				
A-B	11	3			11				
A-C	605	151			605				

08:45 - 09:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	36	9	354	0.101	36	0.1	0.1	11.295	B
C-AB	40	10	467	0.085	39	0.1	0.1	8.411	A
C-A	868	217			868				
A-B	13	3			13				
A-C	722	180			722				

09:00 - 09:15

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	44	11	296	0.149	44	0.1	0.2	14.266	B
C-AB	48	12	430	0.113	48	0.1	0.1	9.421	A
C-A	1062	266			1062				
A-B	17	4			17				
A-C	884	221			884				

09:15 - 09:30

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	44	11	296	0.149	44	0.2	0.2	14.292	B
C-AB	48	12	430	0.113	48	0.1	0.1	9.427	A
C-A	1062	266			1062				
A-B	17	4			17				
A-C	884	221			884				

09:30 - 09:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	36	9	354	0.101	36	0.2	0.1	11.322	B
C-AB	40	10	467	0.085	40	0.1	0.1	8.419	A
C-A	868	217			868				
A-B	13	3			13				
A-C	722	180			722				

09:45 - 10:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	30	8	392	0.077	30	0.1	0.1	9.958	A
C-AB	33	8	494	0.067	33	0.1	0.1	7.809	A
C-A	727	182			727				
A-B	11	3			11				
A-C	605	151			605				

2025 Base Scenario, PM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.34	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)	Run automatically
D6	2025 Base Scenario	PM	ONE HOUR	16:30	18:00	15	✓

Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)
✓	✓	HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A		ONE HOUR	✓	836	100.000
B		ONE HOUR	✓	28	100.000
C		ONE HOUR	✓	947	100.000

Origin-Destination Data

Demand (PCU/hr)

		To		
		A	B	C
From	A	0	8	828
	B	2	0	26
	C	914	33	0

Vehicle Mix

Heavy Vehicle Percentages

		To		
		A	B	C
From	A	0	0	0
	B	0	0	0
	C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS	Average Demand (PCU/hr)	Total Junction Arrivals (PCU)
B-AC	0.09	11.07	0.1	B	26	39
C-AB	0.09	9.24	0.1	A	30	45
C-A					839	1258
A-B					7	11
A-C					760	1140

Main Results for each time segment

16:30 - 16:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	21	5	433	0.049	21	0.0	0.1	8.734	A
C-AB	25	6	491	0.051	25	0.0	0.1	7.711	A
C-A	688	172			688				
A-B	6	2			6				
A-C	623	156			623				

16:45 - 17:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	25	6	402	0.063	25	0.1	0.1	9.548	A
C-AB	30	7	464	0.064	30	0.1	0.1	8.289	A
C-A	822	205			822				
A-B	7	2			7				
A-C	744	186			744				

17:00 - 17:15

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	31	8	356	0.087	31	0.1	0.1	11.065	B
C-AB	36	9	426	0.085	36	0.1	0.1	9.238	A
C-A	1006	252			1006				
A-B	9	2			9				
A-C	912	228			912				

17:15 - 17:30

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	31	8	356	0.087	31	0.1	0.1	11.072	B
C-AB	36	9	426	0.085	36	0.1	0.1	9.241	A
C-A	1006	252			1006				
A-B	9	2			9				
A-C	912	228			912				

17:30 - 17:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	25	6	402	0.063	25	0.1	0.1	9.555	A
C-AB	30	7	464	0.064	30	0.1	0.1	8.296	A
C-A	822	205			822				
A-B	7	2			7				
A-C	744	186			744				

17:45 - 18:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	21	5	433	0.049	21	0.1	0.1	8.744	A
C-AB	25	6	491	0.051	25	0.1	0.1	7.720	A
C-A	688	172			688				
A-B	6	2			6				
A-C	623	156			623				

2025 Design Scenario, AM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		1.18	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)	Run automatically
D7	2025 Design Scenario	AM	ONE HOUR	08:30	10:00	15	✓

Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)
✓	✓	HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A		ONE HOUR	✓	823	100.000
B		ONE HOUR	✓	85	100.000
C		ONE HOUR	✓	1020	100.000

Origin-Destination Data

Demand (PCU/hr)

		To		
		A	B	C
From	A	0	20	803
	B	22	0	63
	C	965	55	0

Vehicle Mix

Heavy Vehicle Percentages

		To		
		A	B	C
From	A	0	0	0
	B	0	0	0
	C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS	Average Demand (PCU/hr)	Total Junction Arrivals (PCU)
B-AC	0.35	20.33	0.5	C	78	117
C-AB	0.14	9.77	0.2	A	50	76
C-A					886	1328
A-B					18	28
A-C					737	1105

Main Results for each time segment

08:30 - 08:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	64	16	373	0.171	63	0.0	0.2	11.581	B
C-AB	41	10	494	0.084	41	0.0	0.1	7.949	A
C-A	727	182			727				
A-B	15	4			15				
A-C	605	151			605				

08:45 - 09:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	76	19	333	0.229	76	0.2	0.3	13.981	B
C-AB	49	12	466	0.106	49	0.1	0.1	8.628	A
C-A	868	217			868				
A-B	18	4			18				
A-C	722	180			722				

09:00 - 09:15

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	94	23	271	0.346	93	0.3	0.5	20.142	C
C-AB	61	15	429	0.141	60	0.1	0.2	9.759	A
C-A	1062	266			1062				
A-B	22	6			22				
A-C	884	221			884				

09:15 - 09:30

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	94	23	270	0.346	94	0.5	0.5	20.335	C
C-AB	61	15	429	0.141	61	0.2	0.2	9.768	A
C-A	1062	266			1062				
A-B	22	6			22				
A-C	884	221			884				

09:30 - 09:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	76	19	333	0.229	77	0.5	0.3	14.120	B
C-AB	49	12	466	0.106	50	0.2	0.1	8.641	A
C-A	868	217			868				
A-B	18	4			18				
A-C	722	180			722				

09:45 - 10:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	64	16	373	0.171	64	0.3	0.2	11.674	B
C-AB	41	10	494	0.084	42	0.1	0.1	7.966	A
C-A	727	182			727				
A-B	15	4			15				
A-C	605	151			605				

2025 Design Scenario, PM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.69	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)	Run automatically
D8	2025 Design Scenario	PM	ONE HOUR	16:30	18:00	15	✓

Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)
✓	✓	HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A		ONE HOUR	✓	848	100.000
B		ONE HOUR	✓	48	100.000
C		ONE HOUR	✓	974	100.000

Origin-Destination Data

Demand (PCU/hr)

		To		
		A	B	C
From	A	0	20	828
	B	8	0	40
	C	914	60	0

Vehicle Mix

Heavy Vehicle Percentages

		To		
		A	B	C
From	A	0	0	0
	B	0	0	0
	C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS	Average Demand (PCU/hr)	Total Junction Arrivals (PCU)
B-AC	0.17	14.35	0.2	B	44	66
C-AB	0.16	10.09	0.2	B	55	83
C-A					839	1258
A-B					18	28
A-C					760	1140

Main Results for each time segment

16:30 - 16:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	36	9	398	0.091	36	0.0	0.1	9.917	A
C-AB	45	11	489	0.092	45	0.0	0.1	8.091	A
C-A	688	172			688				
A-B	15	4			15				
A-C	623	156			623				

16:45 - 17:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	43	11	361	0.119	43	0.1	0.1	11.300	B
C-AB	54	13	461	0.117	54	0.1	0.1	8.829	A
C-A	822	205			822				
A-B	18	4			18				
A-C	744	186			744				

17:00 - 17:15

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	53	13	304	0.174	53	0.1	0.2	14.312	B
C-AB	66	17	423	0.156	66	0.1	0.2	10.075	B
C-A	1006	252			1006				
A-B	22	6			22				
A-C	912	228			912				

17:15 - 17:30

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	53	13	304	0.174	53	0.2	0.2	14.353	B
C-AB	66	17	423	0.156	66	0.2	0.2	10.088	B
C-A	1006	252			1006				
A-B	22	6			22				
A-C	912	228			912				

17:30 - 17:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	43	11	361	0.119	43	0.2	0.1	11.331	B
C-AB	54	13	461	0.117	54	0.2	0.1	8.845	A
C-A	822	205			822				
A-B	18	4			18				
A-C	744	186			744				

17:45 - 18:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	36	9	398	0.091	36	0.1	0.1	9.949	A
C-AB	45	11	489	0.092	45	0.1	0.1	8.111	A
C-A	688	172			688				
A-B	15	4			15				
A-C	623	156			623				

2025 Design Sensitivity Scenario, AM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		1.21	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)	Run automatically
D9	2025 Design Sensitivity Scenario	AM	ONE HOUR	08:30	10:00	15	✓

Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)
✓	✓	HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A		ONE HOUR	✓	849	100.000
B		ONE HOUR	✓	85	100.000
C		ONE HOUR	✓	1046	100.000

Origin-Destination Data

Demand (PCU/hr)

		To		
		A	B	C
From	A	0	20	829
	B	22	0	63
	C	991	55	0

Vehicle Mix

Heavy Vehicle Percentages

		To		
		A	B	C
From	A	0	0	0
	B	0	0	0
	C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS	Average Demand (PCU/hr)	Total Junction Arrivals (PCU)
B-AC	0.36	21.68	0.6	C	78	117
C-AB	0.14	9.94	0.2	A	50	76
C-A					909	1364
A-B					18	28
A-C					761	1141

Main Results for each time segment

08:30 - 08:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	64	16	367	0.174	63	0.0	0.2	11.802	B
C-AB	41	10	489	0.085	41	0.0	0.1	8.028	A
C-A	746	187			746				
A-B	15	4			15				
A-C	624	156			624				

08:45 - 09:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	76	19	326	0.235	76	0.2	0.3	14.404	B
C-AB	49	12	461	0.107	49	0.1	0.1	8.739	A
C-A	891	223			891				
A-B	18	4			18				
A-C	745	186			745				

09:00 - 09:15

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	94	23	260	0.361	93	0.3	0.5	21.440	C
C-AB	61	15	423	0.143	60	0.1	0.2	9.932	A
C-A	1091	273			1091				
A-B	22	6			22				
A-C	913	228			913				

09:15 - 09:30

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	94	23	259	0.361	94	0.5	0.6	21.680	C
C-AB	61	15	423	0.143	61	0.2	0.2	9.942	A
C-A	1091	273			1091				
A-B	22	6			22				
A-C	913	228			913				

09:30 - 09:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	76	19	326	0.235	77	0.6	0.3	14.561	B
C-AB	49	12	461	0.107	50	0.2	0.1	8.751	A
C-A	891	223			891				
A-B	18	4			18				
A-C	745	186			745				

09:45 - 10:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	64	16	367	0.174	64	0.3	0.2	11.899	B
C-AB	41	10	489	0.085	42	0.1	0.1	8.044	A
C-A	746	187			746				
A-B	15	4			15				
A-C	624	156			624				

2025 Design Sensitivity Scenario, PM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.69	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)	Run automatically
D10	2025 Design Sensitivity Scenario	PM	ONE HOUR	16:30	18:00	15	✓

Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)
✓	✓	HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A		ONE HOUR	✓	867	100.000
B		ONE HOUR	✓	48	100.000
C		ONE HOUR	✓	987	100.000

Origin-Destination Data

Demand (PCU/hr)

		To		
		A	B	C
From	A	0	21	846
	B	8	0	40
	C	927	60	0

Vehicle Mix

Heavy Vehicle Percentages

		To		
		A	B	C
From	A	0	0	0
	B	0	0	0
	C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS	Average Demand (PCU/hr)	Total Junction Arrivals (PCU)
B-AC	0.18	14.74	0.2	B	44	66
C-AB	0.16	10.22	0.2	B	55	83
C-A					851	1276
A-B					19	29
A-C					776	1164

Main Results for each time segment

16:30 - 16:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	36	9	395	0.092	36	0.0	0.1	10.015	B
C-AB	45	11	486	0.093	45	0.0	0.1	8.150	A
C-A	698	174			698				
A-B	16	4			16				
A-C	637	159			637				

16:45 - 17:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	43	11	357	0.121	43	0.1	0.1	11.466	B
C-AB	54	13	458	0.118	54	0.1	0.1	8.913	A
C-A	833	208			833				
A-B	19	5			19				
A-C	761	190			761				

17:00 - 17:15

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	53	13	297	0.178	53	0.1	0.2	14.705	B
C-AB	66	17	418	0.158	66	0.1	0.2	10.211	B
C-A	1021	255			1021				
A-B	23	6			23				
A-C	931	233			931				

17:15 - 17:30

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	53	13	297	0.178	53	0.2	0.2	14.742	B
C-AB	66	17	418	0.158	66	0.2	0.2	10.223	B
C-A	1021	255			1021				
A-B	23	6			23				
A-C	931	233			931				

17:30 - 17:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	43	11	357	0.121	43	0.2	0.1	11.499	B
C-AB	54	13	458	0.118	54	0.2	0.1	8.929	A
C-A	833	208			833				
A-B	19	5			19				
A-C	761	190			761				

17:45 - 18:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	36	9	395	0.092	36	0.1	0.1	10.047	B
C-AB	45	11	486	0.093	45	0.1	0.1	8.171	A
C-A	698	174			698				
A-B	16	4			16				
A-C	637	159			637				

2033 Base Scenario, AM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.58	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)	Run automatically
D11	2033 Base Scenario	AM	ONE HOUR	08:30	10:00	15	✓

Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)
✓	✓	HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A		ONE HOUR	✓	870	100.000
B		ONE HOUR	✓	43	100.000
C		ONE HOUR	✓	1073	100.000

Origin-Destination Data

Demand (PCU/hr)

		To		
		A	B	C
From	A	0	16	854
	B	9	0	34
	C	1026	47	0

Vehicle Mix

Heavy Vehicle Percentages

		To		
		A	B	C
From	A	0	0	0
	B	0	0	0
	C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS	Average Demand (PCU/hr)	Total Junction Arrivals (PCU)
B-AC	0.18	16.25	0.2	C	39	59
C-AB	0.12	9.84	0.1	A	43	65
C-A					941	1412
A-B					15	22
A-C					784	1175

Main Results for each time segment

08:30 - 08:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	32	8	377	0.086	32	0.0	0.1	10.421	B
C-AB	35	9	486	0.073	35	0.0	0.1	7.985	A
C-A	772	193			772				
A-B	12	3			12				
A-C	643	161			643				

08:45 - 09:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	39	10	336	0.115	39	0.1	0.1	12.109	B
C-AB	42	11	457	0.092	42	0.1	0.1	8.677	A
C-A	922	231			922				
A-B	14	4			14				
A-C	768	192			768				

09:00 - 09:15

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	47	12	269	0.176	47	0.1	0.2	16.201	C
C-AB	52	13	417	0.124	52	0.1	0.1	9.836	A
C-A	1130	282			1130				
A-B	18	4			18				
A-C	940	235			940				

09:15 - 09:30

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	47	12	269	0.176	47	0.2	0.2	16.251	C
C-AB	52	13	417	0.124	52	0.1	0.1	9.844	A
C-A	1130	282			1130				
A-B	18	4			18				
A-C	940	235			940				

09:30 - 09:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	39	10	336	0.115	39	0.2	0.1	12.149	B
C-AB	42	11	457	0.092	42	0.1	0.1	8.688	A
C-A	922	231			922				
A-B	14	4			14				
A-C	768	192			768				

09:45 - 10:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	32	8	377	0.086	33	0.1	0.1	10.454	B
C-AB	35	9	486	0.073	35	0.1	0.1	7.999	A
C-A	772	193			772				
A-B	12	3			12				
A-C	643	161			643				

2033 Base Scenario, PM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.36	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)	Run automatically
D12	2033 Base Scenario	PM	ONE HOUR	16:30	18:00	15	✓

Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)
✓	✓	HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A		ONE HOUR	✓	891	100.000
B		ONE HOUR	✓	30	100.000
C		ONE HOUR	✓	1008	100.000

Origin-Destination Data

Demand (PCU/hr)

		To		
		A	B	C
From	A	0	9	882
	B	2	0	28
	C	973	35	0

Vehicle Mix

Heavy Vehicle Percentages

		To		
		A	B	C
From	A	0	0	0
	B	0	0	0
	C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS	Average Demand (PCU/hr)	Total Junction Arrivals (PCU)
B-AC	0.10	11.70	0.1	B	28	41
C-AB	0.09	9.63	0.1	A	32	48
C-A					893	1339
A-B					8	12
A-C					809	1214

Main Results for each time segment

16:30 - 16:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	23	6	424	0.053	22	0.0	0.1	8.948	A
C-AB	26	7	482	0.055	26	0.0	0.1	7.892	A
C-A	733	183			733				
A-B	7	2			7				
A-C	664	166			664				

16:45 - 17:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	27	7	391	0.069	27	0.1	0.1	9.873	A
C-AB	31	8	453	0.070	31	0.1	0.1	8.543	A
C-A	875	219			875				
A-B	8	2			8				
A-C	793	198			793				

17:00 - 17:15

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	33	8	341	0.097	33	0.1	0.1	11.693	B
C-AB	39	10	412	0.093	38	0.1	0.1	9.627	A
C-A	1071	268			1071				
A-B	10	2			10				
A-C	971	243			971				

17:15 - 17:30

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	33	8	341	0.097	33	0.1	0.1	11.703	B
C-AB	39	10	412	0.093	39	0.1	0.1	9.633	A
C-A	1071	268			1071				
A-B	10	2			10				
A-C	971	243			971				

17:30 - 17:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	27	7	391	0.069	27	0.1	0.1	9.885	A
C-AB	31	8	453	0.070	32	0.1	0.1	8.550	A
C-A	875	219			875				
A-B	8	2			8				
A-C	793	198			793				

17:45 - 18:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	23	6	424	0.053	23	0.1	0.1	8.962	A
C-AB	26	7	482	0.055	26	0.1	0.1	7.904	A
C-A	733	183			733				
A-B	7	2			7				
A-C	664	166			664				

2033 Design Scenario, AM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		1.33	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)	Run automatically
D13	2033 Design Scenario	AM	ONE HOUR	08:30	10:00	15	✓

Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)
✓	✓	HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A		ONE HOUR	✓	875	100.000
B		ONE HOUR	✓	88	100.000
C		ONE HOUR	✓	1084	100.000

Origin-Destination Data

Demand (PCU/hr)

		To		
		A	B	C
From	A	0	21	854
	B	23	0	65
	C	1026	58	0

Vehicle Mix

Heavy Vehicle Percentages

		To		
		A	B	C
From	A	0	0	0
	B	0	0	0
	C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS	Average Demand (PCU/hr)	Total Junction Arrivals (PCU)
B-AC	0.40	24.27	0.6	C	81	121
C-AB	0.15	10.22	0.2	B	53	80
C-A					941	1412
A-B					19	29
A-C					784	1175

Main Results for each time segment

08:30 - 08:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	66	17	360	0.184	65	0.0	0.2	12.187	B
C-AB	44	11	485	0.090	43	0.0	0.1	8.149	A
C-A	772	193			772				
A-B	16	4			16				
A-C	643	161			643				

08:45 - 09:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	79	20	316	0.250	79	0.2	0.3	15.152	C
C-AB	52	13	456	0.114	52	0.1	0.1	8.909	A
C-A	922	231			922				
A-B	19	5			19				
A-C	768	192			768				

09:00 - 09:15

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	97	24	245	0.395	96	0.3	0.6	23.900	C
C-AB	64	16	416	0.153	64	0.1	0.2	10.204	B
C-A	1130	282			1130				
A-B	23	6			23				
A-C	940	235			940				

09:15 - 09:30

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	97	24	245	0.395	97	0.6	0.6	24.266	C
C-AB	64	16	416	0.153	64	0.2	0.2	10.217	B
C-A	1130	282			1130				
A-B	23	6			23				
A-C	940	235			940				

09:30 - 09:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	79	20	316	0.251	80	0.6	0.3	15.364	C
C-AB	52	13	456	0.114	52	0.2	0.1	8.925	A
C-A	922	231			922				
A-B	19	5			19				
A-C	768	192			768				

09:45 - 10:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	66	17	360	0.184	67	0.3	0.2	12.301	B
C-AB	44	11	485	0.090	44	0.1	0.1	8.166	A
C-A	772	193			772				
A-B	16	4			16				
A-C	643	161			643				

2033 Design Scenario, PM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.72	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)	Run automatically
D14	2033 Design Scenario	PM	ONE HOUR	16:30	18:00	15	✓

Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)
✓	✓	HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A		ONE HOUR	✓	903	100.000
B		ONE HOUR	✓	49	100.000
C		ONE HOUR	✓	1035	100.000

Origin-Destination Data

Demand (PCU/hr)

		To		
		A	B	C
From	A	0	21	882
	B	8	0	41
	C	973	62	0

Vehicle Mix

Heavy Vehicle Percentages

		To		
		A	B	C
From	A	0	0	0
	B	0	0	0
	C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS	Average Demand (PCU/hr)	Total Junction Arrivals (PCU)
B-AC	0.19	15.78	0.2	C	45	67
C-AB	0.17	10.56	0.2	B	57	85
C-A					893	1339
A-B					19	29
A-C					809	1214

Main Results for each time segment

16:30 - 16:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	37	9	388	0.095	36	0.0	0.1	10.242	B
C-AB	47	12	480	0.097	46	0.0	0.1	8.294	A
C-A	733	183			733				
A-B	16	4			16				
A-C	664	166			664				

16:45 - 17:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	44	11	347	0.127	44	0.1	0.1	11.857	B
C-AB	56	14	450	0.124	56	0.1	0.1	9.118	A
C-A	875	219			875				
A-B	19	5			19				
A-C	793	198			793				

17:00 - 17:15

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	54	13	282	0.191	54	0.1	0.2	15.749	C
C-AB	68	17	409	0.167	68	0.1	0.2	10.542	B
C-A	1071	268			1071				
A-B	23	6			23				
A-C	971	243			971				

17:15 - 17:30

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	54	13	282	0.191	54	0.2	0.2	15.775	C
C-AB	68	17	409	0.167	68	0.2	0.2	10.556	B
C-A	1071	268			1071				
A-B	23	6			23				
A-C	971	243			971				

17:30 - 17:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	44	11	347	0.127	44	0.2	0.1	11.898	B
C-AB	56	14	450	0.124	56	0.2	0.1	9.134	A
C-A	875	219			875				
A-B	19	5			19				
A-C	793	198			793				

17:45 - 18:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	37	9	388	0.095	37	0.1	0.1	10.273	B
C-AB	47	12	480	0.097	47	0.1	0.1	8.313	A
C-A	733	183			733				
A-B	16	4			16				
A-C	664	166			664				

2033 Design Sensitivity Scenario , AM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		1.39	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)	Run automatically
D15	2033 Design Sensitivity Scenario	AM	ONE HOUR	08:30	10:00	15	✓

Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)
✓	✓	HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A		ONE HOUR	✓	901	100.000
B		ONE HOUR	✓	88	100.000
C		ONE HOUR	✓	1110	100.000

Origin-Destination Data

Demand (PCU/hr)

		To		
		A	B	C
From	A	0	21	880
	B	23	0	65
	C	1052	58	0

Vehicle Mix

Heavy Vehicle Percentages

		To		
		A	B	C
From	A	0	0	0
	B	0	0	0
	C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS	Average Demand (PCU/hr)	Total Junction Arrivals (PCU)
B-AC	0.42	26.39	0.7	D	81	121
C-AB	0.16	10.41	0.2	B	53	80
C-A					965	1448
A-B					19	29
A-C					808	1211

Main Results for each time segment

08:30 - 08:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	66	17	354	0.187	65	0.0	0.2	12.439	B
C-AB	44	11	480	0.091	43	0.0	0.1	8.229	A
C-A	792	198			792				
A-B	16	4			16				
A-C	663	166			663				

08:45 - 09:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	79	20	308	0.257	79	0.2	0.3	15.668	C
C-AB	52	13	451	0.116	52	0.1	0.1	9.026	A
C-A	946	236			946				
A-B	19	5			19				
A-C	791	198			791				

09:00 - 09:15

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	97	24	233	0.416	96	0.3	0.7	25.892	D
C-AB	64	16	410	0.156	64	0.1	0.2	10.395	B
C-A	1158	290			1158				
A-B	23	6			23				
A-C	969	242			969				

09:15 - 09:30

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	97	24	233	0.416	97	0.7	0.7	26.387	D
C-AB	64	16	410	0.156	64	0.2	0.2	10.407	B
C-A	1158	290			1158				
A-B	23	6			23				
A-C	969	242			969				

09:30 - 09:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	79	20	308	0.257	80	0.7	0.4	15.921	C
C-AB	52	13	451	0.116	52	0.2	0.1	9.041	A
C-A	946	236			946				
A-B	19	5			19				
A-C	791	198			791				

09:45 - 10:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	66	17	354	0.187	67	0.4	0.2	12.564	B
C-AB	44	11	480	0.091	44	0.1	0.1	8.250	A
C-A	792	198			792				
A-B	16	4			16				
A-C	663	166			663				

2033 Design Sensitivity Scenario , PM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.72	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)	Run automatically
D16	2033 Design Sensitivity Scenario	PM	ONE HOUR	16:30	18:00	15	✓

Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)
✓	✓	HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A		ONE HOUR	✓	921	100.000
B		ONE HOUR	✓	49	100.000
C		ONE HOUR	✓	1048	100.000

Origin-Destination Data

Demand (PCU/hr)

		To		
		A	B	C
From	A	0	21	900
	B	8	0	41
	C	986	62	0

Vehicle Mix

Heavy Vehicle Percentages

		To		
		A	B	C
From	A	0	0	0
	B	0	0	0
	C	0	0	0

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Queue (PCU)	Max LOS	Average Demand (PCU/hr)	Total Junction Arrivals (PCU)
B-AC	0.20	16.28	0.2	C	45	67
C-AB	0.17	10.70	0.2	B	57	85
C-A					905	1357
A-B					19	29
A-C					826	1239

Main Results for each time segment

16:30 - 16:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	37	9	384	0.096	36	0.0	0.1	10.344	B
C-AB	47	12	477	0.098	46	0.0	0.1	8.349	A
C-A	742	186			742				
A-B	16	4			16				
A-C	678	169			678				

16:45 - 17:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	44	11	343	0.129	44	0.1	0.1	12.042	B
C-AB	56	14	447	0.125	56	0.1	0.1	9.203	A
C-A	886	222			886				
A-B	19	5			19				
A-C	809	202			809				

17:00 - 17:15

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	54	13	275	0.196	54	0.1	0.2	16.223	C
C-AB	68	17	405	0.169	68	0.1	0.2	10.681	B
C-A	1086	271			1086				
A-B	23	6			23				
A-C	991	248			991				

17:15 - 17:30

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	54	13	275	0.196	54	0.2	0.2	16.281	C
C-AB	68	17	405	0.169	68	0.2	0.2	10.696	B
C-A	1086	271			1086				
A-B	23	6			23				
A-C	991	248			991				

17:30 - 17:45

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	44	11	343	0.129	44	0.2	0.1	12.086	B
C-AB	56	14	447	0.125	56	0.2	0.1	9.221	A
C-A	886	222			886				
A-B	19	5			19				
A-C	809	202			809				

17:45 - 18:00

Stream	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	37	9	384	0.096	37	0.1	0.1	10.380	B
C-AB	47	12	477	0.098	47	0.1	0.1	8.374	A
C-A	742	186			742				
A-B	16	4			16				
A-C	678	169			678				

**PICADY Flow Input for
Hay Green Lane / Sheffield Road**

L:\Hay Green Lane, Birdwell - 18039\ANALYSIS\SPREADSHEETS\210119 15 min PICADY Inputs.xlsx\AM Count

Weekday AM Peak Flows - Count

Flow percentage taken from Peak hour total flows

Arms

- A: Sheffield Road North**
- B: Hay Green Lane**
- C: Sheffield Road South**

TOTAL Light Movements per Hour		A	B	C	
	A		14	715	
	B	8		29	
	C	853	40		

TOTAL HGV Movements per Hour		A	B	C	
	A		0	43	
	B	0		1	
	C	53	2		

	A	B	C	D
A		0.0%	6.0%	
B	0.0%		3.4%	
C	6.2%	5.0%		

Light Vehicular Movements by Time Period	08:30-08:45				
		A	B	C	
	A		2	173	
	B	6		12	
C	197	12			

Light Vehicular Movements by Time Period	08:45-09:00				
		A	B	C	
	A		0	214	
	B	0		4	
C	234	4			

Light Vehicular Movements by Time Period	09:00-09:15				
		A	B	C	
	A		3	151	
	B	1		5	
C	223	14			

Light Vehicular Movements by Time Period	09:15-09:30				
		A	B	C	
	A		9	177	
	B	1		8	
C	199	10			

Light Vehicular Proportion by Time Period	08:00-08:15				
		A	B	C	
	A		14.3%	24.2%	
	B	75.0%		41.4%	
C	23.1%	30.0%			

Light Vehicular Proportion by Time Period	08:15-08:30				
		A	B	C	
	A		0.0%	29.9%	
	B	0.0%		13.8%	
C	27.4%	10.0%			

Light Vehicular Proportion by Time Period	08:30-08:45				
		A	B	C	
	A		21.4%	21.1%	
	B	12.5%		17.2%	
C	26.1%	35.0%			

Light Vehicular Proportion by Time Period	08:45-09:00				
		A	B	C	
	A		64.3%	24.8%	
	B	12.5%		27.6%	
C	23.3%	25.0%			

HGV Movements by Time Period	08:30-08:45				
		A	B	C	
	A		0	18	
	B	0		0	
C	12	0			

HGV Movements by Time Period	08:45-09:00				
		A	B	C	
	A		0	11	
	B	0		0	
C	11	1			

HGV Movements by Time Period	09:00-09:15				
		A	B	C	
	A		0	9	
	B	0		1	
C	16	0			

HGV Movements by Time Period	09:15-09:30				
		A	B	C	
	A		0	5	
	B	0		0	
C	14	1			
D					

HGV Proportion by Time Period	08:30-08:45				
		A	B	C	
	A		0%	42%	
	B	0%		0%	
C	23%	0%			

HGV Proportion by Time Period	08:45-09:00				
		A	B	C	
	A		0%	26%	
	B	0%		0%	
C	21%	50%			

HGV Proportion by Time Period	09:00-09:15				
		A	B	C	
	A		0%	21%	
	B	0%		100%	
C	30%	0%			
D					

HGV Proportion by Time Period	09:15-09:30				
		A	B	C	
	A		0%	12%	
	B	0%		0%	
C	26%	50%			

TOTAL HGV Percentage of LGV by Time Period	08:30-08:45				
		A	B	C	
	A		0%	10%	
	B	0%		0%	
C	6%	0%			

TOTAL HGV Percentage of LGV by Time Period	08:45-09:00				
		A	B	C	
	A		0%	5%	
	B	0%		0%	
C	5%	25%			
D					

TOTAL HGV Percentage of LGV by Time Period	09:00-09:15				
		A	B	C	
	A		0%	6%	
	B	0%		20%	
C	7%	0%			

TOTAL HGV Percentage of LGV by Time Period	09:15-09:30				
		A	B	C	
	A		0%	3%	
	B	0%		0%	
C	7%	10%			

**PICADY Flow Input for
Hay Green Lane / Sheffield Road**

L:\Hay Green Lane, Birdwell - 18039\ANALYSIS\SPREADSHEETS\210119 15 min PICADY Inputs.xlsx\AM Count

Weekday PM Peak Flows - Count

Flow percentage taken from Peak hour total flows

Arms

- A: Sheffield Road North**
- B: Hay Green Lane**
- C: Sheffield Road South**

TOTAL Light Movements per Hour		A	B	C	
	A		8	757	
	B	3		25	
	C	840	31		

TOTAL HGV Movements per Hour		A	B	C	
	A		0	25	
	B	0		0	
	C	22	0		

	A	B	C	D
A		0.0%	3.3%	
B	0.0%		0.0%	
C	2.6%	0.0%		

Light Vehicular Movements by Time Period	16:30-16:45				
		A	B	C	
	A		4	199	
	B	1		8	
C	200	7			

Light Vehicular Movements by Time Period	16:45-17:00				
		A	B	C	
	A		1	187	
	B	1		6	
C	215	8			

Light Vehicular Movements by Time Period	17:00-17:15				
		A	B	C	
	A		1	190	
	B	1		6	
C	224	7			

Light Vehicular Movements by Time Period	17:15-17:30				
		A	B	C	
	A		2	181	
	B	0		5	
C	201	9			

Light Vehicular Proportion by Time Period	08:00-08:15				
		A	B	C	
	A		50.0%	26.3%	
	B	33.3%		32.0%	
C	23.8%	22.6%			

Light Vehicular Proportion by Time Period	08:15-08:30				
		A	B	C	
	A		12.5%	24.7%	
	B	33.3%		24.0%	
C	25.6%	25.8%			

Light Vehicular Proportion by Time Period	08:30-08:45				
		A	B	C	
	A		12.5%	25.1%	
	B	33.3%		24.0%	
C	26.7%	22.6%			

Light Vehicular Proportion by Time Period	08:45-09:00				
		A	B	C	
	A		25.0%	23.9%	
	B	0.0%		20.0%	
C	23.9%	29.0%			

HGV Movements by Time Period	16:30-16:45				
		A	B	C	
	A		0	10	
	B	0		0	
C	5	0			

HGV Movements by Time Period	16:45-17:00				
		A	B	C	
	A		0	7	
	B	0		0	
C	6	0			

HGV Movements by Time Period	17:00-17:15				
		A	B	C	
	A		0	4	
	B	0		0	
C	4	0			

HGV Movements by Time Period	17:15-17:30				
		A	B	C	
	A		0	4	
	B	0		0	
C	7	0			
D					

HGV Proportion by Time Period	16:30-16:45				
		A	B	C	
	A		0%	40%	
	B	0%		0%	
C	23%	0%			

HGV Proportion by Time Period	16:45-17:00				
		A	B	C	
	A		0%	28%	
	B	0%		0%	
C	27%	0%			

HGV Proportion by Time Period	17:00-17:15				
		A	B	C	
	A		0%	16%	
	B	0%		0%	
C	18%	0%			

HGV Proportion by Time Period	17:15-17:30				
		A	B	C	
	A		0%	16%	
	B	0%		0%	
C	32%	0%			

TOTAL HGV Percentage of LGV by Time Period	16:30-16:45				
		A	B	C	
	A		0%	5%	
	B	0%		0%	
C	3%	0%			

TOTAL HGV Percentage of LGV by Time Period	16:45-17:00				
		A	B	C	
	A		0%	4%	
	B	0%		0%	
C	3%	0%			
D					

TOTAL HGV Percentage of LGV by Time Period	17:00-17:15				
		A	B	C	
	A		0%	2%	
	B	0%		0%	
C	2%	0%			

TOTAL HGV Percentage of LGV by Time Period	17:15-17:30				
		A	B	C	
	A		0%	2%	
	B	0%		0%	
C	3%	0%			

**PICADY Flow Input for
Hay Green Lane / Sheffield Road**

L:\Hay Green Lane, Birdwell - 18039\ANALYSIS\SPREADSHEETS\210119 15 min PICADY Inputs.xlsx\AM Count

Weekday AM Peak Flows - 2019 Base

Flow percentage taken from Peak hour total flows

TOTAL Light Movements per Hour		A	B	C	
	A		14	759	
	B	8		30	
	C	912	42		

Arms
A: Sheffield Road North
B: Hay Green Lane
C: Sheffield Road South
D:

TOTAL HGV Movements per Hour		A	B	C	
	A		0	53	
	B	0		1	
	C	65	2		

	A	B	C	
A		0.0%	7.0%	
B	0.0%		3.3%	
C	7.1%	4.8%		

TOTAL Vehicular Movements by Time Period	08:30-08:45				
		A	B	C	
	A		2	184	
	B	6		12	
C	211	13			

08:45-09:00				
	A	B	C	
A		0	227	
B	0		4	
C	250	4		

09:00-09:15				
	A	B	C	
A		3	160	
B	1		5	
C	238	15		

09:15-09:30				
	A	B	C	
A		9	188	
B	1		8	
C	213	11		

TOTAL HGV Movements by Time Period	08:30-08:45				
		A	B	C	
	A		0	22	
	B	0		0	
C	15	0			

08:45-09:00				
	A	B	C	
A		0	14	
B	0		0	
C	13	1		

09:00-09:15				
	A	B	C	
A		0	11	
B	0		1	
C	20	0		

09:15-09:30				
	A	B	C	
A		0	6	
B	0		0	
C	17	1		

TOTAL HGV Percentage by Time Period	08:30-08:45				
		A	B	C	
	A		0%	12%	
	B	0%		0%	
C	7%	0%			

08:45-09:00				
	A	B	C	D
A		0%	6%	
B	0%		0%	
C	5%	24%		

09:00-09:15				
	A	B	C	D
A		0%	7%	
B	0%		19%	
C	8%	0%		

09:15-09:30				
	A	B	C	
A		0%	3%	
B	0%		0%	
C	8%	10%		

PICADY Flow Input for

Hay Green Lane / Sheffield Road

L:\Hay Green Lane, Birdwell - 18039\ANALYSIS\SPREADSHEETS\210119 15 min PICADY Inputs.xlsx\AM Count

Weekday PM Peak Flows - 2019 Base

Flow percentage taken from Peak hour total flows

TOTAL Light Movements per Hour		A	B	C	
	A		8	783	
	B	2		25	
	C	864	31		

Arms

- A: Sheffield Road North
- B: Hay Green Lane
- C: Sheffield Road South
- D:

TOTAL HGV Movements per Hour		A	B	C	
	A		0	37	
	B	0		0	
	C	32	0		

	A	B	C	
A		0.0%	4.7%	
B	0.0%		0.0%	
C	3.7%	0.0%		

TOTAL Vehicular Movements by Time Period	16:30-16:45				
		A	B	C	
	A		4	206	
	B	1		8	
C	206	7			

16:45-17:00				
	A	B	C	
A		1	193	
B	1		6	
C	221	8		

17:00-17:15				
	A	B	C	
A		1	197	
B	1		6	
C	230	7		

17:15-17:30				
	A	B	C	
A		2	187	
B	0		5	
C	207	9		

TOTAL HGV Movements by Time Period	16:30-16:45				
		A	B	C	
	A		0	15	
	B	0		0	
C	7	0			

16:45-17:00				
	A	B	C	
A		0	10	
B	0		0	
C	9	0		

17:00-17:15				
	A	B	C	
A		0	6	
B	0		0	
C	6	0		

17:15-17:30				
	A	B	C	
A		0	6	
B	0		0	
C	10	0		

TOTAL HGV Percentage by Time Period	16:30-16:45				
		A	B	C	
	A		0%	7%	
	B	0%		0%	
C	4%	0%			

16:45-17:00				
	A	B	C	D
A		0%	5%	
B	0%		0%	
C	4%	0%		

17:00-17:15				
	A	B	C	D
A		0%	3%	
B	0%		0%	
C	3%	0%		

17:15-17:30				
	A	B	C	
A		0%	3%	
B	0%		0%	
C	5%	0%		

**PICADY Flow Input for
Hay Green Lane / Sheffield Road**

L:\Hay Green Lane, Birdwell - 18039\ANALYSIS\SPREADSHEETS\210119 15 min PICADY Inputs.xlsx\AM Count

Weekday AM Peak Flows - 2025 Base

Flow percentage taken from Peak hour total flows

TOTAL Light Movements per Hour		A	B	C	
	A		15	803	
	B	8		32	
	C	965	44		

Arms

- A: Sheffield Road North
- B: Hay Green Lane
- C: Sheffield Road South
- D:

TOTAL HGV Movements per Hour		A	B	C	
	A		0	56	
	B	0		1	
	C	69	2		

	A	B	C	
A		0.0%	7.0%	
B	0.0%		3.1%	
C	7.2%	4.5%		

TOTAL Vehicular Movements by Time Period	08:30-08:45				
		A	B	C	
	A		2	194	
	B	6		13	
C	223	13			

08:45-09:00				
	A	B	C	
A		0	240	
B	0		4	
C	265	4		

09:00-09:15				
	A	B	C	
A		3	170	
B	1		6	
C	252	15		

09:15-09:30				
	A	B	C	
A		10	199	
B	1		9	
C	225	11		

TOTAL HGV Movements by Time Period	08:30-08:45				
		A	B	C	
	A		0	23	
	B	0		0	
C	16	0			

08:45-09:00				
	A	B	C	
A		0	14	
B	0		0	
C	14	1		

09:00-09:15				
	A	B	C	
A		0	12	
B	0		1	
C	21	0		

09:15-09:30				
	A	B	C	
A		0	7	
B	0		0	
C	18	1		

TOTAL HGV Percentage by Time Period	08:30-08:45				
		A	B	C	
	A		0%	12%	
	B	0%		0%	
C	7%	0%			

08:45-09:00				
	A	B	C	D
A		0%	6%	
B	0%		0%	
C	5%	23%		

09:00-09:15				
	A	B	C	D
A		0%	7%	
B	0%		18%	
C	8%	0%		

09:15-09:30				
	A	B	C	
A		0%	3%	
B	0%		0%	
C	8%	9%		

PICADY Flow Input for

Hay Green Lane / Sheffield Road

L:\Hay Green Lane, Birdwell - 18039\ANALYSIS\SPREADSHEETS\210119 15 min PICADY Inputs.xlsx\AM Count

Weekday PM Peak Flows - 2025 Base

Flow percentage taken from Peak hour total flows

TOTAL Light Movements per Hour		A	B	C	
	A		8	828	
	B	2		26	
	C	914	33		

Arms

- A: Sheffield Road North
- B: Hay Green Lane
- C: Sheffield Road South
- D:

TOTAL HGV Movements per Hour		A	B	C	
	A		0	39	
	B	0		0	
	C	34	0		

	A	B	C	
A		0.0%	4.7%	
B	0.0%		0.0%	
C	3.7%	0.0%		

TOTAL Vehicular Movements by Time Period	16:30-16:45				
		A	B	C	
	A		4	218	
	B	1		8	
C	218	7			

16:45-17:00				
	A	B	C	
A		1	205	
B	1		6	
C	234	9		

17:00-17:15				
	A	B	C	
A		1	208	
B	1		6	
C	244	7		

17:15-17:30				
	A	B	C	
A		2	198	
B	0		5	
C	219	10		

TOTAL HGV Movements by Time Period	16:30-16:45				
		A	B	C	
	A		0	16	
	B	0		0	
C	8	0			

16:45-17:00				
	A	B	C	
A		0	11	
B	0		0	
C	9	0		

17:00-17:15				
	A	B	C	
A		0	6	
B	0		0	
C	6	0		

17:15-17:30				
	A	B	C	
A		0	6	
B	0		0	
C	11	0		

TOTAL HGV Percentage by Time Period	16:30-16:45				
		A	B	C	
	A		0%	7%	
	B	0%		0%	
C	4%	0%			

16:45-17:00				
	A	B	C	D
A		0%	5%	
B	0%		0%	
C	4%	0%		

17:00-17:15				
	A	B	C	D
A		0%	3%	
B	0%		0%	
C	3%	0%		

17:15-17:30				
	A	B	C	
A		0%	3%	
B	0%		0%	
C	5%	0%		

PICADY Flow Input for

Hay Green Lane / Sheffield Road

L:\Hay Green Lane, Birdwell - 18039\ANALYSIS\SPREADSHEETS\210119 15 min PICADY Inputs.xlsx\AM Count

Weekday AM Peak Flows - 2033 Base

Flow percentage taken from Peak hour total flows

TOTAL Light Movements per Hour		A	B	C	
	A		16	854	
	B	9		34	
	C	1026	47		

Arms

- A: Sheffield Road North
- B: Hay Green Lane
- C: Sheffield Road South
- D:

TOTAL HGV Movements per Hour		A	B	C	
	A		0	60	
	B	0		1	
	C	73	2		

	A	B	C	
A		0.0%	7.0%	
B	0.0%		2.9%	
C	7.1%	4.3%		

TOTAL Vehicular Movements by Time Period	08:30-08:45				
		A	B	C	
	A		2	207	
	B	7		14	
C	237	14			

08:45-09:00				
	A	B	C	
A		0	256	
B	0		5	
C	281	5		

09:00-09:15				
	A	B	C	
A		3	180	
B	1		6	
C	268	16		

09:15-09:30				
	A	B	C	
A		10	211	
B	1		9	
C	239	12		

TOTAL HGV Movements by Time Period	08:30-08:45				
		A	B	C	
	A		0	25	
	B	0		0	
C	17	0			

08:45-09:00				
	A	B	C	
A		0	15	
B	0		0	
C	15	1		

09:00-09:15				
	A	B	C	
A		0	13	
B	0		1	
C	22	0		

09:15-09:30				
	A	B	C	
A		0	7	
B	0		0	
C	19	1		

TOTAL HGV Percentage by Time Period	08:30-08:45				
		A	B	C	
	A		0%	12%	
	B	0%		0%	
C	7%	0%			

08:45-09:00				
	A	B	C	D
A		0%	6%	
B	0%		0%	
C	5%	21%		

09:00-09:15				
	A	B	C	D
A		0%	7%	
B	0%		17%	
C	8%	0%		

09:15-09:30				
	A	B	C	
A		0%	3%	
B	0%		0%	
C	8%	9%		

PICADY Flow Input for

Hay Green Lane / Sheffield Road

L:\Hay Green Lane, Birdwell - 18039\ANALYSIS\SPREADSHEETS\210119 15 min PICADY Inputs.xlsx\AM Count

Weekday PM Peak Flows - 2033 Base

Flow percentage taken from Peak hour total flows

TOTAL Light Movements per Hour		A	B	C	
	A		9	882	
	B	2		28	
	C	973	35		

Arms

- A: Sheffield Road North
- B: Hay Green Lane
- C: Sheffield Road South
- D:

TOTAL HGV Movements per Hour		A	B	C	
	A		0	42	
	B	0		0	
	C	36	0		

	A	B	C	
A		0.0%	4.8%	
B	0.0%		0.0%	
C	3.7%	0.0%		

TOTAL Vehicular Movements by Time Period	16:30-16:45				
		A	B	C	
	A		5	232	
	B	1		9	
C	232	8			

16:45-17:00				
	A	B	C	
A		1	218	
B	1		7	
C	249	9		

17:00-17:15				
	A	B	C	
A		1	221	
B	1		7	
C	259	8		

17:15-17:30				
	A	B	C	
A		2	211	
B	0		6	
C	233	10		

TOTAL HGV Movements by Time Period	16:30-16:45				
		A	B	C	
	A		0	17	
	B	0		0	
C	8	0			

16:45-17:00				
	A	B	C	
A		0	12	
B	0		0	
C	10	0		

17:00-17:15				
	A	B	C	
A		0	7	
B	0		0	
C	7	0		

17:15-17:30				
	A	B	C	
A		0	7	
B	0		0	
C	11	0		

TOTAL HGV Percentage by Time Period	16:30-16:45				
		A	B	C	
	A		0%	7%	
	B	0%		0%	
C	4%	0%			

16:45-17:00				
	A	B	C	D
A		0%	5%	
B	0%		0%	
C	4%	0%		

17:00-17:15				
	A	B	C	D
A		0%	3%	
B	0%		0%	
C	3%	0%		

17:15-17:30				
	A	B	C	
A		0%	3%	
B	0%		0%	
C	5%	0%		

PICADY Flow Input for

Hay Green Lane / Sheffield Road

L:\Hay Green Lane, Birdwell - 18039\ANALYSIS\SPREADSHEETS\210119 15 min PICADY Inputs.xlsx\AM Count

Weekday AM Peak Flows - 2025 Design

Flow percentage taken from Peak hour total flows

TOTAL Light Movements per Hour		A	B	C	
	A		20	803	
	B	22		63	
	C	965	55		

Arms

- A: Sheffield Road North
- B: Hay Green Lane
- C: Sheffield Road South
- D:

TOTAL HGV Movements per Hour		A	B	C	
	A		0	56	
	B	0		1	
	C	69	2		

	A	B	C	
A		0.0%	7.0%	
B	0.0%		1.6%	
C	7.2%	3.6%		

TOTAL Vehicular Movements by Time Period	08:30-08:45				
		A	B	C	
	A		3	194	
	B	17		26	
C	223	17			

08:45-09:00				
	A	B	C	
A		0	240	
B	0		9	
C	265	6		

09:00-09:15				
	A	B	C	
A		4	170	
B	3		11	
C	252	19		

09:15-09:30				
	A	B	C	
A		13	199	
B	3		17	
C	225	14		

TOTAL HGV Movements by Time Period	08:30-08:45				
		A	B	C	
	A		0	23	
	B	0		0	
C	16	0			

08:45-09:00				
	A	B	C	
A		0	14	
B	0		0	
C	14	1		

09:00-09:15				
	A	B	C	
A		0	12	
B	0		1	
C	21	0		

09:15-09:30				
	A	B	C	
A		0	7	
B	0		0	
C	18	1		

TOTAL HGV Percentage by Time Period	08:30-08:45				
		A	B	C	
	A		0%	12%	
	B	0%		0%	
C	7%	0%			

08:45-09:00				
	A	B	C	D
A		0%	6%	
B	0%		0%	
C	5%	18%		

09:00-09:15				
	A	B	C	D
A		0%	7%	
B	0%		9%	
C	8%	0%		

09:15-09:30				
	A	B	C	
A		0%	3%	
B	0%		0%	
C	8%	7%		

PICADY Flow Input for

Hay Green Lane / Sheffield Road

L:\Hay Green Lane, Birdwell - 18039\ANALYSIS\SPREADSHEETS\210119 15 min PICADY Inputs.xlsx\AM Count

Weekday PM Peak Flows - 2025 Design

Flow percentage taken from Peak hour total flows

Arms

- A: Sheffield Road North**
- B: Hay Green Lane**
- C: Sheffield Road South**
- D:**

TOTAL Light Movements per Hour		A	B	C	
	A		20	828	
	B	8		40	
	C	914	60		

TOTAL HGV Movements per Hour		A	B	C	
	A		0	39	
	B	0		0	
	C	34	0		

	A	B	C	
A		0.0%	4.7%	
B	0.0%		0.0%	
C	3.7%	0.0%		

TOTAL Vehicular Movements by Time Period	16:30-16:45				
		A	B	C	
	A		10	218	
	B	3		13	
C	218	14			

16:45-17:00				
	A	B	C	
A		3	205	
B	3		10	
C	234	15		

17:00-17:15				
	A	B	C	
A		3	208	
B	3		10	
C	244	14		

17:15-17:30				
	A	B	C	
A		5	198	
B	0		8	
C	219	17		

TOTAL HGV Movements by Time Period	16:30-16:45				
		A	B	C	
	A		0	16	
	B	0		0	
C	8	0			

16:45-17:00				
	A	B	C	
A		0	11	
B	0		0	
C	9	0		

17:00-17:15				
	A	B	C	
A		0	6	
B	0		0	
C	6	0		

17:15-17:30				
	A	B	C	
A		0	6	
B	0		0	
C	11	0		

TOTAL HGV Percentage by Time Period	16:30-16:45				
		A	B	C	
	A		0%	7%	
	B	0%		0%	
C	4%	0%			

16:45-17:00				
	A	B	C	D
A		0%	5%	
B	0%		0%	
C	4%	0%		

17:00-17:15				
	A	B	C	D
A		0%	3%	
B	0%		0%	
C	3%	0%		

17:15-17:30				
	A	B	C	
A		0%	3%	
B	0%		0%	
C	5%	0%		

PICADY Flow Input for

Hay Green Lane / Sheffield Road

L:\Hay Green Lane, Birdwell - 18039\ANALYSIS\SPREADSHEETS\210119 15 min PICADY Inputs.xlsx\AM Count

Weekday AM Peak Flows - 2025 Design Sensitivity

Flow percentage taken from Peak hour total flows

TOTAL Light Movements per Hour		A	B	C	
	A		20	829	
	B	22		63	
	C	991	55		

Arms

- A: Sheffield Road North
- B: Hay Green Lane
- C: Sheffield Road South
- D:

TOTAL HGV Movements per Hour		A	B	C	
	A		0	56	
	B	0		1	
	C	69	2		

	A	B	C	
A		0.0%	6.8%	
B	0.0%		1.6%	
C	7.0%	3.6%		

TOTAL Vehicular Movements by Time Period	08:30-08:45				
		A	B	C	
	A		3	201	
	B	17		26	
C	229	17			

08:45-09:00				
	A	B	C	
A		0	248	
B	0		9	
C	272	6		

09:00-09:15				
	A	B	C	
A		4	175	
B	3		11	
C	259	19		

09:15-09:30				
	A	B	C	
A		13	205	
B	3		17	
C	231	14		

TOTAL HGV Movements by Time Period	08:30-08:45				
		A	B	C	
	A		0	23	
	B	0		0	
C	16	0			

08:45-09:00				
	A	B	C	
A		0	14	
B	0		0	
C	14	1		

09:00-09:15				
	A	B	C	
A		0	12	
B	0		1	
C	21	0		

09:15-09:30				
	A	B	C	
A		0	7	
B	0		0	
C	18	1		

TOTAL HGV Percentage by Time Period	08:30-08:45				
		A	B	C	
	A		0%	12%	
	B	0%		0%	
C	7%	0%			

08:45-09:00				
	A	B	C	D
A		0%	6%	
B	0%		0%	
C	5%	18%		

09:00-09:15				
	A	B	C	D
A		0%	7%	
B	0%		9%	
C	8%	0%		

09:15-09:30				
	A	B	C	
A		0%	3%	
B	0%		0%	
C	8%	7%		

PICADY Flow Input for

Hay Green Lane / Sheffield Road

L:\Hay Green Lane, Birdwell - 18039\ANALYSIS\SPREADSHEETS\210119 15 min PICADY Inputs.xlsx\AM Count

Weekday PM Peak Flows - 2025 Design Sensitivity

Flow percentage taken from Peak hour total flows

TOTAL Light Movements per Hour		A	B	C	
	A		20	846	
	B	8		40	
	C	927	60		

Arms

- A: Sheffield Road North
- B: Hay Green Lane
- C: Sheffield Road South
- D:

TOTAL HGV Movements per Hour		A	B	C	
	A		0	39	
	B	0		0	
	C	34	0		

	A	B	C	
A		0.0%	4.6%	
B	0.0%		0.0%	
C	3.7%	0.0%		

TOTAL Vehicular Movements by Time Period	16:30-16:45				
		A	B	C	
	A		10	222	
	B	3		13	
C	221	14			

16:45-17:00				
	A	B	C	
A		3	209	
B	3		10	
C	237	15		

17:00-17:15				
	A	B	C	
A		3	212	
B	3		10	
C	247	14		

17:15-17:30				
	A	B	C	
A		5	202	
B	0		8	
C	222	17		

TOTAL HGV Movements by Time Period	16:30-16:45				
		A	B	C	
	A		0	16	
	B	0		0	
C	8	0			

16:45-17:00				
	A	B	C	
A		0	11	
B	0		0	
C	9	0		

17:00-17:15				
	A	B	C	
A		0	6	
B	0		0	
C	6	0		

17:15-17:30				
	A	B	C	
A		0	6	
B	0		0	
C	11	0		

TOTAL HGV Percentage by Time Period	16:30-16:45				
		A	B	C	
	A		0%	7%	
	B	0%		0%	
C	4%	0%			

16:45-17:00				
	A	B	C	D
A		0%	5%	
B	0%		0%	
C	4%	0%		

17:00-17:15				
	A	B	C	D
A		0%	3%	
B	0%		0%	
C	3%	0%		

17:15-17:30				
	A	B	C	
A		0%	3%	
B	0%		0%	
C	5%	0%		

PICADY Flow Input for

Hay Green Lane / Sheffield Road

L:\Hay Green Lane, Birdwell - 18039\ANALYSIS\SPREADSHEETS\210119 15 min PICADY Inputs.xlsx\AM Count

Weekday AM Peak Flows - 2033 Design

Flow percentage taken from Peak hour total flows

TOTAL Light Movements per Hour		A	B	C	
	A		21	854	
	B	23		65	
	C	1026	58		

Arms

- A: Sheffield Road North
- B: Hay Green Lane
- C: Sheffield Road South
- D:

TOTAL HGV Movements per Hour		A	B	C	
	A		0	60	
	B	0		1	
	C	73	2		

	A	B	C	
A		0.0%	7.0%	
B	0.0%		1.5%	
C	7.1%	3.4%		

TOTAL Vehicular Movements by Time Period	08:30-08:45				
		A	B	C	
	A		3	207	
	B	17		27	
C	237	17			

08:45-09:00				
	A	B	C	
A		0	256	
B	0		9	
C	281	6		

09:00-09:15				
	A	B	C	
A		5	180	
B	3		11	
C	268	20		

09:15-09:30				
	A	B	C	
A		14	211	
B	3		18	
C	239	15		

TOTAL HGV Movements by Time Period	08:30-08:45				
		A	B	C	
	A		0	25	
	B	0		0	
C	17	0			

08:45-09:00				
	A	B	C	
A		0	15	
B	0		0	
C	15	1		

09:00-09:15				
	A	B	C	
A		0	13	
B	0		1	
C	22	0		

09:15-09:30				
	A	B	C	
A		0	7	
B	0		0	
C	19	1		

TOTAL HGV Percentage by Time Period	08:30-08:45				
		A	B	C	
	A		0%	12%	
	B	0%		0%	
C	7%	0%			

08:45-09:00				
	A	B	C	D
A		0%	6%	
B	0%		0%	
C	5%	17%		

09:00-09:15				
	A	B	C	D
A		0%	7%	
B	0%		9%	
C	8%	0%		

09:15-09:30				
	A	B	C	
A		0%	3%	
B	0%		0%	
C	8%	7%		

PICADY Flow Input for

Hay Green Lane / Sheffield Road

L:\Hay Green Lane, Birdwell - 18039\ANALYSIS\SPREADSHEETS\210119 15 min PICADY Inputs.xlsx\AM Count

Weekday PM Peak Flows - 2033 Design

Flow percentage taken from Peak hour total flows

TOTAL Light Movements per Hour		A	B	C	
	A		21	882	
	B	8		41	
	C	973	62		

Arms

- A: Sheffield Road North
- B: Hay Green Lane
- C: Sheffield Road South
- D:

TOTAL HGV Movements per Hour		A	B	C	
	A		0	42	
	B	0		0	
	C	36	0		

	A	B	C	
A		0.0%	4.8%	
B	0.0%		0.0%	
C	3.7%	0.0%		

TOTAL Vehicular Movements by Time Period	16:30-16:45				
		A	B	C	
	A		11	232	
	B	3		13	
C	232	14			

16:45-17:00				
	A	B	C	
A		3	218	
B	3		10	
C	249	16		

17:00-17:15				
	A	B	C	
A		3	221	
B	3		10	
C	259	14		

17:15-17:30				
	A	B	C	
A		5	211	
B	0		8	
C	233	18		

TOTAL HGV Movements by Time Period	16:30-16:45				
		A	B	C	
	A		0	17	
	B	0		0	
C	8	0			

16:45-17:00				
	A	B	C	
A		0	12	
B	0		0	
C	10	0		

17:00-17:15				
	A	B	C	
A		0	7	
B	0		0	
C	7	0		

17:15-17:30				
	A	B	C	
A		0	7	
B	0		0	
C	11	0		

TOTAL HGV Percentage by Time Period	16:30-16:45				
		A	B	C	
	A		0%	7%	
	B	0%		0%	
C	4%	0%			

16:45-17:00				
	A	B	C	D
A		0%	5%	
B	0%		0%	
C	4%	0%		

17:00-17:15				
	A	B	C	D
A		0%	3%	
B	0%		0%	
C	3%	0%		

17:15-17:30				
	A	B	C	
A		0%	3%	
B	0%		0%	
C	5%	0%		

PICADY Flow Input for

Hay Green Lane / Sheffield Road

L:\Hay Green Lane, Birdwell - 18039\ANALYSIS\SPREADSHEETS\210119 15 min PICADY Inputs.xlsx\AM Count

Weekday AM Peak Flows - 2033 Design Sensitivity

Flow percentage taken from Peak hour total flows

TOTAL Light Movements per Hour		A	B	C	
	A		21	880	
	B	23		65	
	C	1052	58		

Arms

- A: Sheffield Road North
- B: Hay Green Lane
- C: Sheffield Road South
- D:

TOTAL HGV Movements per Hour		A	B	C	
	A		0	60	
	B	0		1	
	C	73	2		

	A	B	C	
A		0.0%	6.8%	
B	0.0%		1.5%	
C	6.9%	3.4%		

TOTAL Vehicular Movements by Time Period	08:30-08:45				
		A	B	C	
	A		3	213	
	B	17		27	
C	243	17			

08:45-09:00				
	A	B	C	
A		0	263	
B	0		9	
C	289	6		

09:00-09:15				
	A	B	C	
A		5	186	
B	3		11	
C	275	20		

09:15-09:30				
	A	B	C	
A		14	218	
B	3		18	
C	245	15		

TOTAL HGV Movements by Time Period	08:30-08:45				
		A	B	C	
	A		0	25	
	B	0		0	
C	17	0			

08:45-09:00				
	A	B	C	
A		0	15	
B	0		0	
C	15	1		

09:00-09:15				
	A	B	C	
A		0	13	
B	0		1	
C	22	0		

09:15-09:30				
	A	B	C	
A		0	7	
B	0		0	
C	19	1		

TOTAL HGV Percentage by Time Period	08:30-08:45				
		A	B	C	
	A		0%	12%	
	B	0%		0%	
C	7%	0%			

08:45-09:00				
	A	B	C	D
A		0%	6%	
B	0%		0%	
C	5%	17%		

09:00-09:15				
	A	B	C	D
A		0%	7%	
B	0%		9%	
C	8%	0%		

09:15-09:30				
	A	B	C	
A		0%	3%	
B	0%		0%	
C	8%	7%		

PICADY Flow Input for

Hay Green Lane / Sheffield Road

L:\Hay Green Lane, Birdwell - 18039\ANALYSIS\SPREADSHEETS\210119 15 min PICADY Inputs.xlsx\AM Count

Weekday PM Peak Flows - 2033 Design Sensitivity

Flow percentage taken from Peak hour total flows

Arms

- A: Sheffield Road North**
- B: Hay Green Lane**
- C: Sheffield Road South**
- D:**

TOTAL Light Movements per Hour		A	B	C	
	A		21	900	
	B	8		41	
	C	986	62		

TOTAL HGV Movements per Hour		A	B	C	
	A		0	42	
	B	0		0	
	C	36	0		

	A	B	C	
A		0.0%	4.7%	
B	0.0%		0.0%	
C	3.7%	0.0%		

TOTAL Vehicular Movements by Time Period	16:30-16:45				
		A	B	C	
	A		11	237	
	B	3		13	
C	235	14			

16:45-17:00				
	A	B	C	
A		3	222	
B	3		10	
C	252	16		

17:00-17:15				
	A	B	C	
A		3	226	
B	3		10	
C	263	14		

17:15-17:30				
	A	B	C	
A		5	215	
B	0		8	
C	236	18		

TOTAL HGV Movements by Time Period	16:30-16:45				
		A	B	C	
	A		0	17	
	B	0		0	
C	8	0			

16:45-17:00				
	A	B	C	
A		0	12	
B	0		0	
C	10	0		

17:00-17:15				
	A	B	C	
A		0	7	
B	0		0	
C	7	0		

17:15-17:30				
	A	B	C	
A		0	7	
B	0		0	
C	11	0		

TOTAL HGV Percentage by Time Period	16:30-16:45				
		A	B	C	
	A		0%	7%	
	B	0%		0%	
C	3%	0%			

16:45-17:00				
	A	B	C	D
A		0%	5%	
B	0%		0%	
C	4%	0%		

17:00-17:15				
	A	B	C	D
A		0%	3%	
B	0%		0%	
C	2%	0%		

17:15-17:30				
	A	B	C	
A		0%	3%	
B	0%		0%	
C	5%	0%		

Junctions 9

PICADY 9 - Priority Intersection Module

Version: 9.5.0.6896
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Filename: 210118 Hay Green Lane - Direct Input.j9

Path: L:\Hay Green Lane, Birdwell - 18039\ANALYSIS\CAPACITY\Priority Junctions

Report generation date: 19/01/2021 19:36:04

- »2019 Count, AM
- »2019 Count, PM
- »2019 Base, AM
- »2019 Base, PM
- »2025 Base, AM
- »2025 Base, PM
- »2033 Base, AM
- »2033 Base, PM
- »2025 Design, AM
- »2025 Design, PM
- »2025 Design Sensitivity, AM
- »2025 Design Sensitivity, PM
- »2033 Design, AM
- »2033 Design, PM
- »2033 Design Sensitivity, AM
- »2033 Design Sensitivity, PM

Summary of junction performance

	AM			PM		
	Q (PCU)	Delay (s)	RFC	Q (PCU)	Delay (s)	RFC
2019 Count						
Stream B-AC	0.3	13.97	0.22	0.1	10.62	0.10
Stream C-AB	0.1	8.89	0.11	0.1	8.52	0.08
2019 Base						
Stream B-AC	0.3	14.91	0.23	0.1	10.88	0.10
Stream C-AB	0.1	9.10	0.12	0.1	8.64	0.08
2025 Base						
Stream B-AC	0.3	15.77	0.25	0.1	11.36	0.10
Stream C-AB	0.1	9.35	0.13	0.1	8.87	0.09
2033 Base						
Stream B-AC	0.4	18.30	0.30	0.1	11.89	0.12
Stream C-AB	0.2	9.81	0.14	0.1	9.27	0.09
2025 Design						
Stream B-AC	1.5	31.14	0.62	0.2	14.23	0.20
Stream C-AB	0.2	9.55	0.16	0.2	9.65	0.15
2025 Design Sensitivity						
Stream B-AC	1.6	33.63	0.64	0.3	14.53	0.20
Stream C-AB	0.2	9.68	0.16	0.2	9.75	0.15
2033 Design						
Stream B-AC	1.8	37.03	0.68	0.3	15.41	0.21
Stream C-AB	0.2	9.95	0.17	0.2	10.01	0.16

2033 Design Sensitivity						
Stream B-AC	2.0	40.21	0.70	0.3	15.88	0.22
Stream C-AB	0.2	10.12	0.17	0.2	10.14	0.17

Values shown are the highest values encountered over all time segments. Delay is the maximum value of Av. delay per arriving vehicle.

File summary

File Description

Title	Hay Green Lane / Sheffield Road Junction
Location	
Site number	
Date	19/01/2021
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	KG/SJP
Description	Updated to reflect existing 15 minute turning proportions

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Av. delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perTimeSegment	s	-Min	perMin

Analysis Options

Vehicle length (m)	Calculate Q Percentiles	Calculate detailed queueing delay	Calculate residual capacity	RFC Threshold	Av. Delay threshold (s)	Q threshold (PCU)
5.75				0.85	36.00	20.00

Demand Set Summary

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)	Run automatically
D1	2019 Count	AM	DIRECT	08:30	09:30	60	15	✓
D2	2019 Count	PM	DIRECT	16:30	17:30	60	15	✓
D3	2019 Base	AM	DIRECT	08:30	09:30	60	15	✓
D4	2019 Base	PM	DIRECT	16:30	17:30	60	15	✓
D5	2025 Base	AM	DIRECT	08:30	09:30	60	15	✓
D6	2025 Base	PM	DIRECT	16:30	17:30	60	15	✓
D7	2033 Base	AM	DIRECT	08:30	09:30	60	15	✓
D8	2033 Base	PM	DIRECT	16:30	17:30	60	15	✓
D9	2025 Design	AM	DIRECT	08:30	09:30	60	15	✓
D10	2025 Design	PM	DIRECT	16:30	17:30	60	15	✓
D11	2025 Design Sensitivity	AM	DIRECT	08:30	09:30	60	15	✓
D12	2025 Design Sensitivity	PM	DIRECT	16:30	17:30	60	15	✓
D13	2033 Design	AM	DIRECT	08:30	09:30	60	15	✓
D14	2033 Design	PM	DIRECT	16:30	17:30	60	15	✓
D15	2033 Design Sensitivity	AM	DIRECT	08:30	09:30	60	15	✓
D16	2033 Design Sensitivity	PM	DIRECT	16:30	17:30	60	15	✓

Analysis Set Details

ID	Include in report	Network flow scaling factor (%)	Network capacity scaling factor (%)
A1	✓	100.000	100.000

2019 Count, AM

Data Errors and Warnings

No errors or warnings

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.53	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Name	Description	Arm type
A	Sheffield Road (N)		Major
B	Hay Green Lane		Minor
C	Sheffield Road (S)		Major

Major Arm Geometry

Arm	Width of carriageway (m)	Has kerbed central reserve	Has right turn bay	Width for right turn (m)	Visibility for right turn (m)	Blocks?	Blocking queue (PCU)
C	7.90		✓	2.30	90.0	✓	6.20

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Arm	Minor arm type	Lane width (m)	Visibility to left (m)	Visibility to right (m)
B	One lane	2.34	18	14

Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/TS)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
1	B-A	114.488	0.077	0.193	0.122	0.276
1	B-C	147.735	0.083	0.210	-	-
1	C-B	158.224	0.225	0.225	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)	Run automatically
D1	2019 Count	AM	DIRECT	08:30	09:30	60	15	✓

Vehicle mix varies over time	Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	✓	✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Scaling Factor (%)
A		DIRECT	✓	100.000
B		DIRECT	✓	100.000
C		DIRECT	✓	100.000

Origin-Destination Data

08:30 - 08:45

Demand (PCU/TS)

		To		
		A	B	C
From	A	0.00	2.00	173.00
	B	6.00	0.00	12.00
	C	197.00	12.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99
	B	0.33	0.00	0.67
	C	0.94	0.06	0.00

08:45 - 09:00

Demand (PCU/TS)

		To		
		A	B	C
From	A	0.00	0.00	214.00
	B	0.00	0.00	4.00
	C	234.00	4.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.00	1.00
	B	0.00	0.00	1.00
	C	0.98	0.02	0.00

09:00 - 09:15

Demand (PCU/TS)

		To		
		A	B	C
From	A	0.00	3.00	151.00
	B	1.00	0.00	5.00
	C	223.00	14.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.02	0.98
	B	0.17	0.00	0.83
	C	0.94	0.06	0.00

09:15 - 09:30

Demand (PCU/TS)

		To		
		A	B	C
From	A	0.00	9.00	177.00
	B	1.00	0.00	8.00
	C	199.00	10.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.05	0.95
	B	0.11	0.00	0.89
	C	0.95	0.05	0.00

Vehicle Mix

08:30 - 08:45

HV %s

		To		
		A	B	C
From	A	0	0	10
	B	0	0	0
	C	6	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.100
	B	1.000	1.000	1.000
	C	1.060	1.000	1.000

08:45 - 09:00

HV %s

		To		
		A	B	C
From	A	0	0	5
	B	0	0	0
	C	5	25	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.050
	B	1.000	1.000	1.000
	C	1.050	1.250	1.000

09:00 - 09:15

HV %s

		To		
		A	B	C
From	A	0	0	6

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.060

	B	0	0	20
	C	7	0	0

	B	1.000	1.000	1.200
	C	1.070	1.000	1.000

HV %s

09:15 - 09:30

		To		
		A	B	C
From	A	0	0	3
	B	0	0	0
	C	7	10	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.071	1.100	1.000

Detailed Demand Data**Demand for each time segment**

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
08:30-08:45	A	175.00	175.00
	B	18.00	18.00
	C	209.00	209.00
08:45-09:00	A	214.00	214.00
	B	4.00	4.00
	C	238.00	238.00
09:00-09:15	A	154.00	154.00
	B	6.00	6.00
	C	237.00	237.00
09:15-09:30	A	186.00	186.00
	B	9.00	9.00
	C	209.00	209.00

Results**Results Summary for whole modelled period**

Stream	Max RFC	Max Delay (s)	Max Q (PCU)	Max LOS	Av. Demand (PCU/TS)	Total Junction Arrivals (PCU)
B-AC	0.22	13.97	0.3	B	9.25	37.00
C-AB	0.11	8.89	0.1	A	10.00	40.00
C-A					213.25	853.00
A-B					3.50	14.00
A-C					178.75	715.00

Main Results for each time segment**08:30 - 08:45**

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	18.00	18.00	81.88	0.220	17.72	0.0	0.3	13.970	B
C-AB	12.00	12.00	118.86	0.101	11.89	0.0	0.1	8.405	A
C-A	197.00	197.00			197.00				
A-B	2.00	2.00			2.00				
A-C	173.00	173.00			173.00				

08:45 - 09:00

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	4.00	4.00	99.86	0.040	4.23	0.3	0.0	9.433	A

C-AB	4.00	4.00	110.09	0.036	4.07	0.1	0.0	8.892	A
C-A	234.00	234.00			234.00				
A-B	0.00	0.00			0.00				
A-C	214.00	214.00			214.00				

09:00 - 09:15

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	6.00	6.00	97.36	0.062	5.97	0.0	0.1	10.698	B
C-AB	14.00	14.00	123.58	0.113	13.91	0.0	0.1	8.630	A
C-A	223.00	223.00			223.00				
A-B	3.00	3.00			3.00				
A-C	151.00	151.00			151.00				

09:15 - 09:30

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	9.00	9.00	97.91	0.092	8.96	0.1	0.1	10.770	B
C-AB	10.00	10.00	116.38	0.086	10.03	0.1	0.1	8.756	A
C-A	199.00	199.00			199.00				
A-B	9.00	9.00			9.00				
A-C	177.00	177.00			177.00				

2019 Count, PM

Data Errors and Warnings

No errors or warnings

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.34	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)	Run automatically
D2	2019 Count	PM	DIRECT	16:30	17:30	60	15	✓

Vehicle mix varies over time	Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	✓	✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Scaling Factor (%)
A		DIRECT	✓	100.000
B		DIRECT	✓	100.000
C		DIRECT	✓	100.000

Origin-Destination Data

Demand (PCU/TS)

16:30 - 16:45

		To		
		A	B	C
From	A	0.00	4.00	199.00
	B	1.00	0.00	8.00
	C	200.00	7.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.02	0.98
	B	0.11	0.00	0.89
	C	0.97	0.03	0.00

Demand (PCU/TS)

16:45 - 17:00

		To		
		A	B	C
From	A	0.00	1.00	187.00
	B	1.00	0.00	6.00
	C	215.00	8.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99
	B	0.14	0.00	0.86
	C	0.96	0.04	0.00

Demand (PCU/TS)

17:00 - 17:15

		To		
		A	B	C
From	A	0.00	1.00	190.00
	B	1.00	0.00	6.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99
	B	0.14	0.00	0.86

	C	224.00	7.00	0.00
--	---	--------	------	------

	C	0.97	0.03	0.00
--	---	------	------	------

Demand (PCU/TS)

17:15 - 17:30

		To		
		A	B	C
From	A	0.00	2.00	181.00
	B	0.00	0.00	5.00
	C	201.00	9.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99
	B	0.00	0.00	1.00
	C	0.96	0.04	0.00

Vehicle Mix

HV %s

16:30 - 16:45

		To		
		A	B	C
From	A	0	0	5
	B	0	0	0
	C	3	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.050
	B	1.000	1.000	1.000
	C	1.030	1.000	1.000

HV %s

16:45 - 17:00

		To		
		A	B	C
From	A	0	0	4
	B	0	0	0
	C	3	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.040
	B	1.000	1.000	1.000
	C	1.030	1.000	1.000

HV %s

17:00 - 17:15

		To		
		A	B	C
From	A	0	0	2
	B	0	0	0
	C	2	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.020
	B	1.000	1.000	1.000
	C	1.020	1.000	1.000

HV %s

17:15 - 17:30

		To		
		A	B	C
From	A	0	0	2
	B	0	0	0
	C	3	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.020
	B	1.000	1.000	1.000
	C	1.030	1.000	1.000

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
16:30-16:45	A	203.00	203.00
	B	9.00	9.00
	C	207.00	207.00
16:45-17:00	A	188.00	188.00
	B	7.00	7.00
	C	223.00	223.00
17:00-17:15	A	191.00	191.00
	B	7.00	7.00
	C	231.00	231.00
17:15-17:30	A	183.00	183.00
	B	5.00	5.00
	C	210.00	210.00

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Q (PCU)	Max LOS	Av. Demand (PCU/TS)	Total Junction Arrivals (PCU)
B-AC	0.10	10.62	0.1	B	7.00	28.00
C-AB	0.08	8.52	0.1	A	7.75	31.00
C-A					210.00	840.00
A-B					2.00	8.00
A-C					189.25	757.00

Main Results for each time segment

16:30 - 16:45

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	9.00	9.00	93.76	0.096	8.90	0.0	0.1	10.592	B
C-AB	7.00	7.00	112.56	0.062	6.93	0.0	0.1	8.515	A
C-A	200.00	200.00			200.00				
A-B	4.00	4.00			4.00				
A-C	199.00	199.00			199.00				

16:45 - 17:00

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	7.00	7.00	92.86	0.075	7.02	0.1	0.1	10.487	B
C-AB	8.00	8.00	115.93	0.069	7.99	0.1	0.1	8.338	A
C-A	215.00	215.00			215.00				
A-B	1.00	1.00			1.00				
A-C	187.00	187.00			187.00				

17:00 - 17:15

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	7.00	7.00	91.71	0.076	7.00	0.1	0.1	10.623	B
C-AB	7.00	7.00	115.26	0.061	7.01	0.1	0.1	8.316	A
C-A	224.00	224.00			224.00				
A-B	1.00	1.00			1.00				
A-C	190.00	190.00			190.00				

17:15 - 17:30

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	5.00	5.00	109.28	0.046	5.03	0.1	0.0	8.637	A
C-AB	9.00	9.00	117.06	0.077	8.98	0.1	0.1	8.326	A
C-A	201.00	201.00			201.00				
A-B	2.00	2.00			2.00				
A-C	181.00	181.00			181.00				

2019 Base, AM

Data Errors and Warnings

No errors or warnings

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.53	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)	Run automatically
D3	2019 Base	AM	DIRECT	08:30	09:30	60	15	✓

Vehicle mix varies over time	Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	✓	✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Scaling Factor (%)
A		DIRECT	✓	100.000
B		DIRECT	✓	100.000
C		DIRECT	✓	100.000

Origin-Destination Data

Demand (PCU/TS)

08:30 - 08:45

		To		
		A	B	C
From	A	0.00	2.00	184.00
	B	6.00	0.00	12.00
	C	211.00	13.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99
	B	0.33	0.00	0.67
	C	0.94	0.06	0.00

Demand (PCU/TS)

08:45 - 09:00

		To		
		A	B	C
From	A	0.00	0.00	227.00
	B	0.00	0.00	4.00
	C	250.00	4.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.00	1.00
	B	0.00	0.00	1.00
	C	0.98	0.02	0.00

Demand (PCU/TS)

09:00 - 09:15

		To		
		A	B	C
From	A	0.00	3.00	160.00
	B	1.00	0.00	5.00

Proportions

		To		
		A	B	C
From	A	0.00	0.02	0.98
	B	0.17	0.00	0.83

	C	238.00	15.00	0.00
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	C	0.94	0.06	0.00
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Demand (PCU/TS)

09:15 - 09:30

		To		
		A	B	C
From	A	0.00	9.00	188.00
	B	1.00	0.00	8.00
	C	213.00	11.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.05	0.95
	B	0.11	0.00	0.89
	C	0.95	0.05	0.00

Vehicle Mix

HV %s

08:30 - 08:45

		To			
		A	B	C	
From	A	0	0	12	
	B	0	0	0	
	C	7	0	0	

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.120
	B	1.000	1.000	1.000
	C	1.070	1.000	1.000

HV %s

08:45 - 09:00

		To		
		A	B	C
From	A	0	0	6
	B	0	0	0
	C	5	24	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.060
	B	1.000	1.000	1.000
	C	1.050	1.240	1.000

HV %s

09:00 - 09:15

		To		
		A	B	C
From	A	0	0	7
	B	0	0	19
	C	8	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.070
	B	1.000	1.000	1.190
	C	1.080	1.000	1.000

HV %s

09:15 - 09:30

		To		
		A	B	C
From	A	0	0	3
	B	0	0	0
	C	8	10	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.080	1.100	1.000

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
08:30-08:45	A	186.00	186.00
	B	18.00	18.00
	C	224.00	224.00
08:45-09:00	A	227.00	227.00
	B	4.00	4.00
	C	254.00	254.00
09:00-09:15	A	163.00	163.00
	B	6.00	6.00
	C	253.00	253.00
09:15-09:30	A	197.00	197.00
	B	9.00	9.00
	C	224.00	224.00

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Q (PCU)	Max LOS	Av. Demand (PCU/TS)	Total Junction Arrivals (PCU)
B-AC	0.23	14.91	0.3	B	9.25	37.00
C-AB	0.12	9.10	0.1	A	10.75	43.00
C-A					228.00	912.00
A-B					3.50	14.00
A-C					189.75	759.00

Main Results for each time segment

08:30 - 08:45

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	18.00	18.00	77.78	0.231	17.71	0.0	0.3	14.912	B
C-AB	13.00	13.00	116.38	0.112	12.88	0.0	0.1	8.684	A
C-A	211.00	211.00			211.00				
A-B	2.00	2.00			2.00				
A-C	184.00	184.00			184.00				

08:45 - 09:00

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	4.00	4.00	96.60	0.041	4.25	0.3	0.0	9.772	A
C-AB	4.00	4.00	107.16	0.037	4.08	0.1	0.0	9.101	A
C-A	250.00	250.00			250.00				
A-B	0.00	0.00			0.00				
A-C	227.00	227.00			227.00				

09:00 - 09:15

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	6.00	6.00	94.12	0.064	5.97	0.0	0.1	11.050	B
C-AB	15.00	15.00	121.56	0.123	14.89	0.0	0.1	8.838	A
C-A	238.00	238.00			238.00				
A-B	3.00	3.00			3.00				
A-C	160.00	160.00			160.00				

09:15 - 09:30

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	9.00	9.00	94.63	0.095	8.96	0.1	0.1	11.152	B
C-AB	11.00	11.00	113.91	0.097	11.03	0.1	0.1	9.050	A
C-A	213.00	213.00			213.00				
A-B	9.00	9.00			9.00				
A-C	188.00	188.00			188.00				

2019 Base, PM

Data Errors and Warnings

No errors or warnings

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.33	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)	Run automatically
D4	2019 Base	PM	DIRECT	16:30	17:30	60	15	✓

Vehicle mix varies over time	Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	✓	✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Scaling Factor (%)
A		DIRECT	✓	100.000
B		DIRECT	✓	100.000
C		DIRECT	✓	100.000

Origin-Destination Data

Demand (PCU/TS)

16:30 - 16:45

		To		
		A	B	C
From	A	0.00	4.00	206.00
	B	1.00	0.00	8.00
	C	206.00	7.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.02	0.98
	B	0.11	0.00	0.89
	C	0.97	0.03	0.00

Demand (PCU/TS)

16:45 - 17:00

		To		
		A	B	C
From	A	0.00	1.00	193.00
	B	1.00	0.00	6.00
	C	221.00	8.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99
	B	0.14	0.00	0.86
	C	0.97	0.03	0.00

Demand (PCU/TS)

17:00 - 17:15

		To		
		A	B	C
From	A	0.00	1.00	197.00
	B	1.00	0.00	6.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99
	B	0.14	0.00	0.86

	C	230.00	7.00	0.00
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	C	0.97	0.03	0.00
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Demand (PCU/TS)

17:15 - 17:30

		To		
		A	B	C
From	A	0.00	2.00	187.00
	B	0.00	0.00	5.00
	C	207.00	9.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99
	B	0.00	0.00	1.00
	C	0.96	0.04	0.00

Vehicle Mix

HV %s

16:30 - 16:45

		To			
		A	B	C	
From	A	0	0	7	
	B	0	0	0	
	C	4	0	0	

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.070
	B	1.000	1.000	1.000
	C	1.040	1.000	1.000

HV %s

16:45 - 17:00

		To			
		A	B	C	
From	A	0	0	5	
	B	0	0	0	
	C	4	0	0	

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.050
	B	1.000	1.000	1.000
	C	1.040	1.000	1.000

HV %s

17:00 - 17:15

		To			
		A	B	C	
From	A	0	0	3	
	B	0	0	0	
	C	3	0	0	

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.030	1.000	1.000

HV %s

17:15 - 17:30

		To			
		A	B	C	
From	A	0	0	3	
	B	0	0	0	
	C	5	0	0	

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.050	1.000	1.000

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
16:30-16:45	A	210.00	210.00
	B	9.00	9.00
	C	213.00	213.00
16:45-17:00	A	194.00	194.00
	B	7.00	7.00
	C	229.00	229.00
17:00-17:15	A	198.00	198.00
	B	7.00	7.00
	C	237.00	237.00
17:15-17:30	A	189.00	189.00
	B	5.00	5.00
	C	216.00	216.00

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Q (PCU)	Max LOS	Av. Demand (PCU/TS)	Total Junction Arrivals (PCU)
B-AC	0.10	10.88	0.1	B	7.00	28.00
C-AB	0.08	8.64	0.1	A	7.75	31.00
C-A					216.00	864.00
A-B					2.00	8.00
A-C					195.75	783.00

Main Results for each time segment

16:30 - 16:45

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	9.00	9.00	91.89	0.098	8.89	0.0	0.1	10.829	B
C-AB	7.00	7.00	110.99	0.063	6.93	0.0	0.1	8.644	A
C-A	206.00	206.00			206.00				
A-B	4.00	4.00			4.00				
A-C	206.00	206.00			206.00				

16:45 - 17:00

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	7.00	7.00	91.12	0.077	7.02	0.1	0.1	10.704	B
C-AB	8.00	8.00	114.58	0.070	7.99	0.1	0.1	8.443	A
C-A	221.00	221.00			221.00				
A-B	1.00	1.00			1.00				
A-C	193.00	193.00			193.00				

17:00 - 17:15

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	7.00	7.00	89.72	0.078	7.00	0.1	0.1	10.879	B
C-AB	7.00	7.00	113.68	0.062	7.01	0.1	0.1	8.438	A
C-A	230.00	230.00			230.00				
A-B	1.00	1.00			1.00				
A-C	197.00	197.00			197.00				

17:15 - 17:30

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	5.00	5.00	108.00	0.046	5.04	0.1	0.0	8.743	A
C-AB	9.00	9.00	115.71	0.078	8.98	0.1	0.1	8.432	A
C-A	207.00	207.00			207.00				
A-B	2.00	2.00			2.00				
A-C	187.00	187.00			187.00				

2025 Base, AM

Data Errors and Warnings

No errors or warnings

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.55	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)	Run automatically
D5	2025 Base	AM	DIRECT	08:30	09:30	60	15	✓

Vehicle mix varies over time	Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	✓	✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Scaling Factor (%)
A		DIRECT	✓	100.000
B		DIRECT	✓	100.000
C		DIRECT	✓	100.000

Origin-Destination Data

Demand (PCU/TS)

08:30 - 08:45

		To		
		A	B	C
From	A	0.00	2.00	194.00
	B	6.00	0.00	13.00
	C	223.00	13.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99
	B	0.32	0.00	0.68
	C	0.94	0.06	0.00

Demand (PCU/TS)

08:45 - 09:00

		To		
		A	B	C
From	A	0.00	0.00	240.00
	B	0.00	0.00	4.00
	C	265.00	4.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.00	1.00
	B	0.00	0.00	1.00
	C	0.99	0.01	0.00

Demand (PCU/TS)

09:00 - 09:15

		To		
		A	B	C
From	A	0.00	3.00	170.00
	B	1.00	0.00	6.00

Proportions

		To		
		A	B	C
From	A	0.00	0.02	0.98
	B	0.14	0.00	0.86

	C	252.00	15.00	0.00
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	C	0.94	0.06	0.00
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Demand (PCU/TS)

09:15 - 09:30

		To		
		A	B	C
From	A	0.00	10.00	199.00
	B	1.00	0.00	9.00
	C	225.00	11.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.05	0.95
	B	0.10	0.00	0.90
	C	0.95	0.05	0.00

Vehicle Mix

HV %s

08:30 - 08:45

		To			
		A	B	C	
From	A	0	0	12	
	B	0	0	0	
	C	7	0	0	

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.120
	B	1.000	1.000	1.000
	C	1.070	1.000	1.000

HV %s

08:45 - 09:00

		To			
		A	B	C	
From	A	0	0	6	
	B	0	0	0	
	C	5	23	0	

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.060
	B	1.000	1.000	1.000
	C	1.050	1.230	1.000

HV %s

09:00 - 09:15

		To			
		A	B	C	
From	A	0	0	7	
	B	0	0	18	
	C	8	0	0	

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.070
	B	1.000	1.000	1.180
	C	1.080	1.000	1.000

HV %s

09:15 - 09:30

		To			
		A	B	C	
From	A	0	0	3	
	B	0	0	0	
	C	8	9	0	

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.080	1.090	1.000

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
08:30-08:45	A	196.00	196.00
	B	19.00	19.00
	C	236.00	236.00
08:45-09:00	A	240.00	240.00
	B	4.00	4.00
	C	269.00	269.00
09:00-09:15	A	173.00	173.00
	B	7.00	7.00
	C	267.00	267.00
09:15-09:30	A	209.00	209.00
	B	10.00	10.00
	C	236.00	236.00

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Q (PCU)	Max LOS	Av. Demand (PCU/TS)	Total Junction Arrivals (PCU)
B-AC	0.25	15.77	0.3	C	10.00	40.00
C-AB	0.13	9.35	0.1	A	10.75	43.00
C-A					241.25	965.00
A-B					3.75	15.00
A-C					200.75	803.00

Main Results for each time segment

08:30 - 08:45

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	19.00	19.00	75.43	0.252	18.67	0.0	0.3	15.768	C
C-AB	13.00	13.00	114.13	0.114	12.87	0.0	0.1	8.877	A
C-A	223.00	223.00			223.00				
A-B	2.00	2.00			2.00				
A-C	194.00	194.00			194.00				

08:45 - 09:00

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	4.00	4.00	93.28	0.043	4.28	0.3	0.0	10.144	B
C-AB	4.00	4.00	104.24	0.038	4.09	0.1	0.0	9.353	A
C-A	265.00	265.00			265.00				
A-B	0.00	0.00			0.00				
A-C	240.00	240.00			240.00				

09:00 - 09:15

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	7.00	7.00	93.24	0.075	6.96	0.0	0.1	11.331	B
C-AB	15.00	15.00	119.31	0.126	14.89	0.0	0.1	9.015	A
C-A	252.00	252.00			252.00				
A-B	3.00	3.00			3.00				
A-C	170.00	170.00			170.00				

09:15 - 09:30

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	10.00	10.00	92.59	0.108	9.96	0.1	0.1	11.583	B
C-AB	11.00	11.00	111.21	0.099	11.03	0.1	0.1	9.265	A
C-A	225.00	225.00			225.00				
A-B	10.00	10.00			10.00				
A-C	199.00	199.00			199.00				

2025 Base, PM

Data Errors and Warnings

No errors or warnings

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.34	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)	Run automatically
D6	2025 Base	PM	DIRECT	16:30	17:30	60	15	✓

Vehicle mix varies over time	Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	✓	✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Scaling Factor (%)
A		DIRECT	✓	100.000
B		DIRECT	✓	100.000
C		DIRECT	✓	100.000

Origin-Destination Data

Demand (PCU/TS)

16:30 - 16:45

		To		
		A	B	C
From	A	0.00	4.00	218.00
	B	1.00	0.00	8.00
	C	218.00	7.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.02	0.98
	B	0.11	0.00	0.89
	C	0.97	0.03	0.00

Demand (PCU/TS)

16:45 - 17:00

		To		
		A	B	C
From	A	0.00	1.00	205.00
	B	1.00	0.00	6.00
	C	234.00	9.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.00	1.00
	B	0.14	0.00	0.86
	C	0.96	0.04	0.00

Demand (PCU/TS)

17:00 - 17:15

		To		
		A	B	C
From	A	0.00	1.00	208.00
	B	1.00	0.00	6.00

Proportions

		To		
		A	B	C
From	A	0.00	0.00	1.00
	B	0.14	0.00	0.86

	C	244.00	7.00	0.00
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	C	0.97	0.03	0.00
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Demand (PCU/TS)

17:15 - 17:30

		To		
		A	B	C
From	A	0.00	2.00	198.00
	B	0.00	0.00	5.00
	C	219.00	10.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99
	B	0.00	0.00	1.00
	C	0.96	0.04	0.00

Vehicle Mix

HV %s

16:30 - 16:45

		To		
		A	B	C
From	A	0	0	7
	B	0	0	0
	C	4	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.070
	B	1.000	1.000	1.000
	C	1.040	1.000	1.000

HV %s

16:45 - 17:00

		To		
		A	B	C
From	A	0	0	5
	B	0	0	0
	C	4	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.050
	B	1.000	1.000	1.000
	C	1.040	1.000	1.000

HV %s

17:00 - 17:15

		To		
		A	B	C
From	A	0	0	3
	B	0	0	0
	C	3	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.030	1.000	1.000

HV %s

17:15 - 17:30

		To		
		A	B	C
From	A	0	0	3
	B	0	0	0
	C	5	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.050	1.000	1.000

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
16:30-16:45	A	222.00	222.00
	B	9.00	9.00
	C	225.00	225.00
16:45-17:00	A	206.00	206.00
	B	7.00	7.00
	C	243.00	243.00
17:00-17:15	A	209.00	209.00
	B	7.00	7.00
	C	251.00	251.00
17:15-17:30	A	200.00	200.00
	B	5.00	5.00
	C	229.00	229.00

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Q (PCU)	Max LOS	Av. Demand (PCU/TS)	Total Junction Arrivals (PCU)
B-AC	0.10	11.36	0.1	B	7.00	28.00
C-AB	0.09	8.87	0.1	A	8.25	33.00
C-A					228.75	915.00
A-B					2.00	8.00
A-C					207.25	829.00

Main Results for each time segment

16:30 - 16:45

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	9.00	9.00	88.51	0.102	8.89	0.0	0.1	11.287	B
C-AB	7.00	7.00	108.29	0.065	6.93	0.0	0.1	8.874	A
C-A	218.00	218.00			218.00				
A-B	4.00	4.00			4.00				
A-C	218.00	218.00			218.00				

16:45 - 17:00

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	7.00	7.00	87.33	0.080	7.02	0.1	0.1	11.209	B
C-AB	9.00	9.00	111.89	0.080	8.98	0.1	0.1	8.745	A
C-A	234.00	234.00			234.00				
A-B	1.00	1.00			1.00				
A-C	205.00	205.00			205.00				

17:00 - 17:15

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	7.00	7.00	86.19	0.081	7.00	0.1	0.1	11.365	B
C-AB	7.00	7.00	111.21	0.063	7.02	0.1	0.1	8.640	A
C-A	244.00	244.00			244.00				
A-B	1.00	1.00			1.00				
A-C	208.00	208.00			208.00				

17:15 - 17:30

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	5.00	5.00	105.65	0.047	5.04	0.1	0.1	8.950	A
C-AB	10.00	10.00	113.23	0.088	9.97	0.1	0.1	8.714	A
C-A	219.00	219.00			219.00				
A-B	2.00	2.00			2.00				
A-C	198.00	198.00			198.00				

2033 Base, AM

Data Errors and Warnings

No errors or warnings

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.63	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)	Run automatically
D7	2033 Base	AM	DIRECT	08:30	09:30	60	15	✓

Vehicle mix varies over time	Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	✓	✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Scaling Factor (%)
A		DIRECT	✓	100.000
B		DIRECT	✓	100.000
C		DIRECT	✓	100.000

Origin-Destination Data

Demand (PCU/TS)

08:30 - 08:45

		To		
		A	B	C
From	A	0.00	2.00	207.00
	B	7.00	0.00	14.00
	C	237.00	14.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99
	B	0.33	0.00	0.67
	C	0.94	0.06	0.00

Demand (PCU/TS)

08:45 - 09:00

		To		
		A	B	C
From	A	0.00	0.00	256.00
	B	0.00	0.00	5.00
	C	281.00	5.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.00	1.00
	B	0.00	0.00	1.00
	C	0.98	0.02	0.00

Demand (PCU/TS)

09:00 - 09:15

		To		
		A	B	C
From	A	0.00	3.00	180.00
	B	1.00	0.00	6.00

Proportions

		To		
		A	B	C
From	A	0.00	0.02	0.98
	B	0.14	0.00	0.86

	C	268.00	16.00	0.00
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	C	0.94	0.06	0.00
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Demand (PCU/TS)

09:15 - 09:30

		To		
		A	B	C
From	A	0.00	10.00	211.00
	B	1.00	0.00	9.00
	C	239.00	12.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.05	0.95
	B	0.10	0.00	0.90
	C	0.95	0.05	0.00

Vehicle Mix

HV %s

08:30 - 08:45

		To			
		A	B	C	
From	A	0	0	12	
	B	0	0	0	
	C	7	0	0	

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.120
	B	1.000	1.000	1.000
	C	1.070	1.000	1.000

HV %s

08:45 - 09:00

		To		
		A	B	C
From	A	0	0	6
	B	0	0	0
	C	5	21	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.060
	B	1.000	1.000	1.000
	C	1.050	1.210	1.000

HV %s

09:00 - 09:15

		To		
		A	B	C
From	A	0	0	7
	B	0	0	17
	C	8	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.070
	B	1.000	1.000	1.170
	C	1.080	1.000	1.000

HV %s

09:15 - 09:30

		To		
		A	B	C
From	A	0	0	3
	B	0	0	0
	C	8	9	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.080	1.090	1.000

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
08:30-08:45	A	209.00	209.00
	B	21.00	21.00
	C	251.00	251.00
08:45-09:00	A	256.00	256.00
	B	5.00	5.00
	C	286.00	286.00
09:00-09:15	A	183.00	183.00
	B	7.00	7.00
	C	284.00	284.00
09:15-09:30	A	221.00	221.00
	B	10.00	10.00
	C	251.00	251.00

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Q (PCU)	Max LOS	Av. Demand (PCU/TS)	Total Junction Arrivals (PCU)
B-AC	0.30	18.30	0.4	C	10.75	43.00
C-AB	0.14	9.81	0.2	A	11.75	47.00
C-A					256.25	1025.00
A-B					3.75	15.00
A-C					213.50	854.00

Main Results for each time segment

08:30 - 08:45

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	21.00	21.00	69.36	0.303	20.58	0.0	0.4	18.302	C
C-AB	14.00	14.00	111.21	0.126	13.86	0.0	0.1	9.232	A
C-A	237.00	237.00			237.00				
A-B	2.00	2.00			2.00				
A-C	207.00	207.00			207.00				

08:45 - 09:00

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	5.00	5.00	88.89	0.056	5.36	0.4	0.1	10.818	B
C-AB	5.00	5.00	100.64	0.050	5.09	0.1	0.1	9.814	A
C-A	281.00	281.00			281.00				
A-B	0.00	0.00			0.00				
A-C	256.00	256.00			256.00				

09:00 - 09:15

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	7.00	7.00	89.55	0.078	6.97	0.1	0.1	11.695	B
C-AB	16.00	16.00	117.06	0.137	15.89	0.1	0.2	9.330	A
C-A	268.00	268.00			268.00				
A-B	3.00	3.00			3.00				
A-C	180.00	180.00			180.00				

09:15 - 09:30

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	10.00	10.00	88.89	0.112	9.96	0.1	0.1	12.090	B
C-AB	12.00	12.00	108.51	0.111	12.03	0.2	0.1	9.619	A
C-A	239.00	239.00			239.00				
A-B	10.00	10.00			10.00				
A-C	211.00	211.00			211.00				

2033 Base, PM

Data Errors and Warnings

No errors or warnings

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.37	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)	Run automatically
D8	2033 Base	PM	DIRECT	16:30	17:30	60	15	✓

Vehicle mix varies over time	Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	✓	✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Scaling Factor (%)
A		DIRECT	✓	100.000
B		DIRECT	✓	100.000
C		DIRECT	✓	100.000

Origin-Destination Data

Demand (PCU/TS)

16:30 - 16:45

		To		
		A	B	C
From	A	0.00	5.00	232.00
	B	1.00	0.00	9.00
	C	232.00	8.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.02	0.98
	B	0.10	0.00	0.90
	C	0.97	0.03	0.00

Demand (PCU/TS)

16:45 - 17:00

		To		
		A	B	C
From	A	0.00	1.00	218.00
	B	1.00	0.00	7.00
	C	249.00	9.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.00	1.00
	B	0.13	0.00	0.88
	C	0.97	0.03	0.00

Demand (PCU/TS)

17:00 - 17:15

		To		
		A	B	C
From	A	0.00	1.00	221.00
	B	1.00	0.00	7.00

Proportions

		To		
		A	B	C
From	A	0.00	0.00	1.00
	B	0.13	0.00	0.88

	C	259.00	8.00	0.00
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	C	0.97	0.03	0.00
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Demand (PCU/TS)

17:15 - 17:30

		To		
		A	B	C
From	A	0.00	2.00	211.00
	B	0.00	0.00	6.00
	C	233.00	10.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99
	B	0.00	0.00	1.00
	C	0.96	0.04	0.00

Vehicle Mix

HV %s

16:30 - 16:45

		To		
		A	B	C
From	A	0	0	7
	B	0	0	0
	C	4	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.070
	B	1.000	1.000	1.000
	C	1.040	1.000	1.000

HV %s

16:45 - 17:00

		To		
		A	B	C
From	A	0	0	5
	B	0	0	0
	C	4	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.050
	B	1.000	1.000	1.000
	C	1.040	1.000	1.000

HV %s

17:00 - 17:15

		To		
		A	B	C
From	A	0	0	3
	B	0	0	0
	C	3	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.030	1.000	1.000

HV %s

17:15 - 17:30

		To		
		A	B	C
From	A	0	0	3
	B	0	0	0
	C	5	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.050	1.000	1.000

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
16:30-16:45	A	237.00	237.00
	B	10.00	10.00
	C	240.00	240.00
16:45-17:00	A	219.00	219.00
	B	8.00	8.00
	C	258.00	258.00
17:00-17:15	A	222.00	222.00
	B	8.00	8.00
	C	267.00	267.00
17:15-17:30	A	213.00	213.00
	B	6.00	6.00
	C	243.00	243.00

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Q (PCU)	Max LOS	Av. Demand (PCU/TS)	Total Junction Arrivals (PCU)
B-AC	0.12	11.89	0.1	B	8.00	32.00
C-AB	0.09	9.27	0.1	A	8.75	35.00
C-A					243.25	973.00
A-B					2.25	9.00
A-C					220.50	882.00

Main Results for each time segment

16:30 - 16:45

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	10.00	10.00	85.43	0.117	9.87	0.0	0.1	11.891	B
C-AB	8.00	8.00	104.91	0.076	7.92	0.0	0.1	9.271	A
C-A	232.00	232.00			232.00				
A-B	5.00	5.00			5.00				
A-C	232.00	232.00			232.00				

16:45 - 17:00

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	8.00	8.00	85.09	0.094	8.03	0.1	0.1	11.682	B
C-AB	9.00	9.00	108.96	0.083	8.99	0.1	0.1	9.003	A
C-A	249.00	249.00			249.00				
A-B	1.00	1.00			1.00				
A-C	218.00	218.00			218.00				

17:00 - 17:15

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	8.00	8.00	83.76	0.096	8.00	0.1	0.1	11.879	B
C-AB	8.00	8.00	108.29	0.074	8.01	0.1	0.1	8.977	A
C-A	259.00	259.00			259.00				
A-B	1.00	1.00			1.00				
A-C	221.00	221.00			221.00				

17:15 - 17:30

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	6.00	6.00	102.93	0.058	6.04	0.1	0.1	9.292	A
C-AB	10.00	10.00	110.31	0.091	9.98	0.1	0.1	8.970	A
C-A	233.00	233.00			233.00				
A-B	2.00	2.00			2.00				
A-C	211.00	211.00			211.00				

2025 Design, AM

Data Errors and Warnings

No errors or warnings

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		1.66	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)	Run automatically
D9	2025 Design	AM	DIRECT	08:30	09:30	60	15	✓

Vehicle mix varies over time	Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	✓	✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Scaling Factor (%)
A		DIRECT	✓	100.000
B		DIRECT	✓	100.000
C		DIRECT	✓	100.000

Origin-Destination Data

Demand (PCU/TS)

08:30 - 08:45

		To		
		A	B	C
From	A	0.00	3.00	194.00
	B	17.00	0.00	26.00
	C	223.00	17.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.02	0.98
	B	0.40	0.00	0.60
	C	0.93	0.07	0.00

Demand (PCU/TS)

08:45 - 09:00

		To		
		A	B	C
From	A	0.00	0.00	240.00
	B	0.00	0.00	9.00
	C	265.00	6.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.00	1.00
	B	0.00	0.00	1.00
	C	0.98	0.02	0.00

Demand (PCU/TS)

09:00 - 09:15

		To		
		A	B	C
From	A	0.00	4.00	170.00
	B	3.00	0.00	11.00

Proportions

		To		
		A	B	C
From	A	0.00	0.02	0.98
	B	0.21	0.00	0.79

	C	252.00	19.00	0.00
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	C	0.93	0.07	0.00
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Demand (PCU/TS)

09:15 - 09:30

		To		
		A	B	C
From	A	0.00	13.00	199.00
	B	3.00	0.00	17.00
	C	225.00	14.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.06	0.94
	B	0.15	0.00	0.85
	C	0.94	0.06	0.00

Vehicle Mix

HV %s

08:30 - 08:45

		To			
		A	B	C	
From	A	0	0	12	
	B	0	0	0	
	C	7	0	0	

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.120
	B	1.000	1.000	1.000
	C	1.072	1.000	1.000

HV %s

08:45 - 09:00

		To		
		A	B	C
From	A	0	0	6
	B	0	0	0
	C	5	18	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.060
	B	1.000	1.000	1.000
	C	1.050	1.180	1.000

HV %s

09:00 - 09:15

		To		
		A	B	C
From	A	0	0	7
	B	0	0	9
	C	8	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.070
	B	1.000	1.000	1.090
	C	1.080	1.000	1.000

HV %s

09:15 - 09:30

		To		
		A	B	C
From	A	0	0	3
	B	0	0	0
	C	8	7	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.080	1.070	1.000

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
08:30-08:45	A	197.00	197.00
	B	43.00	43.00
	C	240.00	240.00
08:45-09:00	A	240.00	240.00
	B	9.00	9.00
	C	271.00	271.00
09:00-09:15	A	174.00	174.00
	B	14.00	14.00
	C	271.00	271.00
09:15-09:30	A	212.00	212.00
	B	20.00	20.00
	C	239.00	239.00

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Q (PCU)	Max LOS	Av. Demand (PCU/TS)	Total Junction Arrivals (PCU)
B-AC	0.62	31.14	1.5	D	21.50	86.00
C-AB	0.16	9.55	0.2	A	14.00	56.00
C-A					241.25	965.00
A-B					5.00	20.00
A-C					200.75	803.00

Main Results for each time segment

08:30 - 08:45

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	43.00	43.00	69.12	0.622	41.50	0.0	1.5	31.141	D
C-AB	17.00	17.00	113.91	0.149	16.83	0.0	0.2	9.252	A
C-A	223.00	223.00			223.00				
A-B	3.00	3.00			3.00				
A-C	194.00	194.00			194.00				

08:45 - 09:00

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	9.00	9.00	88.10	0.102	10.39	1.5	0.1	11.779	B
C-AB	6.00	6.00	104.24	0.058	6.11	0.2	0.1	9.498	A
C-A	265.00	265.00			265.00				
A-B	0.00	0.00			0.00				
A-C	240.00	240.00			240.00				

09:00 - 09:15

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	14.00	14.00	85.22	0.164	13.92	0.1	0.2	13.061	B
C-AB	19.00	19.00	119.09	0.160	18.87	0.1	0.2	9.373	A
C-A	252.00	252.00			252.00				
A-B	4.00	4.00			4.00				
A-C	170.00	170.00			170.00				

09:15 - 09:30

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	20.00	20.00	86.61	0.231	19.90	0.2	0.3	13.948	B
C-AB	14.00	14.00	110.54	0.127	14.05	0.2	0.2	9.546	A
C-A	225.00	225.00			225.00				
A-B	13.00	13.00			13.00				
A-C	199.00	199.00			199.00				

2025 Design, PM

Data Errors and Warnings

No errors or warnings

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.69	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)	Run automatically
D10	2025 Design	PM	DIRECT	16:30	17:30	60	15	✓

Vehicle mix varies over time	Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	✓	✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Scaling Factor (%)
A		DIRECT	✓	100.000
B		DIRECT	✓	100.000
C		DIRECT	✓	100.000

Origin-Destination Data

Demand (PCU/TS)

16:30 - 16:45

		To		
		A	B	C
From	A	0.00	10.00	218.00
	B	3.00	0.00	13.00
	C	218.00	14.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.04	0.96
	B	0.19	0.00	0.81
	C	0.94	0.06	0.00

Demand (PCU/TS)

16:45 - 17:00

		To		
		A	B	C
From	A	0.00	3.00	205.00
	B	3.00	0.00	10.00
	C	234.00	15.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99
	B	0.23	0.00	0.77
	C	0.94	0.06	0.00

Demand (PCU/TS)

17:00 - 17:15

		To		
		A	B	C
From	A	0.00	3.00	208.00
	B	3.00	0.00	10.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99
	B	0.23	0.00	0.77

	C	244.00	14.00	0.00
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	C	0.95	0.05	0.00
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Demand (PCU/TS)

17:15 - 17:30

		To		
		A	B	C
From	A	0.00	5.00	198.00
	B	0.00	0.00	8.00
	C	219.00	17.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.02	0.98
	B	0.00	0.00	1.00
	C	0.93	0.07	0.00

Vehicle Mix

HV %s

16:30 - 16:45

		To		
		A	B	C
From	A	0	0	7
	B	0	0	0
	C	4	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.070
	B	1.000	1.000	1.000
	C	1.040	1.000	1.000

HV %s

16:45 - 17:00

		To		
		A	B	C
From	A	0	0	5
	B	0	0	0
	C	4	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.047
	B	1.000	1.000	1.000
	C	1.037	1.000	1.000

HV %s

17:00 - 17:15

		To		
		A	B	C
From	A	0	0	3
	B	0	0	0
	C	3	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.030	1.000	1.000

HV %s

17:15 - 17:30

		To		
		A	B	C
From	A	0	0	3
	B	0	0	0
	C	5	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.050	1.000	1.000

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
16:30-16:45	A	228.00	228.00
	B	16.00	16.00
	C	232.00	232.00
16:45-17:00	A	208.00	208.00
	B	13.00	13.00
	C	249.00	249.00
17:00-17:15	A	211.00	211.00
	B	13.00	13.00
	C	258.00	258.00
17:15-17:30	A	203.00	203.00
	B	8.00	8.00
	C	236.00	236.00

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Q (PCU)	Max LOS	Av. Demand (PCU/TS)	Total Junction Arrivals (PCU)
B-AC	0.20	14.23	0.2	B	12.50	50.00
C-AB	0.15	9.65	0.2	A	15.00	60.00
C-A					228.75	915.00
A-B					5.25	21.00
A-C					207.25	829.00

Main Results for each time segment

16:30 - 16:45

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	16.00	16.00	79.43	0.201	15.75	0.0	0.2	14.082	B
C-AB	14.00	14.00	106.94	0.131	13.85	0.0	0.1	9.653	A
C-A	218.00	218.00			218.00				
A-B	10.00	10.00			10.00				
A-C	218.00	218.00			218.00				

16:45 - 17:00

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	13.00	13.00	77.79	0.167	13.04	0.2	0.2	13.909	B
C-AB	15.00	15.00	111.44	0.135	14.99	0.1	0.2	9.332	A
C-A	234.00	234.00			234.00				
A-B	3.00	3.00			3.00				
A-C	205.00	205.00			205.00				

17:00 - 17:15

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	13.00	13.00	76.23	0.171	13.00	0.2	0.2	14.233	B
C-AB	14.00	14.00	110.76	0.126	14.01	0.2	0.1	9.304	A
C-A	244.00	244.00			244.00				
A-B	3.00	3.00			3.00				
A-C	208.00	208.00			208.00				

17:15 - 17:30

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	8.00	8.00	104.90	0.076	8.12	0.2	0.1	9.312	A
C-AB	17.00	17.00	112.56	0.151	16.97	0.1	0.2	9.412	A
C-A	219.00	219.00			219.00				
A-B	5.00	5.00			5.00				
A-C	198.00	198.00			198.00				

2025 Design Sensitivity, AM

Data Errors and Warnings

No errors or warnings

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		1.73	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)	Run automatically
D11	2025 Design Sensitivity	AM	DIRECT	08:30	09:30	60	15	✓

Vehicle mix varies over time	Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	✓	✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Scaling Factor (%)
A		DIRECT	✓	100.000
B		DIRECT	✓	100.000
C		DIRECT	✓	100.000

Origin-Destination Data

Demand (PCU/TS)

08:30 - 08:45

		To		
		A	B	C
From	A	0.00	3.00	201.00
	B	17.00	0.00	26.00
	C	229.00	17.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99
	B	0.40	0.00	0.60
	C	0.93	0.07	0.00

Demand (PCU/TS)

08:45 - 09:00

		To		
		A	B	C
From	A	0.00	0.00	248.00
	B	0.00	0.00	9.00
	C	272.00	6.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.00	1.00
	B	0.00	0.00	1.00
	C	0.98	0.02	0.00

Demand (PCU/TS)

09:00 - 09:15

		To		
		A	B	C
From	A	0.00	4.00	175.00

Proportions

		To		
		A	B	C
From	A	0.00	0.02	0.98

	B	3.00	0.00	11.00
	C	259.00	19.00	0.00

	B	0.21	0.00	0.79
	C	0.93	0.07	0.00

Demand (PCU/TS)

09:15 - 09:30

		To		
From		A	B	C
	A	0.00	13.00	205.00
	B	3.00	0.00	17.00
	C	231.00	14.00	0.00

Proportions

		To		
From		A	B	C
	A	0.00	0.06	0.94
	B	0.15	0.00	0.85
	C	0.94	0.06	0.00

Vehicle Mix

HV %s

08:30 - 08:45

		To		
From		A	B	C
	A	0	0	12
	B	0	0	0
	C	7	0	0

Av. PCU Per Veh

		To		
From		A	B	C
	A	1.000	1.000	1.120
	B	1.000	1.000	1.000
	C	1.070	1.000	1.000

HV %s

08:45 - 09:00

		To		
From		A	B	C
	A	0	0	6
	B	0	0	0
	C	5	18	0

Av. PCU Per Veh

		To		
From		A	B	C
	A	1.000	1.000	1.060
	B	1.000	1.000	1.000
	C	1.050	1.180	1.000

HV %s

09:00 - 09:15

		To		
From		A	B	C
	A	0	0	7
	B	0	0	9
	C	8	0	0

Av. PCU Per Veh

		To		
From		A	B	C
	A	1.000	1.000	1.070
	B	1.000	1.000	1.090
	C	1.080	1.000	1.000

HV %s

09:15 - 09:30

		To		
From		A	B	C
	A	0	0	3
	B	0	0	0
	C	8	7	0

Av. PCU Per Veh

		To		
From		A	B	C
	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.080	1.070	1.000

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
08:30-08:45	A	204.00	204.00
	B	43.00	43.00
	C	246.00	246.00
08:45-09:00	A	248.00	248.00
	B	9.00	9.00
	C	278.00	278.00
09:00-09:15	A	179.00	179.00
	B	14.00	14.00
	C	278.00	278.00
09:15-09:30	A	218.00	218.00
	B	20.00	20.00
	C	245.00	245.00

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Q (PCU)	Max LOS	Av. Demand (PCU/TS)	Total Junction Arrivals (PCU)
B-AC	0.64	33.63	1.6	D	21.50	86.00
C-AB	0.16	9.68	0.2	A	14.00	56.00
C-A					247.75	991.00
A-B					5.00	20.00
A-C					207.25	829.00

Main Results for each time segment

08:30 - 08:45

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	43.00	43.00	66.78	0.644	41.37	0.0	1.6	33.626	D
C-AB	17.00	17.00	112.34	0.151	16.82	0.0	0.2	9.406	A
C-A	229.00	229.00			229.00				
A-B	3.00	3.00			3.00				
A-C	201.00	201.00			201.00				

08:45 - 09:00

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	9.00	9.00	85.24	0.106	10.51	1.6	0.1	12.275	B
C-AB	6.00	6.00	102.44	0.059	6.11	0.2	0.1	9.674	A
C-A	272.00	272.00			272.00				
A-B	0.00	0.00			0.00				
A-C	248.00	248.00			248.00				

09:00 - 09:15

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	14.00	14.00	83.36	0.168	13.91	0.1	0.2	13.407	B
C-AB	19.00	19.00	117.96	0.161	18.87	0.1	0.2	9.480	A
C-A	259.00	259.00			259.00				
A-B	4.00	4.00			4.00				
A-C	175.00	175.00			175.00				

09:15 - 09:30

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	20.00	20.00	84.74	0.236	19.89	0.2	0.3	14.344	B
C-AB	14.00	14.00	109.19	0.128	14.05	0.2	0.2	9.683	A
C-A	231.00	231.00			231.00				
A-B	13.00	13.00			13.00				
A-C	205.00	205.00			205.00				

2025 Design Sensitivity, PM

Data Errors and Warnings

No errors or warnings

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.69	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)	Run automatically
D12	2025 Design Sensitivity	PM	DIRECT	16:30	17:30	60	15	✓

Vehicle mix varies over time	Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	✓	✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Scaling Factor (%)
A		DIRECT	✓	100.000
B		DIRECT	✓	100.000
C		DIRECT	✓	100.000

Origin-Destination Data

Demand (PCU/TS)

16:30 - 16:45

		To		
		A	B	C
From	A	0.00	10.00	222.00
	B	3.00	0.00	13.00
	C	221.00	14.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.04	0.96
	B	0.19	0.00	0.81
	C	0.94	0.06	0.00

Demand (PCU/TS)

16:45 - 17:00

		To		
		A	B	C
From	A	0.00	3.00	209.00
	B	3.00	0.00	10.00
	C	237.00	15.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99
	B	0.23	0.00	0.77
	C	0.94	0.06	0.00

Demand (PCU/TS)

17:00 - 17:15

		To		
		A	B	C
From	A	0.00	3.00	212.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99

	B	3.00	0.00	10.00
	C	247.00	14.00	0.00

	B	0.23	0.00	0.77
	C	0.95	0.05	0.00

Demand (PCU/TS)

17:15 - 17:30

		To		
From		A	B	C
	A	0.00	5.00	202.00
	B	0.00	0.00	8.00
	C	222.00	17.00	0.00

Proportions

		To		
From		A	B	C
	A	0.00	0.02	0.98
	B	0.00	0.00	1.00
	C	0.93	0.07	0.00

Vehicle Mix

HV %s

16:30 - 16:45

		To		
From		A	B	C
	A	0	0	7
	B	0	0	0
	C	4	0	0

Av. PCU Per Veh

		To		
From		A	B	C
	A	1.000	1.000	1.070
	B	1.000	1.000	1.000
	C	1.040	1.000	1.000

HV %s

16:45 - 17:00

		To		
From		A	B	C
	A	0	0	5
	B	0	0	0
	C	4	0	0

Av. PCU Per Veh

		To		
From		A	B	C
	A	1.000	1.000	1.046
	B	1.000	1.000	1.000
	C	1.037	1.000	1.000

HV %s

17:00 - 17:15

		To		
From		A	B	C
	A	0	0	3
	B	0	0	0
	C	3	0	0

Av. PCU Per Veh

		To		
From		A	B	C
	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.030	1.000	1.000

HV %s

17:15 - 17:30

		To		
From		A	B	C
	A	0	0	3
	B	0	0	0
	C	5	0	0

Av. PCU Per Veh

		To		
From		A	B	C
	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.050	1.000	1.000

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
16:30-16:45	A	232.00	232.00
	B	16.00	16.00
	C	235.00	235.00
16:45-17:00	A	212.00	212.00
	B	13.00	13.00
	C	252.00	252.00
17:00-17:15	A	215.00	215.00
	B	13.00	13.00
	C	261.00	261.00
17:15-17:30	A	207.00	207.00
	B	8.00	8.00
	C	239.00	239.00

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Q (PCU)	Max LOS	Av. Demand (PCU/TS)	Total Junction Arrivals (PCU)
B-AC	0.20	14.53	0.3	B	12.50	50.00
C-AB	0.15	9.75	0.2	A	15.00	60.00
C-A					231.75	927.00
A-B					5.25	21.00
A-C					211.25	845.00

Main Results for each time segment

16:30 - 16:45

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	16.00	16.00	78.21	0.205	15.75	0.0	0.3	14.353	B
C-AB	14.00	14.00	106.04	0.132	13.85	0.0	0.2	9.747	A
C-A	221.00	221.00			221.00				
A-B	10.00	10.00			10.00				
A-C	222.00	222.00			222.00				

16:45 - 17:00

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	13.00	13.00	76.53	0.170	13.04	0.3	0.2	14.187	B
C-AB	15.00	15.00	110.54	0.136	14.99	0.2	0.2	9.420	A
C-A	237.00	237.00			237.00				
A-B	3.00	3.00			3.00				
A-C	209.00	209.00			209.00				

17:00 - 17:15

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	13.00	13.00	74.94	0.173	13.00	0.2	0.2	14.529	B
C-AB	14.00	14.00	109.86	0.127	14.01	0.2	0.1	9.391	A
C-A	247.00	247.00			247.00				
A-B	3.00	3.00			3.00				
A-C	212.00	212.00			212.00				

17:15 - 17:30

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	8.00	8.00	104.02	0.077	8.12	0.2	0.1	9.396	A
C-AB	17.00	17.00	111.66	0.152	16.97	0.1	0.2	9.501	A
C-A	222.00	222.00			222.00				
A-B	5.00	5.00			5.00				
A-C	202.00	202.00			202.00				

2033 Design, AM

Data Errors and Warnings

No errors or warnings

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		1.87	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)	Run automatically
D13	2033 Design	AM	DIRECT	08:30	09:30	60	15	✓

Vehicle mix varies over time	Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	✓	✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Scaling Factor (%)
A		DIRECT	✓	100.000
B		DIRECT	✓	100.000
C		DIRECT	✓	100.000

Origin-Destination Data

Demand (PCU/TS)

08:30 - 08:45

		To		
		A	B	C
From	A	0.00	3.00	207.00
	B	17.00	0.00	27.00
	C	237.00	17.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99
	B	0.39	0.00	0.61
	C	0.93	0.07	0.00

Demand (PCU/TS)

08:45 - 09:00

		To		
		A	B	C
From	A	0.00	0.00	256.00
	B	0.00	0.00	9.00
	C	281.00	6.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.00	1.00
	B	0.00	0.00	1.00
	C	0.98	0.02	0.00

Demand (PCU/TS)

09:00 - 09:15

		To		
		A	B	C
From	A	0.00	5.00	180.00
	B	3.00	0.00	11.00

Proportions

		To		
		A	B	C
From	A	0.00	0.03	0.97
	B	0.21	0.00	0.79

	C	268.00	20.00	0.00
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	C	0.93	0.07	0.00
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Demand (PCU/TS)

09:15 - 09:30

		To		
		A	B	C
From	A	0.00	14.00	211.00
	B	3.00	0.00	18.00
	C	239.00	15.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.06	0.94
	B	0.14	0.00	0.86
	C	0.94	0.06	0.00

Vehicle Mix

HV %s

08:30 - 08:45

		To			
		A	B	C	
From	A	0	0	12	
	B	0	0	0	
	C	7	0	0	

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.120
	B	1.000	1.000	1.000
	C	1.071	1.000	1.000

HV %s

08:45 - 09:00

		To		
		A	B	C
From	A	0	0	6
	B	0	0	0
	C	5	17	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.060
	B	1.000	1.000	1.000
	C	1.050	1.170	1.000

HV %s

09:00 - 09:15

		To		
		A	B	C
From	A	0	0	7
	B	0	0	9
	C	8	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.070
	B	1.000	1.000	1.090
	C	1.080	1.000	1.000

HV %s

09:15 - 09:30

		To		
		A	B	C
From	A	0	0	3
	B	0	0	0
	C	8	7	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.080	1.070	1.000

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
08:30-08:45	A	210.00	210.00
	B	44.00	44.00
	C	254.00	254.00
08:45-09:00	A	256.00	256.00
	B	9.00	9.00
	C	287.00	287.00
09:00-09:15	A	185.00	185.00
	B	14.00	14.00
	C	288.00	288.00
09:15-09:30	A	225.00	225.00
	B	21.00	21.00
	C	254.00	254.00

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Q (PCU)	Max LOS	Av. Demand (PCU/TS)	Total Junction Arrivals (PCU)
B-AC	0.68	37.03	1.8	E	22.00	88.00
C-AB	0.17	9.95	0.2	A	14.50	58.00
C-A					256.25	1025.00
A-B					5.50	22.00
A-C					213.50	854.00

Main Results for each time segment

08:30 - 08:45

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	44.00	44.00	64.96	0.677	42.15	0.0	1.8	37.033	E
C-AB	17.00	17.00	110.99	0.153	16.82	0.0	0.2	9.539	A
C-A	237.00	237.00			237.00				
A-B	3.00	3.00			3.00				
A-C	207.00	207.00			207.00				

08:45 - 09:00

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	9.00	9.00	81.93	0.110	10.72	1.8	0.1	12.932	B
C-AB	6.00	6.00	100.64	0.060	6.11	0.2	0.1	9.840	A
C-A	281.00	281.00			281.00				
A-B	0.00	0.00			0.00				
A-C	256.00	256.00			256.00				

09:00 - 09:15

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	14.00	14.00	80.94	0.173	13.91	0.1	0.2	13.889	B
C-AB	20.00	20.00	116.62	0.172	19.85	0.1	0.2	9.675	A
C-A	268.00	268.00			268.00				
A-B	5.00	5.00			5.00				
A-C	180.00	180.00			180.00				

09:15 - 09:30

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	21.00	21.00	83.15	0.253	20.87	0.2	0.3	14.930	B
C-AB	15.00	15.00	107.61	0.139	15.05	0.2	0.2	9.948	A
C-A	239.00	239.00			239.00				
A-B	14.00	14.00			14.00				
A-C	211.00	211.00			211.00				

2033 Design, PM

Data Errors and Warnings

No errors or warnings

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.70	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)	Run automatically
D14	2033 Design	PM	DIRECT	16:30	17:30	60	15	✓

Vehicle mix varies over time	Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	✓	✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Scaling Factor (%)
A		DIRECT	✓	100.000
B		DIRECT	✓	100.000
C		DIRECT	✓	100.000

Origin-Destination Data

Demand (PCU/TS)

16:30 - 16:45

		To		
		A	B	C
From	A	0.00	11.00	232.00
	B	3.00	0.00	13.00
	C	232.00	14.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.05	0.95
	B	0.19	0.00	0.81
	C	0.94	0.06	0.00

Demand (PCU/TS)

16:45 - 17:00

		To		
		A	B	C
From	A	0.00	3.00	218.00
	B	3.00	0.00	10.00
	C	249.00	16.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99
	B	0.23	0.00	0.77
	C	0.94	0.06	0.00

Demand (PCU/TS)

17:00 - 17:15

		To		
		A	B	C
From	A	0.00	3.00	221.00
	B	3.00	0.00	10.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99
	B	0.23	0.00	0.77

	C	259.00	14.00	0.00
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	C	0.95	0.05	0.00
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Demand (PCU/TS)

17:15 - 17:30

		To		
		A	B	C
From	A	0.00	5.00	211.00
	B	0.00	0.00	8.00
	C	233.00	18.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.02	0.98
	B	0.00	0.00	1.00
	C	0.93	0.07	0.00

Vehicle Mix

HV %s

16:30 - 16:45

		To		
		A	B	C
From	A	0	0	7
	B	0	0	0
	C	4	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.070
	B	1.000	1.000	1.000
	C	1.040	1.000	1.000

HV %s

16:45 - 17:00

		To		
		A	B	C
From	A	0	0	5
	B	0	0	0
	C	4	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.050
	B	1.000	1.000	1.000
	C	1.037	1.000	1.000

HV %s

17:00 - 17:15

		To		
		A	B	C
From	A	0	0	3
	B	0	0	0
	C	3	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.030	1.000	1.000

HV %s

17:15 - 17:30

		To		
		A	B	C
From	A	0	0	3
	B	0	0	0
	C	5	0	0

Av. PCU Per Veh

		To		
		A	B	C
From	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.050	1.000	1.000

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
16:30-16:45	A	243.00	243.00
	B	16.00	16.00
	C	246.00	246.00
16:45-17:00	A	221.00	221.00
	B	13.00	13.00
	C	265.00	265.00
17:00-17:15	A	224.00	224.00
	B	13.00	13.00
	C	273.00	273.00
17:15-17:30	A	216.00	216.00
	B	8.00	8.00
	C	251.00	251.00

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Q (PCU)	Max LOS	Av. Demand (PCU/TS)	Total Junction Arrivals (PCU)
B-AC	0.21	15.41	0.3	C	12.50	50.00
C-AB	0.16	10.01	0.2	B	15.50	62.00
C-A					243.25	973.00
A-B					5.50	22.00
A-C					220.50	882.00

Main Results for each time segment

16:30 - 16:45

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	16.00	16.00	74.66	0.214	15.73	0.0	0.3	15.207	C
C-AB	14.00	14.00	103.56	0.135	13.85	0.0	0.2	10.014	B
C-A	232.00	232.00			232.00				
A-B	11.00	11.00			11.00				
A-C	232.00	232.00			232.00				

16:45 - 17:00

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	13.00	13.00	72.83	0.178	13.05	0.3	0.2	15.067	C
C-AB	16.00	16.00	108.51	0.147	15.98	0.2	0.2	9.723	A
C-A	249.00	249.00			249.00				
A-B	3.00	3.00			3.00				
A-C	218.00	218.00			218.00				

17:00 - 17:15

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	13.00	13.00	71.40	0.182	13.00	0.2	0.2	15.410	C
C-AB	14.00	14.00	107.84	0.130	14.02	0.2	0.2	9.596	A
C-A	259.00	259.00			259.00				
A-B	3.00	3.00			3.00				
A-C	221.00	221.00			221.00				

17:15 - 17:30

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	8.00	8.00	102.00	0.078	8.14	0.2	0.1	9.601	A
C-AB	18.00	18.00	109.64	0.164	17.96	0.2	0.2	9.813	A
C-A	233.00	233.00			233.00				
A-B	5.00	5.00			5.00				
A-C	211.00	211.00			211.00				

2033 Design Sensitivity, AM

Data Errors and Warnings

No errors or warnings

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		1.96	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)	Run automatically
D15	2033 Design Sensitivity	AM	DIRECT	08:30	09:30	60	15	✓

Vehicle mix varies over time	Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	✓	✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Scaling Factor (%)
A		DIRECT	✓	100.000
B		DIRECT	✓	100.000
C		DIRECT	✓	100.000

Origin-Destination Data

Demand (PCU/TS)

08:30 - 08:45

		To		
		A	B	C
From	A	0.00	3.00	213.00
	B	17.00	0.00	27.00
	C	243.00	17.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99
	B	0.39	0.00	0.61
	C	0.93	0.07	0.00

Demand (PCU/TS)

08:45 - 09:00

		To		
		A	B	C
From	A	0.00	0.00	263.00
	B	0.00	0.00	9.00
	C	289.00	6.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.00	1.00
	B	0.00	0.00	1.00
	C	0.98	0.02	0.00

Demand (PCU/TS)

09:00 - 09:15

		To		
		A	B	C
From	A	0.00	5.00	186.00
	B			
	C			

Proportions

		To		
		A	B	C
From	A	0.00	0.03	0.97
	B			
	C			

	B	3.00	0.00	11.00
	C	275.00	20.00	0.00

	B	0.21	0.00	0.79
	C	0.93	0.07	0.00

Demand (PCU/TS)

09:15 - 09:30

		To		
From		A	B	C
	A	0.00	14.00	218.00
	B	3.00	0.00	18.00
	C	245.00	15.00	0.00

Proportions

		To		
From		A	B	C
	A	0.00	0.06	0.94
	B	0.14	0.00	0.86
	C	0.94	0.06	0.00

Vehicle Mix

HV %s

08:30 - 08:45

		To		
From		A	B	C
	A	0	0	12
	B	0	0	0
	C	7	0	0

Av. PCU Per Veh

		To		
From		A	B	C
	A	1.000	1.000	1.120
	B	1.000	1.000	1.000
	C	1.070	1.000	1.000

HV %s

08:45 - 09:00

		To		
From		A	B	C
	A	0	0	6
	B	0	0	0
	C	5	17	0

Av. PCU Per Veh

		To		
From		A	B	C
	A	1.000	1.000	1.060
	B	1.000	1.000	1.000
	C	1.050	1.170	1.000

HV %s

09:00 - 09:15

		To		
From		A	B	C
	A	0	0	7
	B	0	0	9
	C	8	0	0

Av. PCU Per Veh

		To		
From		A	B	C
	A	1.000	1.000	1.070
	B	1.000	1.000	1.090
	C	1.080	1.000	1.000

HV %s

09:15 - 09:30

		To		
From		A	B	C
	A	0	0	3
	B	0	0	0
	C	8	7	0

Av. PCU Per Veh

		To		
From		A	B	C
	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.080	1.070	1.000

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
08:30-08:45	A	216.00	216.00
	B	44.00	44.00
	C	260.00	260.00
08:45-09:00	A	263.00	263.00
	B	9.00	9.00
	C	295.00	295.00
09:00-09:15	A	191.00	191.00
	B	14.00	14.00
	C	295.00	295.00
09:15-09:30	A	232.00	232.00
	B	21.00	21.00
	C	260.00	260.00

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Q (PCU)	Max LOS	Av. Demand (PCU/TS)	Total Junction Arrivals (PCU)
B-AC	0.70	40.21	2.0	E	22.00	88.00
C-AB	0.17	10.12	0.2	B	14.50	58.00
C-A					263.00	1052.00
A-B					5.50	22.00
A-C					220.00	880.00

Main Results for each time segment

08:30 - 08:45

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	44.00	44.00	62.78	0.701	41.98	0.0	2.0	40.208	E
C-AB	17.00	17.00	109.64	0.155	16.82	0.0	0.2	9.678	A
C-A	243.00	243.00			243.00				
A-B	3.00	3.00			3.00				
A-C	213.00	213.00			213.00				

08:45 - 09:00

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	9.00	9.00	78.79	0.114	10.89	2.0	0.1	13.607	B
C-AB	6.00	6.00	99.06	0.061	6.11	0.2	0.1	10.006	B
C-A	289.00	289.00			289.00				
A-B	0.00	0.00			0.00				
A-C	263.00	263.00			263.00				

09:00 - 09:15

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	14.00	14.00	78.73	0.178	13.91	0.1	0.2	14.359	B
C-AB	20.00	20.00	115.27	0.174	19.85	0.1	0.2	9.813	A
C-A	275.00	275.00			275.00				
A-B	5.00	5.00			5.00				
A-C	186.00	186.00			186.00				

09:15 - 09:30

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	21.00	21.00	80.95	0.259	20.86	0.2	0.4	15.476	C
C-AB	15.00	15.00	106.04	0.141	15.05	0.2	0.2	10.121	B
C-A	245.00	245.00			245.00				
A-B	14.00	14.00			14.00				
A-C	218.00	218.00			218.00				

2033 Design Sensitivity, PM

Data Errors and Warnings

No errors or warnings

Junction Network

Junctions

Junction	Name	Junction type	Major road direction	Use circulating lanes	Junction Delay (s)	Junction LOS
1	untitled	T-Junction	Two-way		0.70	A

Junction Network Options

Driving side	Lighting
Left	Normal/unknown

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)	Run automatically
D16	2033 Design Sensitivity	PM	DIRECT	16:30	17:30	60	15	✓

Vehicle mix varies over time	Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)	O-D data varies over time
✓	✓	✓	HV Percentages	2.00	✓

Demand overview (Traffic)

Arm	Linked arm	Profile type	Use O-D data	Scaling Factor (%)
A		DIRECT	✓	100.000
B		DIRECT	✓	100.000
C		DIRECT	✓	100.000

Origin-Destination Data

Demand (PCU/TS)

16:30 - 16:45

		To		
		A	B	C
From	A	0.00	11.00	237.00
	B	3.00	0.00	13.00
	C	235.00	14.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.04	0.96
	B	0.19	0.00	0.81
	C	0.94	0.06	0.00

Demand (PCU/TS)

16:45 - 17:00

		To		
		A	B	C
From	A	0.00	3.00	222.00
	B	3.00	0.00	10.00
	C	252.00	16.00	0.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99
	B	0.23	0.00	0.77
	C	0.94	0.06	0.00

Demand (PCU/TS)

17:00 - 17:15

		To		
		A	B	C
From	A	0.00	3.00	226.00

Proportions

		To		
		A	B	C
From	A	0.00	0.01	0.99

	B	3.00	0.00	10.00
	C	263.00	14.00	0.00

	B	0.23	0.00	0.77
	C	0.95	0.05	0.00

Demand (PCU/TS)

17:15 - 17:30

		To		
From		A	B	C
	A	0.00	5.00	215.00
	B	0.00	0.00	8.00
	C	236.00	18.00	0.00

Proportions

		To		
From		A	B	C
	A	0.00	0.02	0.98
	B	0.00	0.00	1.00
	C	0.93	0.07	0.00

Vehicle Mix

HV %s

16:30 - 16:45

		To		
From		A	B	C
	A	0	0	7
	B	0	0	0
	C	3	0	0

Av. PCU Per Veh

		To		
From		A	B	C
	A	1.000	1.000	1.070
	B	1.000	1.000	1.000
	C	1.030	1.000	1.000

HV %s

16:45 - 17:00

		To		
From		A	B	C
	A	0	0	5
	B	0	0	0
	C	4	0	0

Av. PCU Per Veh

		To		
From		A	B	C
	A	1.000	1.000	1.047
	B	1.000	1.000	1.000
	C	1.037	1.000	1.000

HV %s

17:00 - 17:15

		To		
From		A	B	C
	A	0	0	3
	B	0	0	0
	C	2	0	0

Av. PCU Per Veh

		To		
From		A	B	C
	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.020	1.000	1.000

HV %s

17:15 - 17:30

		To		
From		A	B	C
	A	0	0	3
	B	0	0	0
	C	5	0	0

Av. PCU Per Veh

		To		
From		A	B	C
	A	1.000	1.000	1.030
	B	1.000	1.000	1.000
	C	1.050	1.000	1.000

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/TS)	Demand in PCU (PCU/TS)
16:30-16:45	A	248.00	248.00
	B	16.00	16.00
	C	249.00	249.00
16:45-17:00	A	225.00	225.00
	B	13.00	13.00
	C	268.00	268.00
17:00-17:15	A	229.00	229.00
	B	13.00	13.00
	C	277.00	277.00
17:15-17:30	A	220.00	220.00
	B	8.00	8.00
	C	254.00	254.00

Results

Results Summary for whole modelled period

Stream	Max RFC	Max Delay (s)	Max Q (PCU)	Max LOS	Av. Demand (PCU/TS)	Total Junction Arrivals (PCU)
B-AC	0.22	15.88	0.3	C	12.50	50.00
C-AB	0.17	10.14	0.2	B	15.50	62.00
C-A					246.50	986.00
A-B					5.50	22.00
A-C					225.00	900.00

Main Results for each time segment

16:30 - 16:45

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	16.00	16.00	73.12	0.219	15.73	0.0	0.3	15.610	C
C-AB	14.00	14.00	102.44	0.137	13.84	0.0	0.2	10.142	B
C-A	235.00	235.00			235.00				
A-B	11.00	11.00			11.00				
A-C	237.00	237.00			237.00				

16:45 - 17:00

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	13.00	13.00	71.50	0.182	13.05	0.3	0.2	15.412	C
C-AB	16.00	16.00	107.61	0.149	15.98	0.2	0.2	9.818	A
C-A	252.00	252.00			252.00				
A-B	3.00	3.00			3.00				
A-C	222.00	222.00			222.00				

17:00 - 17:15

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	13.00	13.00	69.67	0.187	13.00	0.2	0.2	15.880	C
C-AB	14.00	14.00	106.71	0.131	14.02	0.2	0.2	9.711	A
C-A	263.00	263.00			263.00				
A-B	3.00	3.00			3.00				
A-C	226.00	226.00			226.00				

17:15 - 17:30

Stream	Total Demand (PCU/TS)	Junction Arrivals (PCU)	Capacity (PCU/TS)	RFC	Throughput (PCU/TS)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
B-AC	8.00	8.00	101.11	0.079	8.14	0.2	0.1	9.694	A
C-AB	18.00	18.00	108.74	0.166	17.96	0.2	0.2	9.908	A
C-A	236.00	236.00			236.00				
A-B	5.00	5.00			5.00				
A-C	215.00	215.00			215.00				

Appendix J Scoping Note



Place Directorate
David Shepherd
Culture, Regeneration and Property
PO Box 634, Barnsley, S70 9FE
Development Management
Head of Service: Joe Jenkinson

Emma Winter
Carter Jonas LLP
9 Bond Court
Leeds
LS1 2JZ

My Ref: 2019/ENQ/00871
Date: 11/03/2020
Enquiries to: Richard Gilbert
Direct Dial: 01226 772108
E-Mail: richardgilbert@barnsley.gov.uk

Dear Ms Winter,

Proposal Description: Residential development of approximately 130 dwellings and associated works.

Location: Land at Hay Green Lane, Birdwell, Barnsley, S70 5XE

The following content constitutes the formal response to your pre-application enquiry following assessment of your proposal by the LPA, consultees and a meeting with yourselves on the 13th January 2020 at the council offices at Westgate Plaza One. The format of the response will first outline the details of the consultee responses and will then address the points of clarification requested within your covering letter (14th November 2019).

Planning Policy Context

Planning decisions should be made in accordance with the development plan unless material considerations indicate otherwise and the NPPF does not change the statutory status of the development plan as the starting point for decision making. The Local Plan is the Borough's statutory development plan which is supported by a series of adopted Supplementary Planning Documents and Supplementary Planning Guidance Notes, which are other material considerations.

Local Plan

The new Local Plan was adopted at the Full Council meeting held 3rd January 2019 after it was found to be sound by the appointed Planning Inspector following the examination process. This means that it now takes on full weight for decision making process in planning law terms as the development plan for the Borough, superseding the remaining saved policies from the Unitary Development Plan (adopted in the year 2000) and the Core Strategy (adopted in 2011). Though not exclusively relevant to this proposal, the following policies are most relevant and the applicant should make themselves aware of their contents: GD1 – General Development, D1 – High Quality Design and Placemaking, H1 – The Number of New Homes to Be Built, H2 – The Distribution of New Homes, H3 – Uses on Allocated Housing Sites, H6 – Housing Mix and Efficient Use of Land and H7 – Affordable Housing.

Local Plan Site Policy – HS59

Indicative number of dwellings – 118

The development will be expected to:

- Produce a detailed ecology report in support of any development proposal;
- Respect the historic setting of the listed barn opposite Herons Way by retaining the existing mature field boundary with its existing hedge and trees to the east, and by the use of appropriate site layout, sympathetic design that reflects the setting, scaling, massing, details and materials; and
- Provide appropriate access and off site highway works.

Archaeological remains may be present on this site therefore proposals must be accompanied by an appropriate archaeological assessment (including a field evaluation if necessary) that must include the following:

- Information identifying the likely location and extent of the remains, and the nature of the remains;
- An assessment of the significance of the remains; and
- Consideration of how the remains would be affected by the proposed development.

Site Context

HS59 is a greenfield site on the eastern flank of Birdwell which is a part of Hoyland Principal Town. Hay Green Lane is a residential street that stretches directly east of the A61 trunk road that connects Junction 36 of the M1 to Worsbrough and the greater Urban Barnsley area. The development of Birdwell appears to have occurred in the late 19th Century and early 20th Century on Chapel Street with subsequent expansion along Sheffield Road in the following decades as a consequence of ribbon development. A major increase to the conurbation's size occurred in the 1950s/60s with the development of residential areas across Worsbrough Road and Heron's Way that now forms the majority of Birdwell's settlement layout. Hay Green Lane itself is constituted of a mix of terraced and detached houses with on-street parking being prominent where the former are located. A primary school is also located at the access onto Hay Green Lane from Sheffield Road with a Grade II listed barn further east upon Hay Green Lane.

The site itself is formed of arable land across its eastern section while its western flank is composed of allotments. A district level park, Birdwell Recreation Ground, sits at HS59's south western corner while a mix of residential curtilages and the highway of Hay Green Lane border the site to the north and west. The remainder of HS59 is contained by farmland across its eastern and southern boundaries which is designated as Safeguarded Land in the Local Plan Policies Map (SL4).

Several significant development schemes are evolving within Hoyland Principal Town, the closest being the Hoyland North and Hoyland West masterplan sites that will bring forward significant levels of commercial and residential development to the south of HS59.

Consultee Responses

Affordable Housing

- The site is in an area of the Borough that requires 10% of the units on the site to be affordable. The tenure split is 80% affordable homes for rent and 20% affordable home ownership. 13 units would be expected to be delivered on the basis of the current proposal though it is acceded that this is likely to be reduced in line with the yield of the allocation site for HS59 in the Local Plan (118 indicative units).
- An affordable housing statement will be required to be submitted, which requires the following:
 - Total number of residential units proposed
 - The percentage of affordable housing units proposed and how this relates to the requirement set out in Local Plan Policy H8 and Affordable Housing SPD
 - The mix of units (bedroom numbers and unit sizes (sqm)) and justification
 - Tenure of each unit and justification
 - Plot number(s) of affordable units

- A layout plan with the location of each affordable unit highlighted, with a key showing the unit type and justification for the proposed location(s)
 - Details of design and proposed quality standards of build
 - Car parking space(s) per affordable unit
 - Details of any Registered Provider acting as a partner in the development
 - Proposed ongoing housing management i.e. transfer to a Registered Provider on BMBC's Framework
 - Anticipated OMV and transfer value of each affordable property (if applicable)
 - Proposed phasing of delivery and occupation on sites to be delivered over more than one phase
 - The arrangements to ensure that the provision is affordable for both first and future occupiers or if not possible for the subsidy to be recycled for alternative affordable housing provision
 - **Viability Assessment:** if provision is proposed that is non-compliant with policy. Note that it is the Council's policy to ensure that viability assessments are reviewed independently prior to arriving at a conclusion on the findings. The Council shall recover the fees associated with this work from applicants.
 - Developers should seek to engage with Registered Providers and the Council's Housing and Energy Team at an early stage.
- The design of the houses should meet the minimum space standards under the South Yorkshire Residential Design Guide. Be indistinguishable in their layout from the market housing on the site, be composed of the same materials and specifications internally and externally while being distributed evenly across the site in small clusters of no more than three dwellings. These characteristics will ensure transfer to a Registered Provider.

Air Quality

- The proposed development is adjacent to the A61 Sheffield Road. Whilst not an air quality management area (AQMA), this road is detailed within the Barnsley MBC Air Quality and Emissions Good Practice Planning Guidance (<https://www.barnsley.gov.uk/media/9219/pdc-2018-sep-bmbc-aqande-technical-planning-guidance-v11.pdf>), where mitigation of air quality impact is recommended. An assessment shall therefore be undertaken in accordance with the attached guidance and subsequent mitigation proposed.
- The proposed development would be classed as "medium" with reference to the Barnsley MBC Air Quality and Emissions Good Practice Planning Guidance.

Biodiversity

- Requirement for a phase one habitat survey to be submitted. (Though Joel Gandhi has indicated that one is being carried out in correspondence dated 13/01/2020).
- The survey should have regard to the Preliminary Ecological Appraisals conducted for SL4 surrounding the allocation to the south and east.
- I note that the revised illustrative masterplan received on 30th January 2020 (No. 17 5085 12) retains the ash and oak tree as well as the hedgerows on the site's boundaries.
- Any removal of hedgerows on the site is to be subject to assessment under the Hedgerow Regulations 1997 including heritage criteria
- 10% Biodiversity Net Gain required under a recognised metric (Warwickshire, Defra etc). There is the potential for this to be brought forward via the undevelopable area crossed by powerlines to the south east within the HS59 allocation boundary. The net gain will not be accepted on the Safeguarded Land area previously proposed.

Forestry

- The Tree Officer has assessed the submission of the Tree Survey Schedule and Plan conducted on the 22nd November 2019. As stated above, I expect the Ash tree (Grade C) on the eastern boundary of the allocation as well as the Oak (Grade B) in the western section to be retained. Irrespectively an AIA and method statement are required for the trees and hedges on site and further comments cannot be produced without a more detailed layout plan being submitted which shows the locations of specific dwellings as well as a revised road layout.

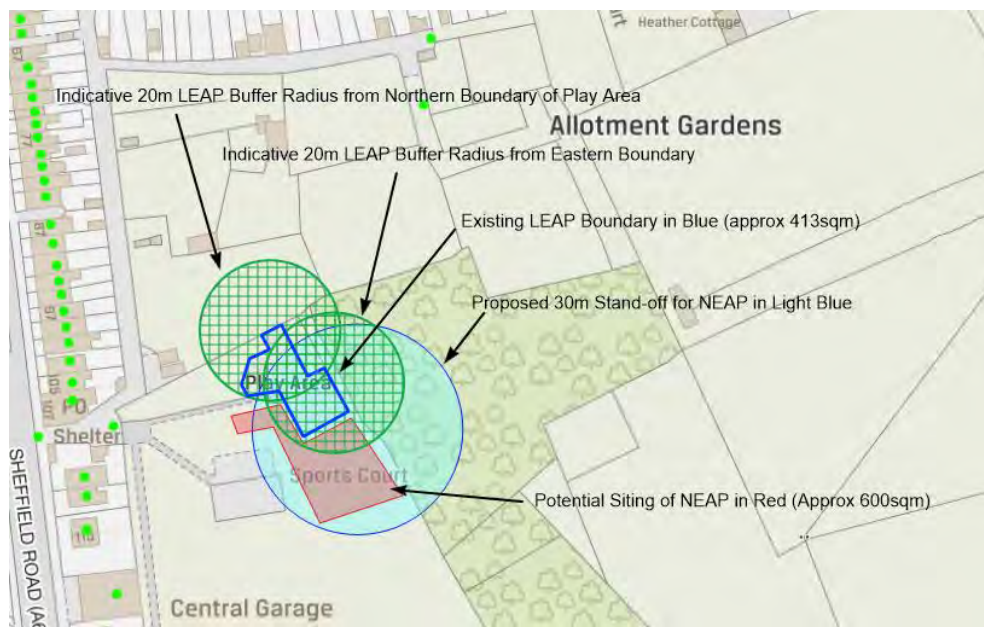
Parks

- The LPA's position in relation to the POS has been provided to yourself on the 30th January 2020. It stated the following:

The council's position on the site is that it is located next to a district level park and the allocation does not therefore require on-site open space provision of 15%. Instead the LPA wish for compliance with guidance in relation to the stand-off distances to the existing LEAP and proposed NEAP in Birdwell Recreation Ground. The areas adjacent to the play areas will form a part of the park and the council require a full off-site contribution towards facilities within the park, specifically upgrade of the play area to NEAP and new changing facilities for the playing pitches. This contribution is in lieu of meeting the 15% on-site public open space provision and parks have agreed to reduce the separation distance to the NEAP from 40m to 30m to allow a larger developable area. The bullet points and Figure 1 below provide a summary of the requirements:

- Adherence to separation distances between boundary of the play areas to boundaries of the proposed dwellings – LEAP 20m and NEAP 30m.
- Undevelopable land in south east corner to be used for SUDS and Biodiversity due to overhead powerlines.
- Full off-site contribution in lieu of full on-site provision of open space (15%).

Fig. 1



Please note that the NEAP standoff in Figure 1 is taken from the north eastern corner and is not reflective of the full stand-off required across the entire eastern boundary of the NEAP – this also relates to the LEAP isochrones indicated in green. Please ensure that the sketch plan is amended so that the NEAP and off-site contribution are referenced.

- The off-site POS calculation is dependent on the housing mix of the site, as this has not been provided yet, we are unable to make the contribution calculation.

Conservation Officer & South Yorkshire Architectural Service (SYAS)

- The development is adjacent to a Grade II listed Barn (ID 1315079). The north western section of SL4 provides a stand-off distance to the Barn and the proposal should maintain the field boundary nearest to the Barn. The dwellings near the Barn should be sympathetic to the scale, mass, detailing and materials of the listed building in line with the HS59's policy wording.
- A Heritage Statement will need to be produced in line with the SPD Heritage Impact Assessment.
- SYAS have reviewed the desk based archaeological assessment (DBA) conducted by Prospect Archaeology and received by the LPA on 13th January 2020. They have made the following comments:

The DBA is a thorough and well researched report and SYAS agree with its conclusions. The report found that for the last few hundred years, settlement has focussed around the farms at Hay Green. There may be some archaeological potential, in connection with this phase, where the application area fronts onto Hay Green Road to the north. Before this period, little information is available to assess the archaeological potential and the DBA recommends further work to evaluate the archaeological potential of the site. Again, SYAS agree with this and recommend that, in the first instance, a geophysical survey of the site is undertaken and the results submitted as supporting information with any future application. Based on the results of the geophysical survey, I will take a view on whether trial trenching is required pre- or post-determination. The survey will also help to confirm the extent of previous ground disturbance, such as open casting, which can then be excluded from further archaeological consideration. Early mining remains, such as bell pits and shaft mounds, remain of archaeological interest.

- On the basis of the above, a Geophysical Survey of the site will need to be carried out to determine if trial trenching is required.
- The DBA also recommends that the eastern boundary is improved with more natural screening, i.e. trees, hedges etc.

BMBC Highway Drainage & Yorkshire Water

- A Flood Risk Assessment, conceptual Drainage Plan and SUDS Design Statement will be required as a part of a submitted application.
- The Council have no records of any culverted or open watercourses crossing the site and are not aware of any flooding issues associated with the site, and would confirm that it is not affected by any flood plains from major watercourses in the area.
- There should be no increase in surface water runoff from the new development. NPPF recognizes that the management of flood risk is not simply restricted to flood plains and that a catchment-wide approach should be employed.
- The nearest combined sewer is in nearby Hay Green Lane. The developer should contact Yorkshire Water if they wish to discharge to this sewer to discuss allowable discharge rates.
- Any balancing facility should be designed to accommodate a 1 in 30 year flow from the site below ground and a 1 in 100 year flow retained within the site (including an allowance of 30% for climate change), without causing any flooding to buildings.

- There are alternatives to conventional storage for the control of surface water run-off that are favoured by the authority where ground conditions are suitable. Sustainable Urban Drainage techniques (SUD's) tackle surface water run-off problems at source using features such as soakaways, permeable pavements, grassed swales, infiltration trenches, ponds and wetlands to attenuate flood peak flows, produce water quality improvements and environmental enhancements.
- The authority seeks to promote the use of SUD's techniques to this site and the authority expects the developer of the site to submit detailed investigations such that the use of SUD's has been fully explored.
- The undevelopable area in the south east corner of the site could potentially incorporate the drainage attenuation with the biodiversity gain mentioned above. The revised illustrative masterplan (30th January 2020 No. 17 5085 12) makes reference to this aspect as well as the incorporation of a swale beside the road.
- With respect of Waste Water, YW provided the following comments:

It would be advisable for the agent/applicant to make an enquiry, as there is public sewerage infrastructure recorded running¹ through the site.

Information regarding drainage to public sewer would be deemed as a formal Pre Planning Enquiry request. The developer should note the following:

Our charges for this service are £164.00 + VAT for each site. If historic sewer flooding information is required, there will be an additional charge of £164.00 + VAT.

The developer should provide the following, if available:

- *Ordnance Survey plan scale 1:1250 or 1:2500 with the site boundary is clearly marked*
- *type of development and number of units proposed*
- *anticipated rates of discharge in litres per second of foul and surface water*
- *high and low points of the site and watersheds*
- *general direction of fall*
- *preferred drainage outfall routes and proposed point of access*
- *a copy of any topographical survey (if available)*

Our comments will be based on a desk top study. The study will be in respect of the public sewer network only. It will include comments on the availability of public sewers and a complimentary extract from the statutory sewer map. We will respond within 10 working days of receipt of payment.

- A foul drainage assessment will be required if the development is not connected to the public sewer and the YW new supplies team will need to be contacted in relation to the water supply connection.

Pollution Control

An Environmental Health Officer has conducted a visit to the site and has noted that the sound level emanating from the Dearne Valley Parkway has the potential to cause a nuisance to future residential dwellings. As such a Noise Assessment should be conducted by suitably qualified acoustician. It should outline the existing noise environment, the potential noise sources from the development, or the noise sources likely to affect the development, together with any mitigation measures.

It is important that the methodology and scope of such an assessment be discussed with Regulatory Services prior to commencement. Further guidance is included in the following:

¹ With respect of the underlined sections, a distinction is drawn between public sewerage infrastructure and culverted or open watercourses.

- BS 7445(2003) Description and measurement of environmental noise
- BS8233(2014) Sound insulation and noise reduction for buildings
- WHO Guidance on Community Noise
- BS5228(2009) Construction Noise and Vibration

Highways DC

Highways have made the following comments in respect of the application:

Existing Highway Network:

- Hay Green Lane – Limited width highway with demand for on street parking reducing the available carriageway to effectively a single lane. History of complaints from residents and records of payments for H markings to highlight private access and deter obstructive parking.
- Planning application [2015/0393](#) involves minor residential development (3 no. flats) and whilst not of concern in terms of highway capacity should be borne in mind in relation to the existing/proposed requirements for VAC around this section of A61 (it may need to amend the current dropped kerb location).

Scope of Work / Data Collection / Traffic Assessment

- Recommended that parking surveys are undertaken along Hay Green Lane, from A61 to the furthest development access. Surveys to 12 hour period and record length and purpose of stay (purpose being residential, school drop off, school (teacher). Include on highway and parking bay locations.
- Traffic survey data for A61/Hay Green Lane junction to include 2 no. private access points to Birdwell Venue (opposite school/within school keep clear markings). Also to record school crossing patrol demand / operation. An informal agreement currently permits parents to use the car park to the rear of The Birdwell Venue for drop off and collection. I believe SCP operating times are 08:25 until 08:55 in the morning & 15:15 until 15:45 in the afternoon.
- Additional traffic data recommended for the A61/Chapel Street junction, including pedestrian demand at the southern arm A61 zebra crossing and frequency/dwell time of bus stop at Travellers Inn. Whilst not of concern in terms of additional vehicle turning movements, any additional demand at the zebra crossing and any direct relationship between the two junctions may need to be considered at application stage.

Trip Generation and Distribution

Use of the Hoyland North Masterplan TAAR trip generations cannot be fully accepted at this time. HDC are working towards clarification of these figures due to recently identified issues. Use of existing Hay Green Lane junction turning proportions would require additional supporting information. A number of planned future developments at both Hoyland North and Hoyland West masterplan sites would bring a significant increase in employment opportunities to the south of the proposed site within the expected future assessment timescales. This could have an impact on existing turning movements at the A61 Sheffield Road/Hay Green Lane junction and as such evidence should be provided as to how this is considered in distributions to/from the proposed site.

Consented / Committed Development

Whilst there are no significant approved developments to note in terms of future highway capacity assessments, both the Hoyland North and Hoyland West masterplan sites to the south of the proposed development could have a realistic impact on through movements along the A61 within the proposed development assessment period. The potential impact should be considered as this may have a direct impact on the safe operation of the A61 Sheffield Road.

Layout and Access

HDC would not support any proposed 'emergency access'. Either an all movements junction or a pedestrian/cycle link into the site would be required. Vehicular access restricted for emergency services vehicles only cannot be controlled or enforced with any reasonable degree of certainty.

The remaining comments summarise some key elements required to achieved a compliant design. This is not exhaustive.

The design of residential streets within the development shall follow the guiding principles and technical guidance contained within the South Yorkshire Residential Design Guide (SYRDG). This can be accessed at: <https://www.barnsley.gov.uk/media/4657/eb131-south-yorkshire-residential-guide.pdf>

Please note Section 4 – Technical Requirements – 4B Site and Parking Geometry particularly (but not exclusively)

B1.1 Street Types

B1.2.1 Design Speed – Residential Streets

B.1.3 Forward Visibility

B1.6.5/B1.6.6 Emergency Vehicle Access

B.2.1.5 Minimum carriageway / vehicle track space width

B.3.7 Turning Areas – Note Cul-de-sacs longer than 20m require a turning area suitable for the turning requirements of a refuse vehicle

B.4 Speed Restraint – Individual speed controlling features for following design speeds are required at these spacing's:

- Streets principally serving residential developments are to be designed to achieve vehicle speeds of no more than 20mph. Shared space streets should be designed to achieve vehicle speeds of around 10mph and always below 15mph. For streets with design speeds of 20mph, speed controlling measures should generally be provided at 70m intervals. Less than 20mph design speed will generally require speed reducing features at 40m intervals. Typical features utilised for speed controlling measures are identified in section 4B .4.1.3 of the SYRDG.

Swept Path Requirements:

- Refuse Vehicle – Dennis 10.3m rear steer (For information the refuse vehicles currently in use in Barnsley has a wheelie Bin Lifting facility and measures 10.3m x 2.5m.)
- Car – Large Car (2006)
- Track tests are to be carried out in accordance with the requirements contained within the South Yorkshire Residential Design Guide sections 4b.2.1.28-29 and 4B.3.3.9-4B.3.3.12.

Parking Requirements: The minimum level of parking should be in line with the parking standards contained within the Councils Parking SPD. These being:

- 1-2 bed units; 1 allocated spaces per dwelling,
- 3+ bed units; 2 allocated spaces per dwelling.
- Parking including Electric and cycle parking – to be provided in accordance standards contained in Barnsley's Parking SPD.
- For C3 dwellinghouses 1 visitor space per 4 dwellings subject to layout. Flexibility for visitor parking will be considered on a site by site basis.
- For garages to be classed as a parking space they must have internal measurements of 3m x 6m (single) or 6m x 6m (double).

The site layout should be designed to maximise and encourage safe, sustainable movement through walking and cycling, providing links to existing networks.

Any boundary, fence, hedge or building shall be set back a minimum of 2m at the end of cul-de-sacs to avoid damage resulting from the overhang of manoeuvring vehicles.

Layout including turning areas to be "tracked" using design vehicles identified above. Carriageway widening on curves may be required based on the following criteria:

- Two cars passing on the curve
- Refuse Vehicle using the entire carriageway

2m long ramps to provide transition between conventional street and shared space street types. Footway lead ins to be provided, extending 2m beyond the top of the ramp to ensure a smooth transition to and from the shared surface for pedestrians.

On streets with design speeds of 20mph and below, forward visibilities below those given in table 7.1 of MfS will be permissible but a minimum of 15m will always apply. Visibility curves may be required to demonstrate compliance with this.

The applicant should be mindful that a maximum of 5 properties can be served from a shared private drive and any private drive that exceeds 20m should include a turning head within the design.

- The development will require a sustainable transport contribution of £131,638 calculated in line with the requirements of the Sustainable Travel SPD (130 Dwellings x 10 Trips x £101.26). It is anticipated that this figure will be reduced in line with the quantum of units expected on the revised developable area.
- Every dwelling is expected to be provisioned with an Electric Vehicle Charging Point (EVCP) as set out in the SPD Sustainable Travel. EVCP can act as a form of mitigation for air quality, however this should be discussed with the Council's Air Quality Officer

Public Rights of Way

- There is an aspiration for the PROW footpath (#40), that extends from Hay Green Lane in the north to the Dearne Valley Parkway further south, to be hard surfaced and increased in width to allow capacity for cyclists and equestrians. This is especially important given the volume of employment land coming forward adjacent to the Parkway as it will allow residents of Birdwell to utilise this transport link to gain access to workplaces in a sustainable manner and thereby reducing the need for vehicle journeys. It is intended that these works can be financed via the sustainable travel contribution.
- A connection is shown to footpath #40 across the undevelopable eastern corner of the estate – the status of this route and future maintenance responsibility should be made clear at an early stage. If it is to be a public right of way or under the responsibility of a landscape management company then an all-weather surface should be provided. The width of the footpath should also be discerned and should fit in with the biodiversity gain/drainage system.
- The western link to the recreation ground should also be clarified with the Council's Parks department so that the connection integrates with the proposal for the NEAP via the off-site POS contribution.
- The emergency access onto Hay Green Lane is likely to be used by pedestrians and the status of this link should also be clarified. The applicant may wish to take steps to prevent this connection acquiring PROW status, such as submitting a deposit under section 31(6) of the Highways Act 1980

Education

- The proposal would create, in line with the SPD Financial Contributions for Schools, the following school places and S106 contribution:

Primary – 27 pupils at £16,000 = £432,000
Secondary – 20 pupils at £16,000 = £320,000

TOTAL - £752,000

- It is expected that this contribution will decrease in value in line with the lower quantum of units proposed as a result of the alterations to the developable area calculation.

The Coal Authority (TCA) & South Yorkshire Mining Advisory Service (SYMAS) + Contaminated Land

- A CMRA has been reviewed by SYMAS who have agreed with the recommendations of the report in relation to intrusive site investigations. They have advised that such measures would be conditioned as a part of any future approval.
- In respect of the report's other recommendations, the Contaminated Land Officer has been consulted and I will provide his comments once received.

Sustainability/Energy

- An Energy/Sustainability Statement should demonstrate how the proposed development would minimise resource and energy consumption compared to the minimum required under current Building Regulations legislation and how it is located and designed to withstand the longer term impacts of climate change. It should also detail how the proposed development would incorporate decentralised, renewable or low carbon energy.
- The statement should include approaches to integrate modern methods of construction and/or higher specification materials to improve energy efficiency beyond current building regulations levels.

Design

- The Building for Life 12 Assessment should be incorporated into the Design and Access Statement. The 12 questions should be answered and accompanied by a justification in accordance with the advice produced by the Design Council CABE: <https://www.designcouncil.org.uk/resources/guide/building-life-12-third-edition>
- A planning application should provide a clear layout of each individual dwelling, its parking arrangement including the number of spaces, the road layout as well as the areas of soft landscaping. The arrangement of external spaces, external separation distances and internal spacing standards should conform to the requirements set out in the SPD Design of Housing Development as well as the South Yorkshire Residential Design Guide.
- A brief summary of the main requirements is as follows:
 - High quality boundary treatments from public vantage points, i.e. brick walls beside highways.
 - One tree per plot – mix of sizes from Rowan and Cherry up to Oak, Beech and Chestnut. Planting is most advantageous in open space and at the front of properties but can also be provided in rear gardens.
 - Hedging or low-level walls to front boundaries.
 - Parking to the front of properties should be broken up by soft landscaping to a ratio of 50/50 soft and hard landscaping as continuous strips of parking is not acceptable.
 - Side of dwelling parking should not occur on corner plots.
 - Street width to height ratios of 1:2 or 1:4 provide good levels of enclosure anything above 1:4 will require street trees.
 - The character of new streets should not be uniform but should vary as part of a hierarchy, depending on their location in order to integrate development into the locality, to retain local distinctiveness and create vibrant, legible and memorable places. Longer streets with continuity of elements tend to have a stronger sense of integration whereas short blocks, arbitrary curves and the lack of a continuous building line create a greater sense of fragmentation and can be disorientating.

- All new streets should be defined by the fronts of plots with buildings orientated to face the public highway, space or private street space to create an active frontage. Blank side elevations facing onto streets should be avoided.
- Avoidance of cul-de-sacs where possible in favour of connected through streets.
- Where front elevations face a road the dwellings should be an appropriate distance apart. The Council will accept a minimum of 12m where the dwellings are of the same storey and it will achieve a streetscape that reflects local character.
- The minimum back-to-back dimension between facing habitable rooms, (ie any room used or intended to be used for sleeping, cooking, living or eating purposes), should be 21metres. Where housing abuts the edge of existing settlements, the back-to-back dimension towards existing housing should be greater than 21metres. Advice will be given on a case by case basis, based on the privacy and outlook of the existing dwelling.
- Where the proposed dwelling/s is/are more than two storeys in height (excluding rooms in the roofspace), the back-to-back separation distance/s should increase by 3 metres for every additional storey.
- Proposed habitable room windows at first floor level and above should be a minimum of 10m from the boundary of any private garden which they would face and habitable room windows in existing dwellings at first floor level and above should be a minimum of 10m from any proposed private garden which they would face. A reduced distance may be accepted for bungalows provided they meet garden size standards and ensure adequate levels of amenity for occupants in terms of outlook, privacy and daylight.
- Proposed walls without habitable room windows (usually side elevations) should be at least 12 metres from original habitable room windows. Where the proposed dwelling is more than two storeys in height (excluding rooms in the roofspace), the separation distance should increase by 2m for every additional storey.
- Rear gardens of proposed dwellings should be at least 50m² in the case of two bedroom houses/bungalows and 60m² for houses/bungalows with three or more bedrooms. Smaller gardens may be acceptable in corner plots if privacy and daylighting can be maintained.
- Internal spacing standards are set out in the table below:

Dwelling Size	Studio 1 Person	1 Bed 1 Person	1 Bed 2 Person	2 Bed 3 Person	3 Bed 4 Person	4 Bed 5 Person or more
Double Bedroom		12	12	12	12	12
Single Bedroom			7	7	7	7
Living Room (L)		13	13	13	15	15
Living/ Dining (DL)		16	16	17	18	19
Dining room (D)						
Kitchen (K)		13	9	11	13	13
Kitchen/ Dining (KD)		9	13	13	11	12
Open Plan/ combined (KDL)		24	24	27	30	
Bathroom/ WC combined	3.5	3.5	3.5	3.5	3.5	3.5
Storage	1.5	2.5	3.5	3.75	4.5	5.5
Overall floor area	33	46	47	62	77	93

K=cooking D=eating L=living

- The above list is not exhaustive and does not include every aspect of guidance set out in the SPD or the SYRDG.

Waste Management

- Long private drives are to be avoided due to the inability of the typical 26 tonne waste collection vehicle to manoeuvre appropriately. Waste Management have indicated that they can supply the tracking information of the vehicle upon request.
- Where private drives are indicated, a bin collection point will need to be installed near to the main highway.

Superfast South Yorkshire (SFSY)

- The development will be expected to provide for full-fibre gigabit capable internet into properties from multiple suppliers. More information in relation to this is provided from Superfast South Yorkshire below:

What is full-fibre? Full-fibre networks use fibre optic cables to connect the exchange directly to each premise. Full-fibre connections are capable of delivering speeds greater than 1 gigabit per second (Gbps; 1 Gbps is equal to 1000 Mbps). Full-fibre networks are more reliable than copper-based networks and cheaper to maintain and operate. Full-fibre networks are also important for supporting high capacity mobile broadband networks, particularly future 5G networks. Full-fibre networks, also referred to as fibre-to-the-premises (FTTP) or fibre-to-the-home (FTTH), consist of fibre optic cables running from the local exchange directly to each premises. Fibre optic cables transmit data using light and can carry more data with faster speeds and significantly less signal loss with distance compared to copper cables.

Developers will be asked to consider installing multiple full fibre infrastructure suppliers in order to provide choice and competition to consumers. Occupiers should be able to access broadband ideally from a choice of at least two providers upon occupation of the premises. Developers should also consider their ability to upgrade the infrastructure in the future in order to minimise disruption to occupiers/users.

Developers should consider the following design principles: minimise and/or mitigate against the visual presence of infrastructure on the façade of buildings, minimise physical obstructions on footpaths and cycle ways, maximise the use of recessed infrastructure, carefully consider the location of cabinets to minimise visual clutter in the street scene.

Broadband Infrastructure Suppliers:

Openreach :

For sites with 30+ plots: <https://www.openreach.com/newsites-registration-form--30--plots-1>

For sites with 2 -29 plots: <https://www.openreach.com/network-services/>

General information: <https://www.openreach.com/fibre-broadband/fibre-for-developers>

Virgin Media:

<https://www.virginmedia.com/lightning/network-expansion/property-developers>

CityFibre:

<https://www.cityfibre.com/property/>

Hyperoptic

<https://hyperoptic.com/>

Gigaclear

<http://www.gigaclear.com/>

(This list is not exhaustive)

Other support:

The Superfast South Yorkshire Team is available to offer advice and discuss connectivity solutions to new sites with developers and can be emailed at hello@superfastsouthyorkshire.co.uk

For more information please visit: <http://www.superfastsouthyorkshire.co.uk/sfsy/developments>

Northern Powergrid

The following response was gained from Northern Powergrid in respect of the overhead lines which cross the south eastern corner of the site:

As mentioned in the case of Wayleave Agreements these are terminable specific to the landowner which protects our rights for our apparatus on third party land. In the case of Permanent Deeds these are legal documents and as the name suggests provide us with permanent legal rights for our apparatus to remain in place. Our Wayleave Team will better explain the position in respect of either document type.

For your information and setting out the position of the Company we are a Statutory Utility with obligations to maintain and ensure electrical supplies throughout both our north and southern areas are not compromised. The documentation we have to protect our rights on private land reflect this obligation. I can't emphasise enough how important it is that supplies are not affected and with any up and coming development in any area that can impact on our apparatus it is vital we are given the opportunity to consider the effects and open up lines of communication to ensure we minimise the impact.

The LPA's understanding of the consultee response is that if the overhead lines are subject to a Wayleave Agreement, the landowner who has entered into the agreement with Northern Powergrid can request for the lines to be placed underground. The liability for the financial cost of undertaking this work is unknown but may potentially be worth exploring with Northern Powergrid if the land is able to be made 'developable'. If deeds are held by Northern Powergrid for use of the land upon which the infrastructure resides, then it is unlikely that the infrastructure can be moved without the cost being borne by the developer.

Responses to Queries Raised in Cover Letter

Early release of Area B for residential development:

This is a matter for planning policy upon review of the Local Plan in 2024. Area B is currently within the Safeguarded allocation SL4 and the principle of development is not to be considered at the current time given that the Local Plan has been adopted only one year prior.

The Authority Monitoring Report and Five Year Deliverable Housing Supply Note are in the process of being reviewed. If the housing land supply proves to be insufficient, it is expected that a call for sites will be undertaken upon review of the plan in the years to come, at which point your client will be able to promote their site as a viable residential allocation.

In respect of the redline boundary, this should only reflect the boundary of HS59 as indicated on the Local Plan Policies Map. Any incursion of the red line into the surrounding safeguarded land allocation will not be acceptable.

There will be a requirement for the road layout to provide a viable access onto SL4 and the outline application should provide scope for this possibility in the submission of its layout.

The proposed access off Hay Green Lane; The proposed scope of the Transport Assessment and confirmation of any site-specific considerations; The illustrative masterplan including layout and key technical matters;

These points have been addressed in the consultation responses above. Any queries relating to the issues identified therein can be raised with the case officer.

Consultation requirements with local residents and the Parish Council;

A Planning Statement should set out the relevant National, Regional and Local planning policies and guidance and explain how the development relates to the various policies. It should include details of pre-application discussions and summarise the other technical documents submitted with the application (e.g. Flood Risk Assessment).

A Planning Statement should also include a summary of any Community Consultation that was carried out prior to submission of the application. There is, however, no obligation for community consultation to be carried out by the applicant. The LPA will not participate in consultations undertaken by the applicant or agent and will consult adjoining properties separately through neighbour notification letters, site notices and a press notice.

* At the time of writing, the LPA are aware that a consultation event is to be held by the applicant at Birdwell Primary School on Wednesday 12th February 2020 between 15.30-19.00.

Likely Section 106 contributions;

Education - £752,000

Sustainable Travel - £131,638

POS Off-site Contribution – Calculated from Housing Mix – The case officer can provide the GS contribution calculator upon request.

Affordable Housing – 13 Dwellings.

As stated above, these figures are indicative and subject to change dependent on the housing density and developable area forwarded by the applicant. It should be noted that the LPA expect the allocation yield, as indicated in the Local Plan, to be met in full. Any shortfall in developable area will require an increase to the site density to reach the yield.

In relation to the submission of a viability appraisal (VA) with an outline application, such information would not be based upon a concrete set of plans and would thus not present the level of detail required to accurately predict build costs given the length of time that typically occurs between outline and reserved matters permissions. Likewise the estimation of revenues stemming from development sites' have proven unreliable at early outline application stages and such forecasting is to be discouraged until submission of a reserved matters application. Consequently any VA would be afforded minimal weight through the outline assessment process and would likely result in the applicant's time being squandered in lieu of the timely processing of the outline permission. At this preliminary stage, the LPA wish to see the required provisions set out as formulas in an S106 Agreement.

Indeed the majority of policy requirements, such as education contributions, are non negotiable and require refusal of an application where school place shortages exist and where a full financial contribution is not forthcoming. Similarly it is acknowledged that affordable housing policy is capable of being subject to VAs, but given that the requirement in this part of the Borough is only 10% and

that the minimum provision set out in the NPPF requires large sites to also deliver a minimum of 10%, it would be against both local and national policy to deliver less. Both the aforementioned examples reinforce the surplus requirement for a VA should an outline application be submitted.

Validation requirements to support an application on the site:

- Affordable Housing Statement
- Air Quality Assessment
- Arboricultural Impact Assessment & Method Statement
- Phase One Habitat Survey
- Bat Survey of Ash Tree
- Heritage Impact Assessment
- Geophysical Archaeological Survey
- A Flood Risk Assessment, conceptual Drainage Plan and SUDS Design Statement
- Foul Drainage Assessment if not connecting foul water to public sewer.
- Noise Assessment
- Traffic Assessment and Travel Plan
- Energy/Sustainability Statement
- Design and Access Statement with Building for Life 12 incorporated.
- Planning Statement
- Coal Mining Risk Assessment/Report of Intrusive Site Investigations and Mitigation (awaiting Contaminated Land and SYMAS responses)
- S106 Draft Head of Terms
- National validation requirements

Please note that any survey or report submitted must be less than 2 years old unless they are accompanied with an addenda which has reviewed the original survey or report and updated findings and mitigation proposals to satisfy current requirements where appropriate. Any additional requirements as set out in the consultee comments should also be in your application.

You should note that the council reserves the right to request additional information should that be considered necessary or be identified through the consultation process. It may also be possible to look to combine some of the documents where appropriate. Validation would depend upon receipt of all relevant forms, fees, plans and documents.

These comments will be treated as a material consideration in relation to any planning application submitted in the next 2 years for a scheme that reflects the advice provided in this letter. However, please note that the views and opinions in this letter do not constitute a formal response or decision of the Council in relation to any future planning application

The timing of an application for residential development:

You provided an indication during our meeting on the 13th January 2020 that you intend to submit an outline application including access with all other matters reserved, in March 2020. The LPA do not have a preference as to when the application is to be submitted as long as the validation requirements are satisfied. However the LPA would request the applicant to be mindful of the six week Purdah period in advance of the Local Elections on the 7th May 2020.

If you require any further information on the above please do not hesitate to contact me.

Yours sincerely

Richard Gilbert

For and on behalf of
Development Management
Joe Jenkinson BA (Hons) MSc
Head of Planning and Building Control

www.barnsley.gov.uk/developmentmanagement